

GRADING NOTES

1. A Geotechnical Engineer shall be employed by the owner and be on site during grading operations. All soils tests shall be verified by the Geotechnical Engineer concurrent with the grading and backfilling operations.
2. The grading contractor shall perform a complete grading and compaction operation as shown on the plans, stated in these notes, or reasonably implied there from, all in accordance with the plans and notes as interpreted by the Geotechnical Engineer.
3. The Contractor shall notify the Soils Engineer at least two days in advance of the start of the grading operation.
4. All areas shall be allowed to drain. All low points shall be provided with temporary ditches.
5. A sediment control plan that includes monitored and maintained sediment control basins and/or straw bales should be implemented as soon as possible. No graded area is to be allowed to remain bare without being seeded and mulched. Care should be exercised to prevent soil from damaging adjacent property and silting up existing downstream storm drainage system.
6. Debris and foundation material from any existing on-site building or structure which is scheduled to be razed for this development must be disposed of off-site.
7. All trash and debris on site, either existing or from construction, must be removed and properly disposed of off-site.
8. Soft soil in the bottom and banks of any existing or former pond sites or tributaries or on any sediment basins or traps should be removed, spread out and permitted to dry sufficiently to be used as fill. None of this material should be placed in proposed public right-of-way locations or on any storm sewer locations.
9. Site preparation includes the clearance of all stumps, trees, bushes, shrubs, and weeds; the grubbing and removal of roots and other surface obstructions from the site; and the demolition and removal of any man-made structures. The material shall be properly disposed of off-site. Topsoil and grass in the fill areas shall be thoroughly disced prior to the placement of any fill. The Soils Engineer shall approve the discing operation.
10. Compaction equipment shall consist of tamping rollers, pneumatic-tired rollers, vibratory roller, or high speed impact type drum rollers acceptable to the Soils Engineer. The roller shall be designed so as to avoid the creation of a layered fill without proper blending of successive fill layers.

1. The Soils Engineer shall observe and test the placement of the fill to verify that specifications are met. A series of fill density tests will be determined on each lift of fill. Interim reports showing fill quality will be made to the Owner at regular intervals.
2. The Soils Engineer shall notify the Contractor of rejection of a lift of fill or portion thereof. The Contractor shall rework the rejected portion of fill and obtain notification from the Soils Engineer of its acceptance prior to the placement of additional fill.
3. All areas to receive fill shall be scarified to a depth of not less than 6 inches and then compacted in accordance with the specifications given below. Natural slopes steeper than 1 vertical to 5 horizontal to receive fill shall have horizontal benches, cut into the slopes before the placement of any fill. The width and height to be determined by the Soils Engineer. The fill shall be loosely placed in horizontal layers not exceeding 8 inches in thickness and compacted in accordance with the specifications given below. The Soils Engineer shall be responsible for determining the acceptability of soils placed. Any unacceptable soils placed shall be removed at the Contractor's expense.
4. The sequence of operation in the fill areas will be fill, compact, verify acceptable soil density, and repetition of the sequence. The acceptable moisture contents during the filling operation are those at which satisfactory dry densities can be obtained. The acceptable moisture contents during the filling operation in the remaining areas are from 2 to 5 percent above the optimum moisture control.

5. The surface of the fill shall be finished so that it will not impound water. If at the end of a days work it would appear that there may be rain prior to the next working day, the surface shall be finished smooth. If the surface has been finished smooth for any reason, it shall be scarified before proceeding with the placement of succeeding lifts. Fill shall not be placed on frozen ground, nor shall filling operations continue when the temperature is such as to permit the layer under placement to freeze.

6. Fill and backfill should be compacted to the criteria specified in the following table:

CATEGORY	MINIMUM PERCENT COMPACTION
Fill in building areas below footings	90%
Fill under slabs, walks, and pavement	90%
Fill other than building areas	88%
Natural subgrade	88%
Pavement subgrade	90%
Pavement base course	90%

Measured as a percent of the maximum dry density as determined by modified Proctor Test (ASTM-D-1557).

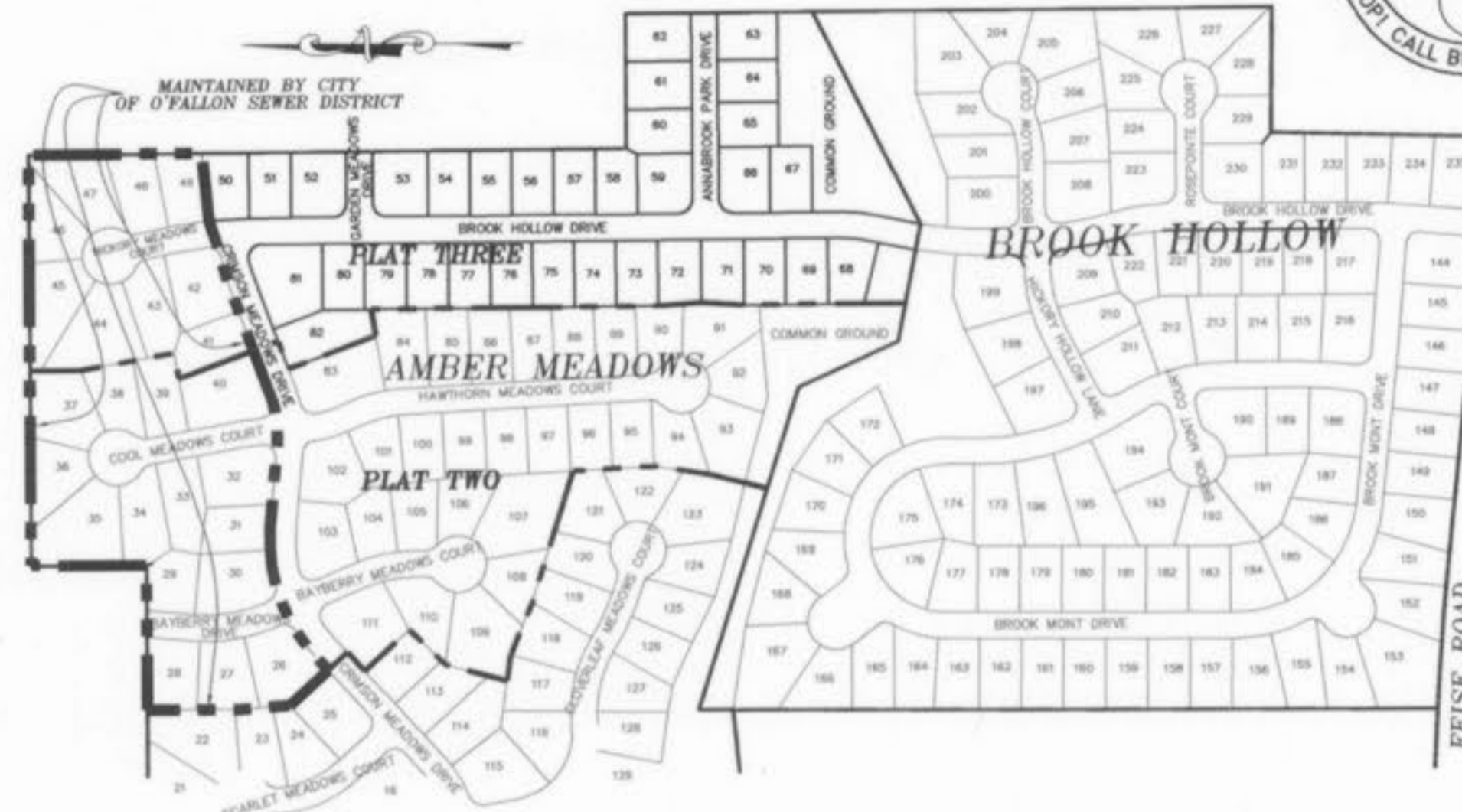
Moisture content must be within 2 percent below or 4 percent above optimum moisture content if fill is deeper than 10 feet.

GENERAL NOTES

1. Underground utilities have been plotted from available information and therefore their locations shall be considered approximate only. The verification of the location of all underground utilities, either shown or not shown on these plans shall be the responsibility of the contractor, and shall be located prior to any grading or construction of the improvements.
2. All manhole tops & flowlines built without elevations furnished by the Engineer will be the responsibility of the sewer contractor.
3. 8" P.V.C. sanitary sewer pipe shall meet the following standards. A.S.T.M.-D-3034 SDR-35, with wall thickness compression joint A.S.T.M.-D-3212. An appropriate rubber seal waterstop as approved by the sewer district shall be installed between P.V.C. pipe and masonry structures.
4. All filled places, including trench backfills, under buildings, proposed storm and sanitary sewer lines and/or paved areas, shall be compacted to 90% maximum density as determined by the "Modified AASHTO T-180 Compaction Test," (A.S.T.M.-D-1557). All filled places within public roadways shall be compacted to 95% of maximum density as determined by the "Standard Proctor Test AASHTO T-99, Method C" (A.S.T.M.-D-698).
5. All trench backfills under paved areas shall be granular backfill, and shall be compacted to 90% of the maximum density as determined by the "Modified AASHTO T-180 Compaction Test," (A.S.T.M.-D-1557). All other trench backfills may be earth material (free of large clods or stones). All trench backfills shall be water jetted.
6. All sanitary house connections have been designed so that the minimum vertical distance from the low point of the basement to the flow line of a sanitary sewer at the corresponding house connection is not less than the diameter of the pipe plus the vertical distance of 2 1/2 feet.
7. No area shall be cleared without the permission of the Project Engineer.
8. All P.V.C. sanitary sewer is to be SDR-35 or equal with clean 1/2" to 1" granular stone bedding uniformly graded. This bedding shall extend from 4" below the pipe to the springline of the pipe. Immediate backfill over pipe shall consist of same size "clean" or minus stone from springline of pipe to 6" above the top of pipe.
9. All soils test shall be verified by a Soils Engineer concurrent with the grading and backfilling operations.
10. Easements shall be provided for sanitary sewers, and all utilities on the Record Plat. See Record Plat for location and size of easements.
11. Maintenance and upkeep of the common ground area shall be the responsibility of the developer and/or successors.
12. A 25' building line shall be established along all Public Rights-Of-Way.
13. All water lines shall be laid at least 10 feet horizontally, from any sanitary sewer, storm sewer, or manhole. 18" vertical clearance from outside of pipe to outside of pipe shall be maintained wherever water lines must cross sanitary sewers, laterals, or storm drains. The water line shall be laid at such an elevation that the bottom of the water line is above the top of the drain or sewer. A full length of water pipe shall be centered over the sewer line to be crossed so that the joints will be equally distant from the sewer and as remote therefrom as possible. This vertical separation shall be maintained for that portion of the water line located within 10 feet horizontally, of any sewer or drain it crosses.
14. All PVC water pipe shall conform to A.S.T.M.-D-2241, SDR 21 Standard Specification for P.V.C. Pressure Pipe, 200 P.S.I. working pressure for water, with approved joint.
15. Water lines, valves, sleeves, meters, and fittings shall meet all specifications and installation requirements of Public Water Supply District No. 2 of St. Charles County.
16. All water hydrants and valves shall be ductile iron and installed in accordance with plans and details. All ductile iron pipe for water mains shall conform to A.W.W.A. Specifications C-106 and/or C-108. The ductile iron fittings shall conform to A.W.W.A. Specification CC-110. All rubber gasket joints for water ductile iron pressure pipe and fittings shall conform to A.W.W.A. Specification C-111.
17. All sanitary manholes shall be waterproofed on the exterior in accordance with Missouri Department of Natural Resources specifications 10 CSR-8.120 (7E).
18. Brick will not be used in the construction of sanitary sewer manholes.
19. All pipes shall have positive drainage through manholes. No flat base structures are allowed.
20. The City of O'Fallon and Duckett Creek Sanitary District shall be notified 48 hours prior to construction for coordination and inspection.
21. Gas, water and other underground utilities shall not conflict with the depth or horizontal location of existing or proposed sanitary or storm sewers, including house laterals.
22. All existing site improvements disturbed, damaged or destroyed shall be repaired or replaced to closely match pre-construction conditions.
23. The contractor shall prevent all storm, surface water, mud and construction debris from entering the existing sanitary sewer system.
24. All construction and materials shall conform to the current construction standards of the City of O'Fallon and Duckett Creek Sanitary District.
25. All sanitary and storm sewer trench backfills shall be water jetted. Granular backfill will be used under pavement areas.

A SET OF AS-BUILT PLANS FOR
AMBER MEADOWS
PLAT THREE

A TRACT OF LAND BEING PART OF FRACTIONAL SECTION 6,
TOWNSHIP 46 NORTH, RANGE 3 EAST
OF THE FIFTH PRINCIPAL MERIDIAN,
CITY OF O'FALLON, ST. CHARLES COUNTY, MISSOURI



SHEET INDEX

- 1 COVER SHEET
- 2-3 SITE PLANS
- 4-5 SANITARY SEWER PROFILES
- 6-7 STORM SEWER PROFILES



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DEVELOPMENT NOTES

1. Area of plat three Duckett area only: 12.21
2. Existing Zoning: R-1 (City of O'Fallon)
3. Proposed Use: Single Family Residential
4. Number of Lots Proposed: 33 Lots
5. Area in Common Ground: 1.53 Acres
6. Area in Right-of-Way: 2.40 Acres
7. Area in Lots: 8.28 Acres
8. Average Lot Area (not including common ground): 14,097 Square Feet
9. Average Lot Area (including common ground): 16,117 Square Feet
10. The proposed height and lot setbacks are as follows:
Minimum Front Yard: 25 feet
Minimum Side Yard: 5 feet
Minimum Rear Yard: 25 feet
Minimum Lot Area: 10,000 square feet
Maximum Height of Building: 2 1/2 stories or 35 feet
11. Current Owner of Property: First Land Company of St. Charles County, Inc. P.O. Box 176 St. Peters, MO 63376
12. Site is served by: Duckett Creek & City of O'Fallon sewers
AmerenUE
St. Charles Gas Company
St. Charles County P.W.S.D.No. 2
Verizon Telephone Company
Fort Zumwalt School District
O'Fallon Fire Protection District

13. No Floodplain exists on this tract per F.I.R.M. #29183C0240 E. dated Aug. 2, 1996.
14. Topographic information is by Walker & Associates.
15. Boundary information is per survey completed by Bax Engineering in September of 2001.
16. One tree shall be planted for every lot. Every corner lot shall have two street trees planted in the right-of-way.
17. All local streets will be constructed to City of O'Fallon standards. Streets will consist of 26 foot wide concrete pavement with integral rolled curb centered in a 50 foot right-of-way. Minimum radius shall be 150 feet.
18. All cul-de-sacs and bubbles will have pavement radii of 42 feet with right-of-way radii of 54 feet. Street intersections shall have a minimum rounding radius of 25 feet with pavement radii of 37 feet.
19. Minimum street grades shall be 1%.
20. A 4 foot wide concrete sidewalk shall be constructed on one side of streets where indicated.
21. All homes shall have a minimum of 2 off-street parking places with 2-car garages.
22. All proposed utilities must be located underground.
23. The developer realizes that they will comply with current Tree Preservation Ordinance Number 1689 and provide landscaping as set forth in Article 23 of the City of O'Fallon Zoning Ordinances.
24. Additional lighting may be required by the City of O'Fallon.
25. The following lots are susceptible to street movement: 79 & 80, 71 & 72

26. Tree Preservation Calculations:

Existing trees	11.49 acres
x 20%	2.30 acres
Saved trees	4.20 acres
Trees removed	7.29 acres

27. Landscape Requirements
1 Tree per lot & 2 trees per corner lot = 165
Trees provided = 178

27. Detention for this development to be provided in detention basins.
28. All existing creeks to remain shall be enclosed within a stormwater discharge easement. The width of which to be specified by the size of the water shed drained by the creek and to be determined during the improvement plan approval process.
29. Per section 405.120 of the O'Fallon City Code, written verification compliance with the construction plans shall be required by the developer.
Proposed rip-rap pads to be evaluated following installation to determine if pad is of sufficient size to prevent erosion.
30. Street trees to have a minimum of 2" caliper per O'Fallon standards. Species to be selected by homeowner from O'Fallon Tree Planting Guide. Street trees to be maintained by the Home Owner's Association per subdivision C.C. & R.'s.
31. Where disturbance occurs within the 60' wide drainage easement during construction, the area will be heavily revegetated following completion of project.

52. Contractor shall assume complete responsibility for controlling all siltation and erosion of the project area. The contractor shall use whatever means necessary to control erosion and siltation including, but not limited to, staked straw bales and/or siltation fabric fences (possible methods of control are detailed in the plan). Control shall commence with grading and be maintained throughout the project until acceptance of the work by the owner and/or the City of O'Fallon. The contractor's responsibilities include all design and implementation as required to prevent erosion and the depositing of silt. The Owner and/or the City of O'Fallon may at their option direct the contractor in his methods as deemed fit to protect property and improvements. Any depositing of silts or mud on new or existing pavement or in new or existing storm sewers or swales shall be removed after each rain and affected areas cleaned to the satisfaction of the Owner and/or City of O'Fallon.

VEGETATIVE ESTABLISHMENT
For Urban Development Sites

Seeding Rates:
Permanent:
Tall Fescue - 30 lbs./ac.
Smooth Brome - 20 lbs./ac.
Combined Fescue @ 15 lbs./ac. and Brome @ 10 lbs./ac.
Temporary:
Wheat or Rye - 150 lbs./ac. (3.5 lbs. per square foot)
Oats - 120 lbs./ac. (2.75 lbs. per square foot)
Seeding Periods:
Fescue or Brome - March 1 to June 1
Wheat or Rye - August 1 to November 1
Oats - March 15 to September 15
Mulch Rates: 100 lbs. per 1,000 sq. feet (4,356 lbs. per acre)
Fertilizer Rates: Nitrogen 30 lbs./ac.
Phosphate 30 lbs./ac.
Potassium 30 lbs./ac.
Lime 600 lbs./ac. ENM*
* ENM = effective neutralizing material as per State evaluation of quarried rock.

REFERENCE BENCHMARK

R.M. #65 - ELEV.=509.47 (U.S.G.S. DATUM)
CHISELED "L" ON THE SOUTH END OF THE WEST HEADWALL OF COUNTY HIGHWAY K BRIDGE OVER BELLEAU CREEK.

SITE BENCHMARK

ELEV.=667.66 NAVD 1929 DATUM (U.S.G.S.)
ST. CHARLES COUNTY GEOGRAPHIC REFERENCE STATION "ORF" STANDARD BRASS DISK STAMPED "ORF 1931" IN A SQUARE CONCRETE POST IN A SMALL MOUND ± 10" NORTHWEST OF THE NORTHWEST CORNER OF A SHED ADDITION TO AN OLDER BARN; 20'-25" SOUTHWEST OF A SMALL POND; 39' NORTHEAST OF A LONE PEAR TREE AND 24.9' NORTHEAST OF A METAL WITNESS POST AND SIGN. LOCATED AT 1301 BRYAN ROAD 350' NORTHWEST OF HOUSE.

SEWER MEASUREMENTS

THE EXISTING SEWER LENGTHS, SIZES, FLOWLINES, DEPTHS OF STRUCTURES AND SEWERS AND LOCATIONS WITH RESPECT TO EXISTING OR PROPOSED EASEMENTS HAVE BEEN MEASURED. THE RESULTS OF THOSE MEASUREMENTS ARE SHOWN ON THIS SET OF FINAL MEASUREMENT PLANS.

ALL PUBLIC SEWERS ARE LOCATED WITHIN DESIGNATED EXISTING OR PROPOSED EASEMENTS EXCEPT AS FOLLOWS:

SIGNED: _____
P.E./L.S. _____
DATE _____



ASBUILTS NOTE:
ALL DISTANCE AND SLOPE CALCULATIONS ARE FROM CENTER OF STRUCTURE TO CENTER OF STRUCTURE.

DISCLAIMER OF RESPONSIBILITY
I hereby certify that the documents attached to this sheet, and I hereby disclaim any responsibility for all other drawings, specifications, estimates, reports or other documents or instruments relating to or intended to be used for any part or parts of the architectural or engineering project or survey.

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REVISIONS

NO.	DATE	DESCRIPTION

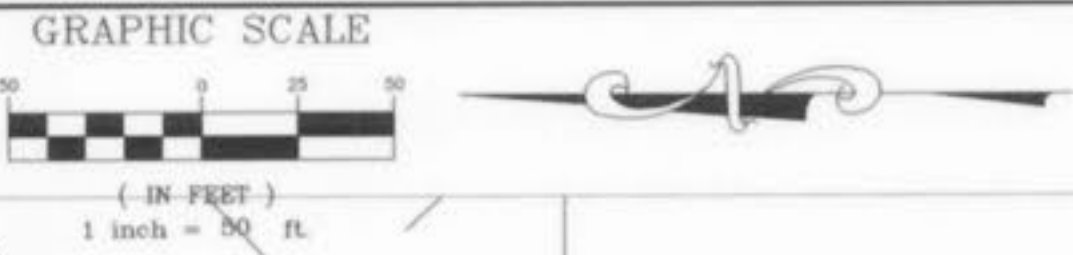


1052 South Cloverleaf Drive
St. Peters, MO. 63376-6445
314-928-5552
FAX 928-1718

7-21-03
DATE
00-11289
PROJECT NUMBER
1
SHEET OF
11289AASB3.DWG
FILE NAME
ECF
DRAWN CHECKED

AS-BUILTS ADDED JULY, 2004

Amber Meadows plat 3 App Aug 27, 2004 JAK



Blow-off hydrants and water meters shall not be located in any pavement or hard surfaced areas including, but not limited to, driveways, sidewalks, walkways, and streets. Since the location of all such areas is not shown on this plan all costs to relocate any blow-off hydrants and water meters from any pavement or hard-surfaced areas shall be borne by the Developer or the builder.



Underground utilities have been plotted from available information and, therefore, their locations shall be considered approximate only. The verification of the location of all underground utilities, either shown or not shown, on these plans shall be the responsibility of the Contractor, and shall be located prior to any grading or construction of the improvements.

Note: Street trees shown are to be installed upon completion of home site by homebuilder in conjunction with sidewalk installation.

AS-BUILTS ADDED JULY, 2004

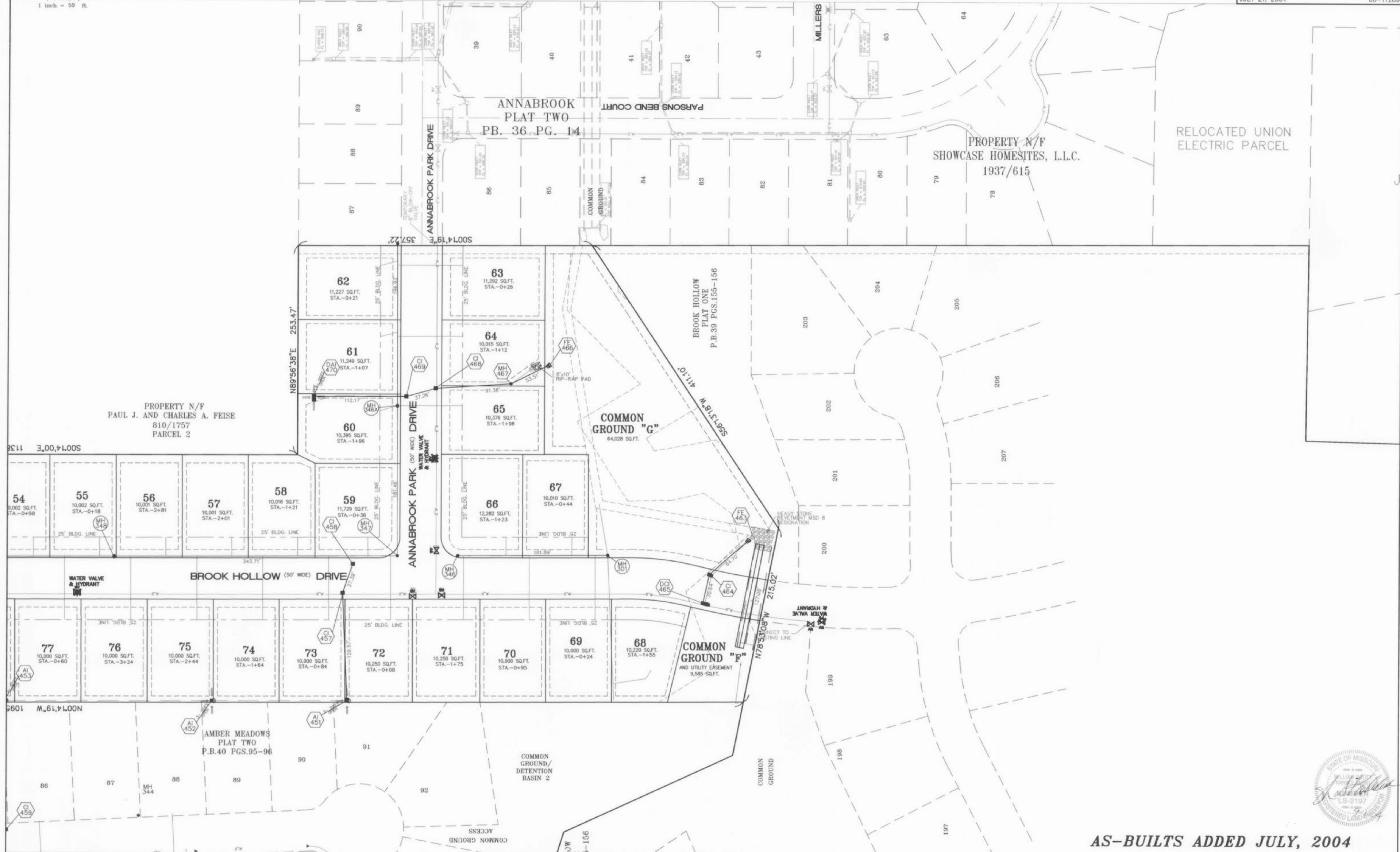
These engineering plans have been prepared at the request of the developer for construction with some rock data, but not sufficient enough to determine the exact location of all existing rock conditions.

If existing rock conditions are encountered during construction it shall be the responsibility of the developer and or his contractor to contact Box Engineering Co., Inc. and the soils engineer for the project at the time of encounter to determine the best design to continue construction.





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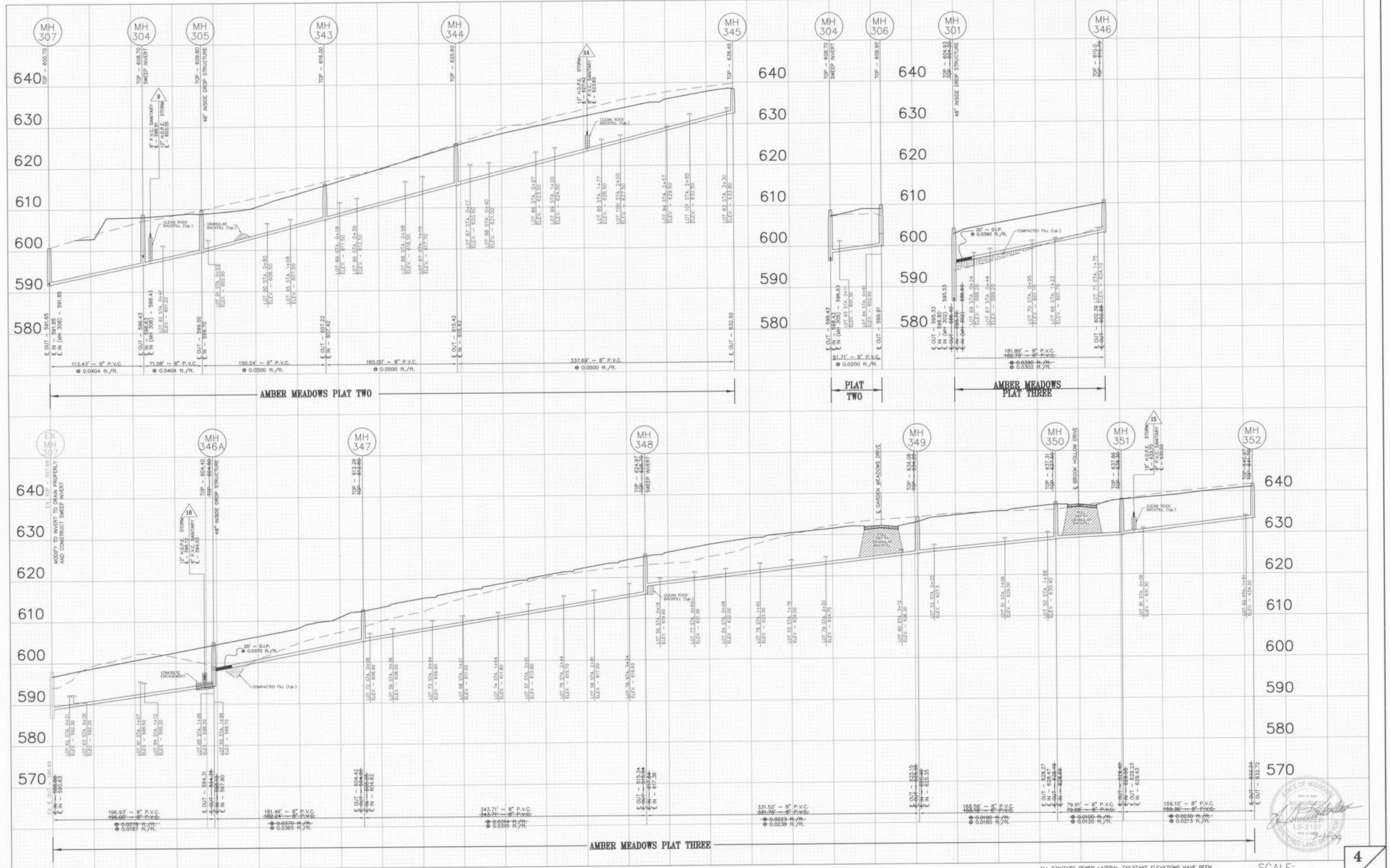
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AS-BUILTS ADDED JULY, 2004

Ambermeadows Plat 3 App 3/2/04 ANK

NOTE: ALL SANITARY SEWER ON THIS SHEET WITHIN
 DUCKETT CREEK SANITARY SEWER DISTRICT.



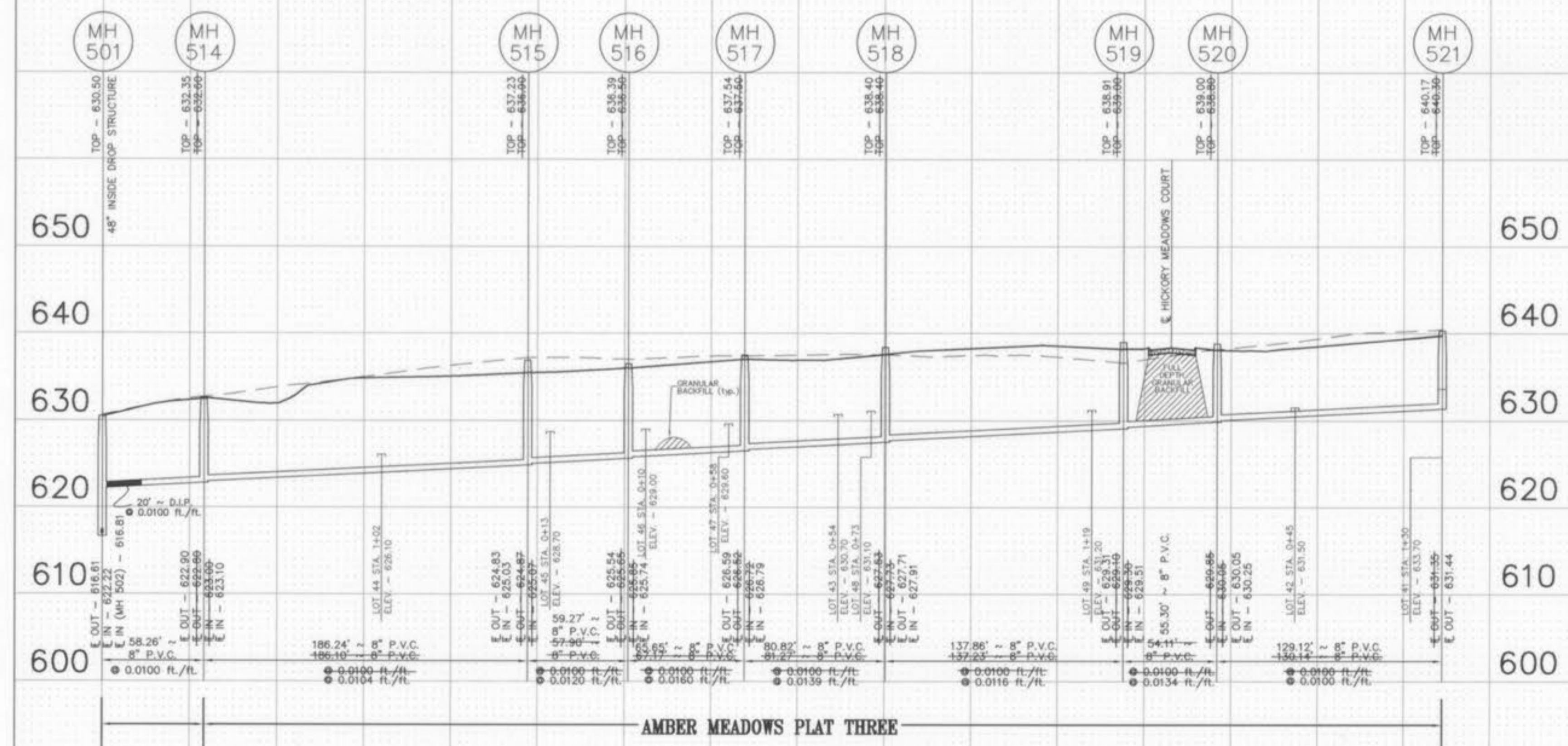
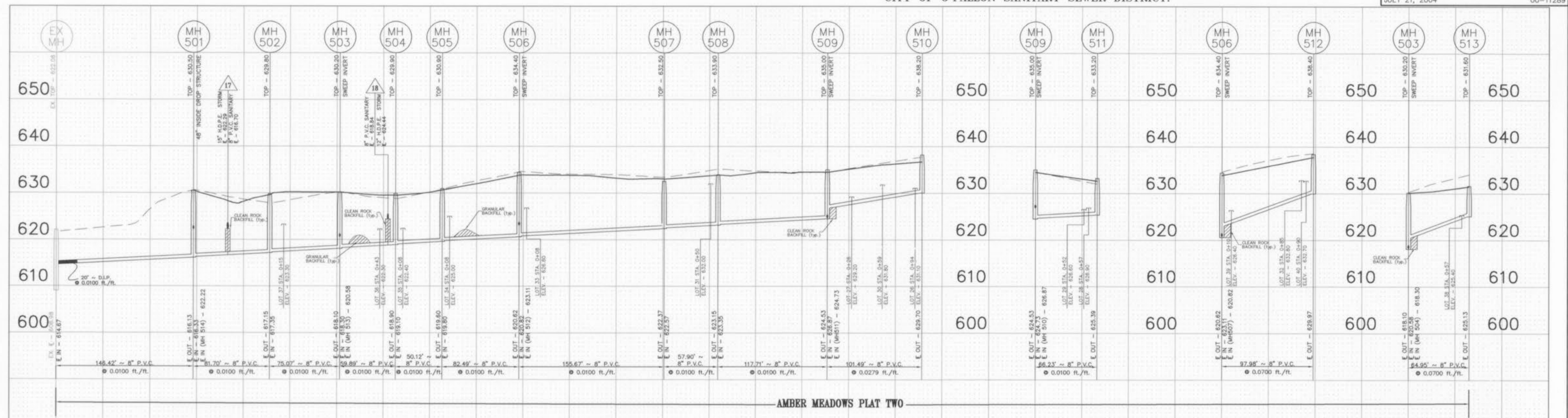
UNDERGROUND UTILITIES HAVE BEEN PLOTTED FROM AVAILABLE INFORMATION AND THEREFORE THEIR LOCATIONS SHALL BE CONSIDERED APPROXIMATE ONLY. THE VERIFICATION OF THE LOCATION OF ALL UNDERGROUND UTILITIES, EITHER SHOWN OR NOT SHOWN ON THESE PLANS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR, AND SHALL BE LOCATED PRIOR TO ANY GRADING OR CONSTRUCTION OF THE IMPROVEMENTS.

AS-BUILTS ADDED JULY, 2004

ALL SANITARY SEWER LATERAL TAILSTAKE ELEVATIONS HAVE BEEN DESIGNED FOR AN 8.0' BASEMENT POUR.

SCALE:
 VERTICAL = 1" = 10'
 HORIZONTAL = 1" = 50'

NOTE: ALL SANITARY SEWER ON THIS SHEET WITHIN
 CITY OF O'FALLON SANITARY SEWER DISTRICT.



NOTE: ALL SANITARY SEWER ON THIS SHEET TO BE
 INSTALLED PER CITY OF O'FALLON REQUIREMENTS.
 SEE O'FALLON ORDINANCES FOR REQUIREMENTS NOT
 PER DUCKETT CREEK DETAILS AT THE END OF THIS
 SET.

AMBER MEADOWS
 PLAT TWO

AMBER MEADOWS
 PLAT THREE



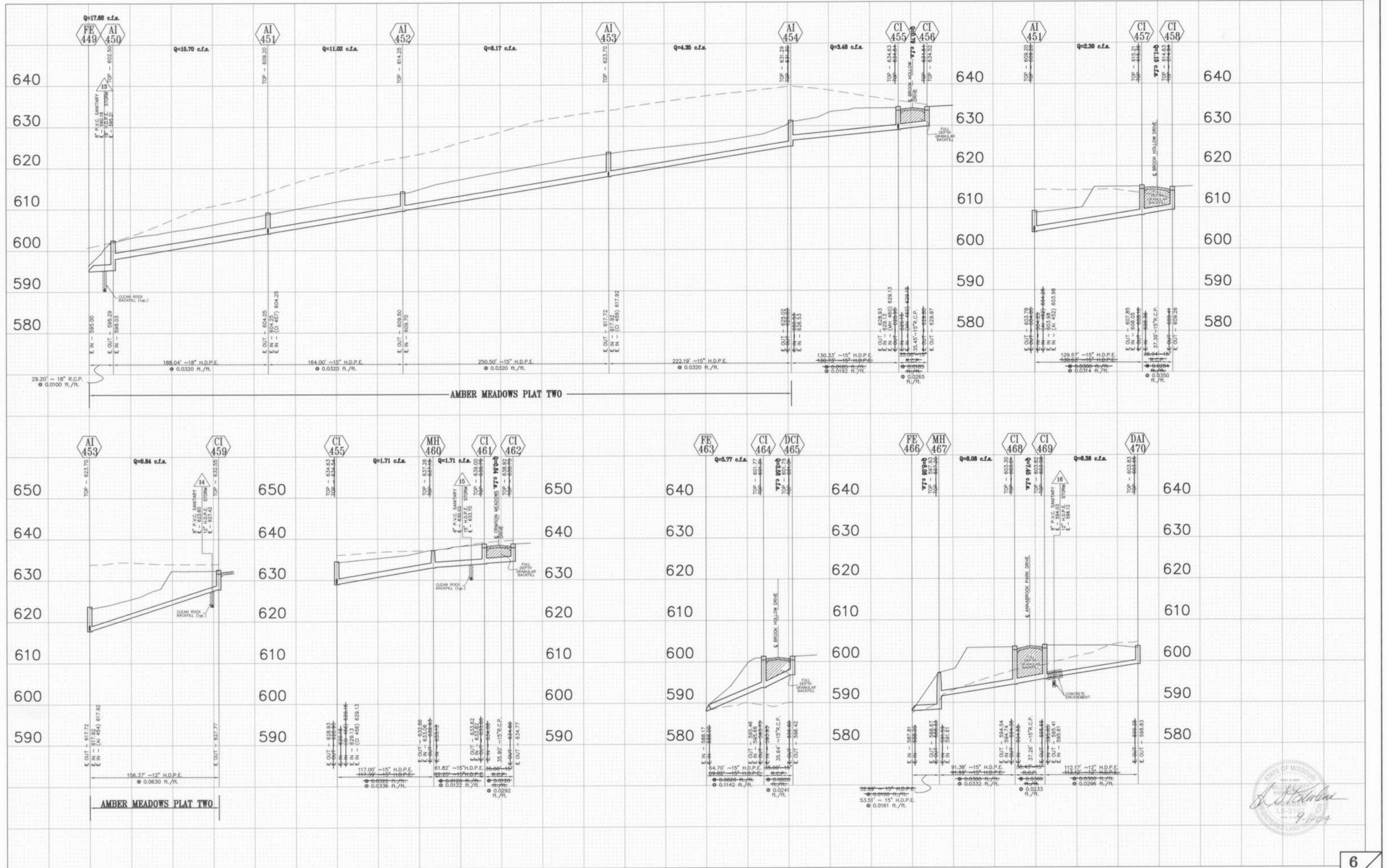
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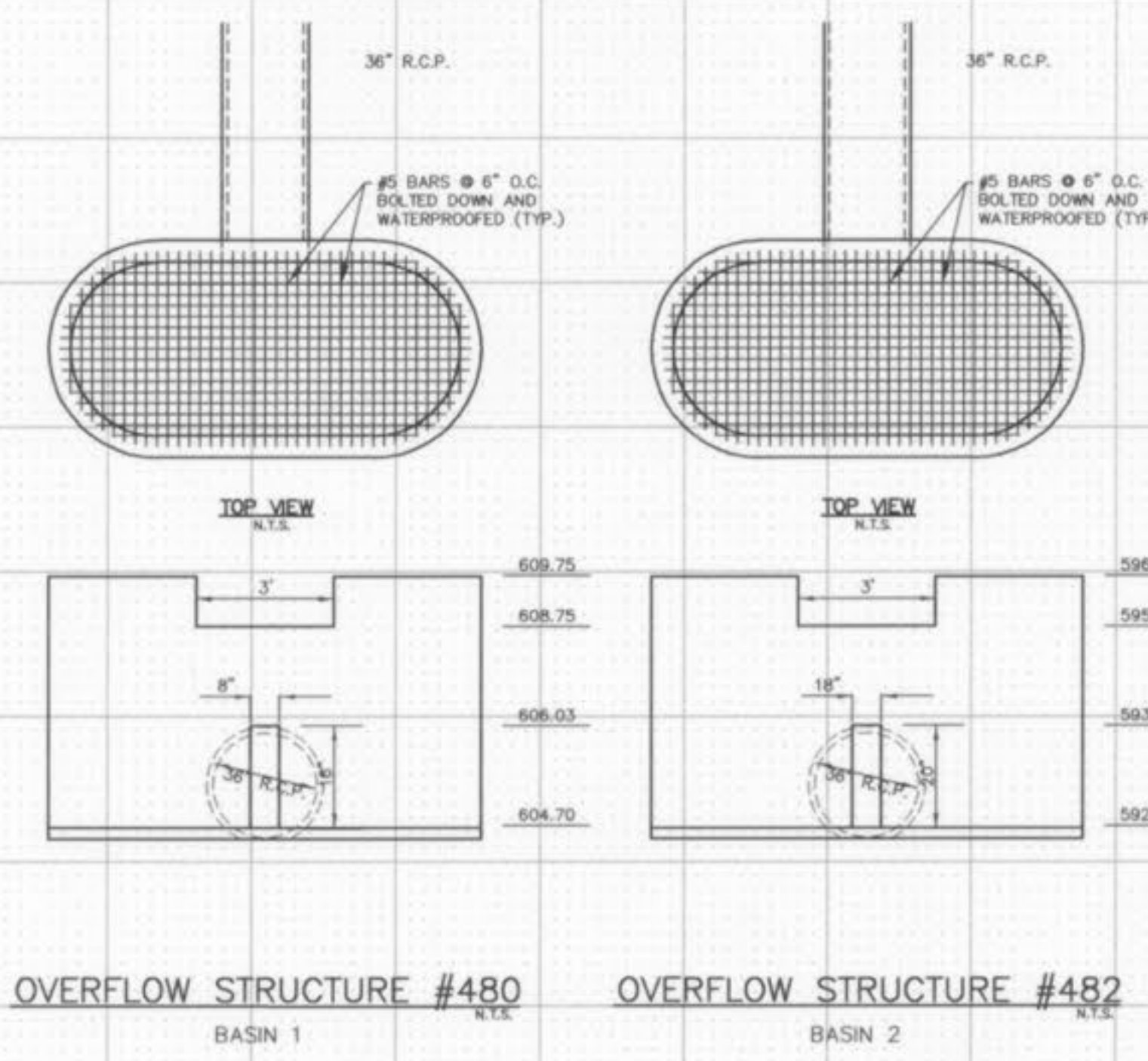
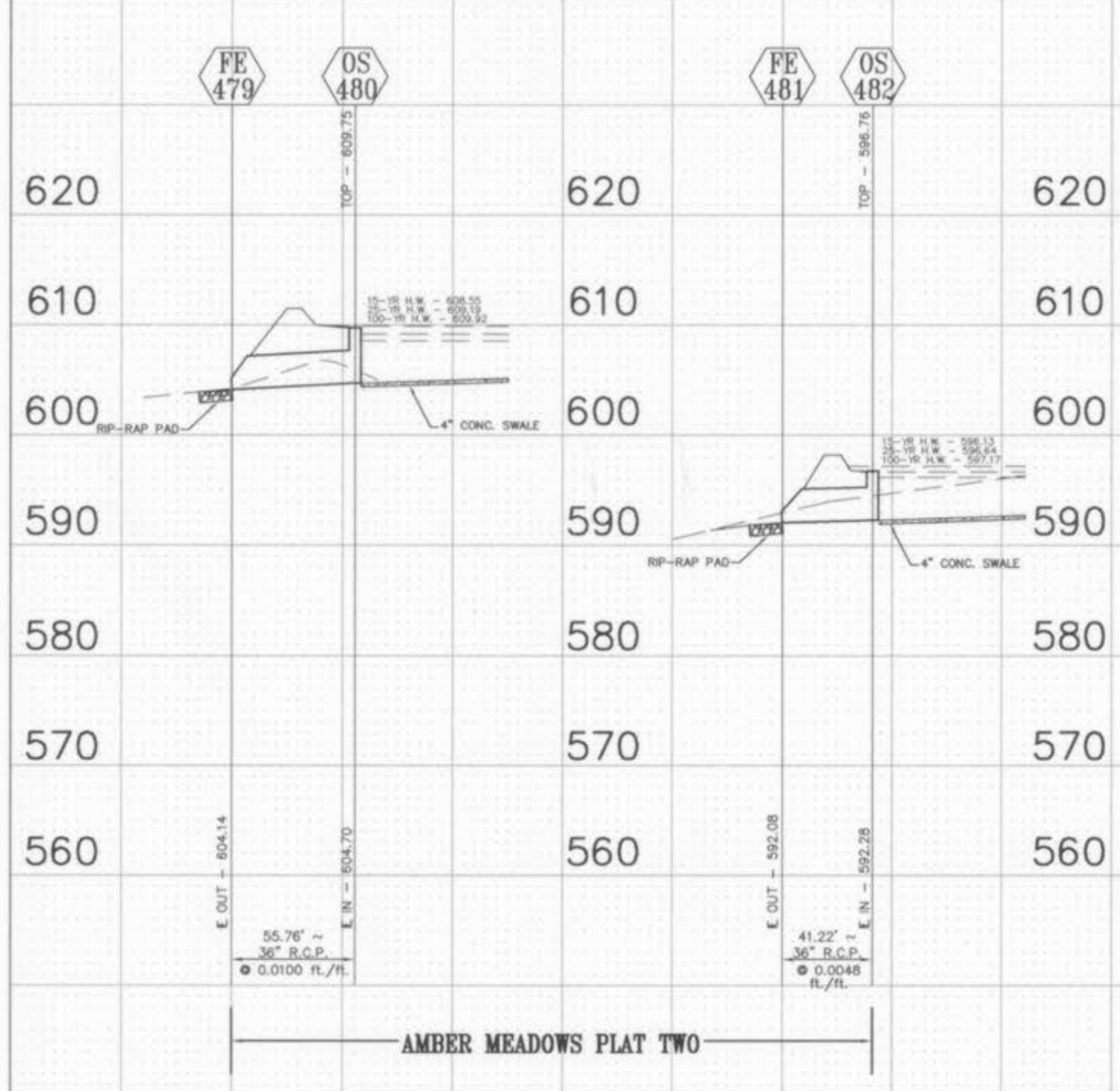
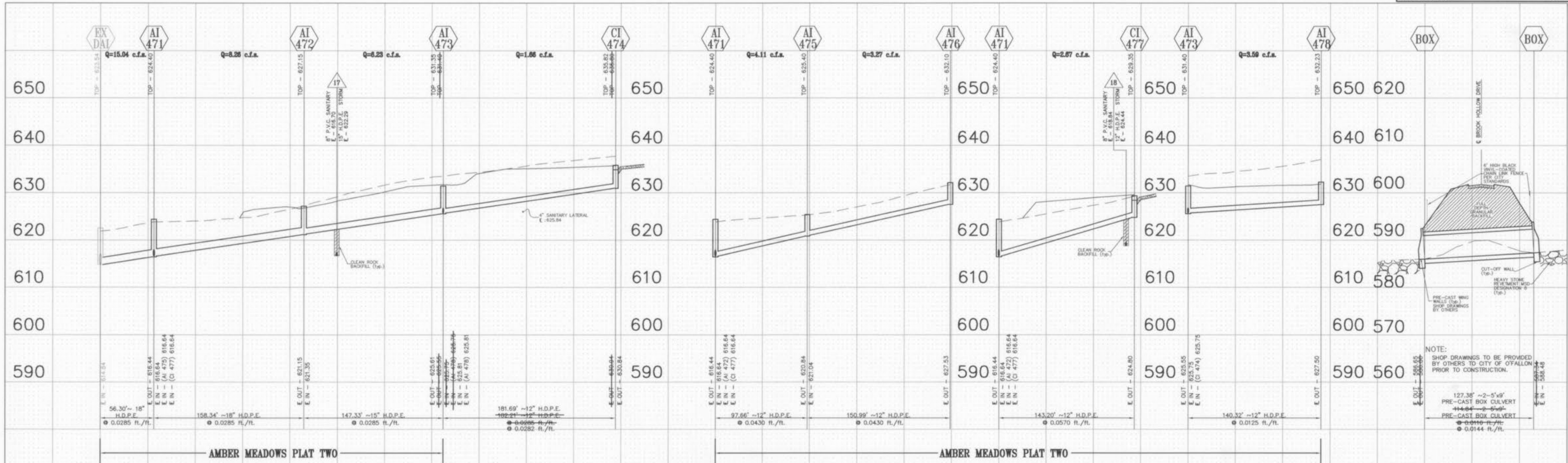
AS-BUILTS ADDED JULY, 2004

ALL SANITARY SEWER LATERAL TAKE ELEVATIONS HAVE BEEN
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SCALE:
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 HORIZONTAL = 1" = 50'

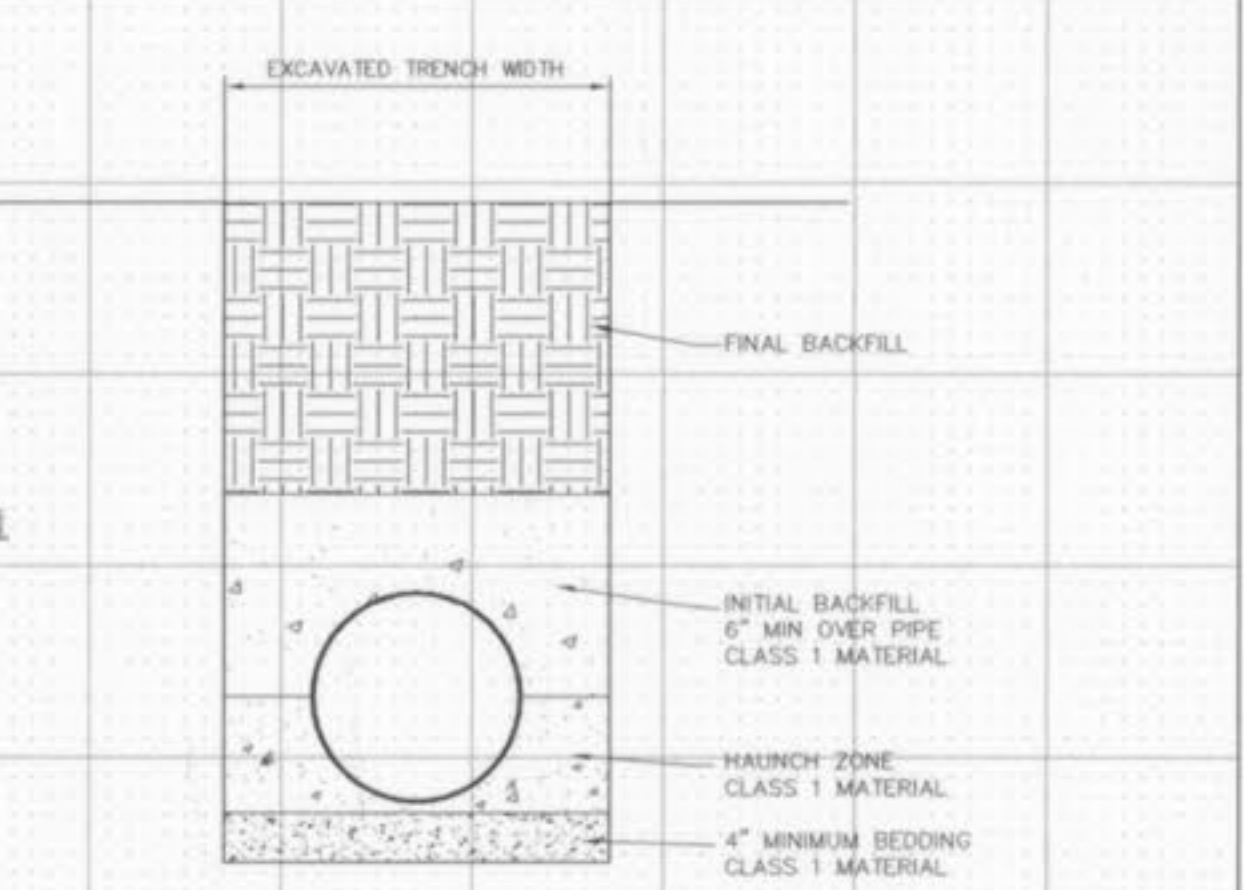
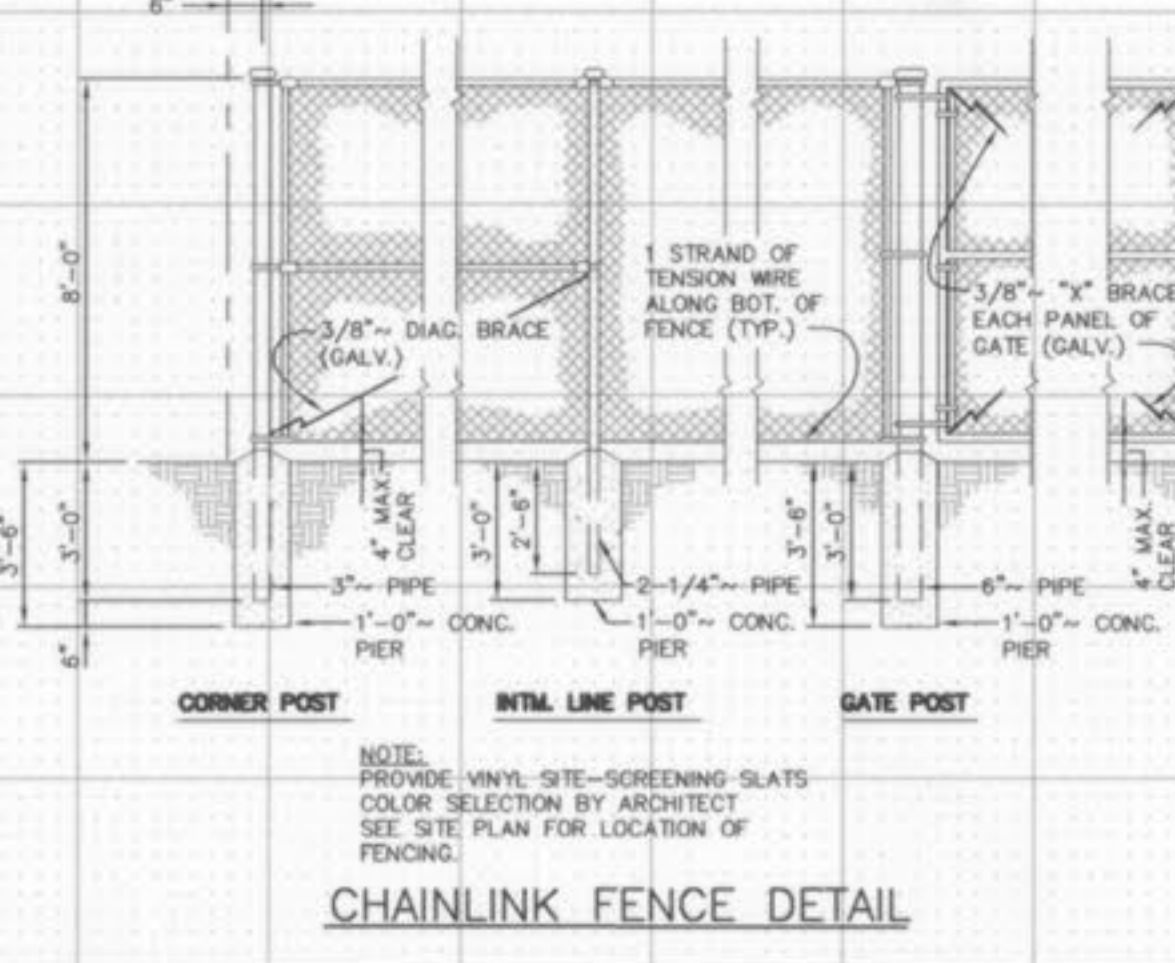
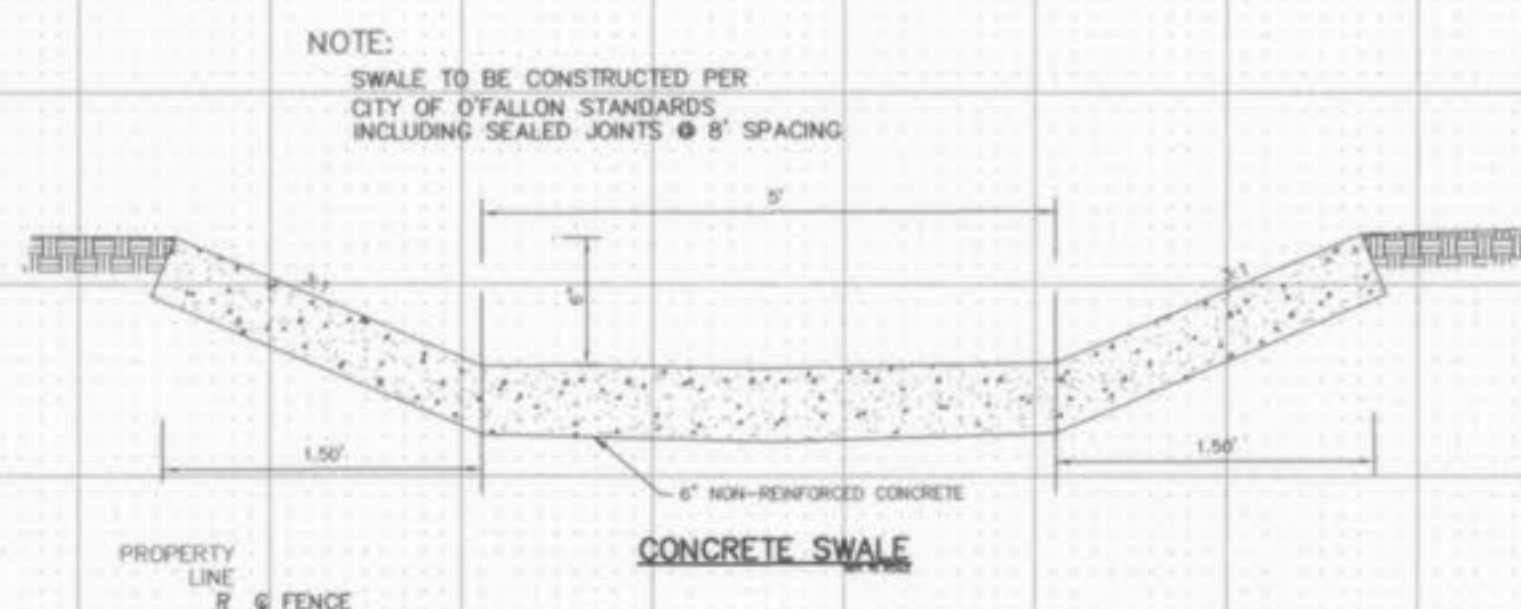
After number 11-289 App 3/2004 ABC





OVERFLOW STRUCTURE #480
 BASIN 1
 N.T.S.
 The Overflow Structure is to be a Standard Double Untrapped Street Inlet Precast Concrete (without top). See M.S.D. Detail 35. The bottom must be constructed to the correct height so that no brick will be used. A rectangular orifice 8'w. x 16' h, with a flowline of 604.70 will be used. (See Detention Calculations.)

OVERFLOW STRUCTURE #482
 BASIN 2
 N.T.S.
 The Overflow Structure is to be a Standard Double Untrapped Street Inlet Precast Concrete (without top). See M.S.D. Detail 35. The bottom must be constructed to the correct height so that no brick will be used. A rectangular orifice 18'w. x 20' h, with a flowline of 592.30 will be used. (See Detention Calculations.)



- The use of High Density Polyethylene Corrugated pipe A.S.T.M. N12 or Equal will be permitted as an acceptable alternative to reinforced concrete pipe. Pipe shall meet A.S.T.M. D-2321 and AASHTO M-234-921. Concrete flared end sections and inlet structures shall be required. Pipe must have smooth interior wall and is not to be used inside the Public Right-of-Way.
- All concrete pipe or HDPE pipe shall be installed with a ring rubber type gaskets per M.S.D. Standard Construction Specifications or Manufacturer.
- In typical conditions the minimum trench width is determined by the size of the pipe and the ability to get compaction equipment between the pipe and the trench walls. The minimum trench width should not be less than the outside diameter plus 16 inches or the pipe outside diameter times 1.25 plus 12 inches, whichever is greater. High speed trenchers may enable satisfactory installation of pipe in narrower trenches. Poor in situ soil conditions such as peat, muck, running sands, or expansive clays will require substantially wider backfill as well as deeper foundation and bedding. Trench width and foundation depth should be based on a thorough site investigation.
- Backfill in the area up to the springline should be carefully placed and compacted to achieve a minimum E value of 1,000 psi as detailed in ASTM D2321. A minimum of 12" of backfill should be placed and compacted above the crown of the pipe. It is typical for trenches to be backfilled entirely with Type I or Type II materials when under pavement.
- Flexible pipe should never be installed in a concrete cradle, as done for rigid pipe in a Class A installation. This type of installation could create concentrated forces at the ends of the cradle when the pipe has deflected.



AS-BUILTS ADDED JULY, 2004

SCALE:
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 HORIZONTAL = 1" = 50'

7
 7

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