

A SET OF AS-BUILT PLANS FOR AVONDALE HEIGHTS PLAT EIGHT

A TRACT OF LAND BEING ALL OF LOTS NO. 85, 86, 87, 88, 89, 90, 91, 92, 93,
AND PART OF LOTS 74, 75, 76, 77, 84, AND 94 OF "DARDENNE FARMS PLAT THREE"
A SUBDIVISION ACCORDING TO THE PLAT THERETO RECORDED IN PLAT BOOK 27, PAGE 77
OF THE ST. CHARLES COUNTY RECORDS, PART OF U.S. SURVEY 67,
PART OF FRACTIONAL SECTION 9, AND PART OF SECTION 4,
TOWNSHIP 46 NORTH, RANGE 3 EAST OF THE FIFTH PRINCIPAL MERIDIAN,
CITY OF O'FALLON, ST. CHARLES COUNTY, MISSOURI



DEVELOPMENT NOTES

1. Area of plat: 98.44 Acres (Total Site)
14.70 Acres (Plat Seven)
22.25 Acres (Plat Eight)
2. Proposed Zoning: R-1 (City of O'Fallon)
3. Proposed Use: Single Family Homes
4. Number of Lots Proposed: 29 Lots (Plat Seven)
43 Lots (Plat Eight)
5. The proposed height and lot setbacks are as follows:
Minimum Front Yard: 25 feet
Minimum Side Yard: 5 feet
Minimum Rear Yard: 25 feet
Minimum Lot Area: 10,000 square feet
Maximum Height of Building: 2 1/2 stories or 35 feet
6. Current Owner & Developer of Property: TJC Development, Inc.
631 Avondale Drive
St. Peters, MO 63376
7. Site is served by:
Duckett Creek Sanitary District
AmeriTEC
St. Charles Gas Company
St. Charles County Public Water District No. 2
Verizon Telephone Company
Fort Zumwalt School District
O'Fallon Fire Protection District
8. Flood Plain exists on this site per F.I.R.M. #29183 C 0239E, dated Aug. 2, 1996. See plan for limits of the 100 Year Flood Plain and the 100 Year Floodway.
9. Topographic information is per aerial topography by Walker & Associates and field topography by Bax Engineering Co. Inc.
10. Boundary information is per Bax Engineering during June, 1999.
11. All local streets will be constructed to City of O'Fallon standards. Streets will consist of 26 foot wide concrete pavement with integral rolled curb centered in a 50 foot right-of-way. Minimum radius shall be 150 feet.
12. All cul-de-sacs and bubbles will have pavement radii of 42 feet with right-of-way radii of 54 feet. Street intersections shall have a minimum rounding radius of 25 feet with pavement radii of 37 feet.
13. Minimum street grades shall be 1%.
14. A 4 foot wide concrete sidewalk shall be constructed on one side of streets where indicated.
15. All homes shall have a minimum of 2 off-street parking places with 2-car garages.
16. All utilities must be located underground.
17. The developer realizes that they will comply with current Tree Preservation Ordinance Number 1689 and provide landscaping as set forth in Article 23 of the City of O'Fallon Zoning Ordinances. Additional lighting may be required by the City of O'Fallon.
18. The following lots are susceptible to street movement: 617, 618, 619, 620, 621, 622, 623, 624, 625, 627, 628, 629, 630, 631, 632, 633, 636, 637, 638, 639, 640, 641, 642, 643, 647, 651, 652, 657, 658, 659, 660, 662, 663, 667, 668, 669, 670, 674, 676, 677, 679, 680, 685, 686, 687, 688, 689, 690, 691, 692.
19. Calculations in accordance to the Tree Preservation Ordinance: (For Entire Development)
Existing trees 15.92 acres
x 20% 3.18 acres
Saved trees 2.92 acres
Trees removed 13.15 acres
Trees Replaced = 0.26 acres x 15 Trees/acre = 4 Trees (Street Tree Requirements for 72 Lots)
1 Tree per lot & 2 per corner lot = 86 Trees
20. Detention for this development to be provided by Retention Basins "A" and "B".
21. Corner lots along Fallon Parkway (Collector Road) shall have driveway access to secondary streets only.
22. Parking will not be allowed on the north side of O'Fallon Parkway.
23. All proposed fencing requires a separate permit through the Planning Division. Fencing is not proposed in these plans.
24. All proposed signs must be approved separately through the Planning Division.
25. All existing creeks and streams will have an associated storm water easement that will be provided on the Record Plat.
26. Sidewalks, curb ramps, and accessible parking spaces shall be constructed in accordance with the current approved "American with Disabilities Act Accessibility Guidelines" (ADAAC) along with the required grades, construction materials, specifications and signage. If any conflict occurs between the above information and the plans the ADAAC guidelines shall take precedence and the contractor prior to any construction shall notify the Project Engineer.
27. Written verification of compliance with the construction plans shall be required by the Developer per section 405.120 of City of O'Fallon Code.
28. Street trees to have a minimum of 2" caliper per O'Fallon standards. Species to be selected by homeowner from O'Fallon Tree Planting Guide. Street trees to be maintained by the Home Owner's Association per subdivision C.C. & R.'s.
29. Where ground is disturbed within the 60' wide drainage easement permanent hardy vegetation shall be established.

GRADING NOTES

1. A Geotechnical Engineer shall be employed by the owner and be on site during grading operations. All soils tests shall be verified by the Geotechnical Engineer concurrent with the grading and backfilling operations.
2. The grading contractor shall perform a complete grading and compaction operation as shown on the plans, stated in these notes, or reasonably implied therefrom, all in accordance with the plans and notes as interpreted by the Geotechnical Engineer.
3. The Contractor shall notify the Soils Engineer at least two days in advance of the start of the grading operation.
4. All areas shall be allowed to drain. All low points shall be provided with temporary ditches.
5. A sediment control plan that includes monitored and maintained sediment control basins and/or straw bales should be implemented as soon as possible. No graded area is to be allowed to remain bare without being seeded and mulched. Care should be exercised to prevent soil from damaging adjacent property and siltling up existing downstream storm drainage system.
6. Debris and foundation material from any existing on-site building or structure which is scheduled to be razed for this development must be disposed of off-site.
7. All trash and debris on site, either existing or from construction, must be removed and properly disposed of off-site.
8. Soft soil in the bottom and banks of any existing or former pond sites or tributaries or on any sediment basins or traps should be removed, spread out and permitted to dry completely to be used as fill. None of this material should be placed in proposed public right-of-way locations or on any storm sewer locations.
9. Site preparation includes the clearance of all stumps, trees, bushes, shrubs, and weeds; the grubbing and removal of roots and other surface obstructions from the site; and the demolition and removal of any man-made structures. The material shall be properly disposed of off-site. Topsoil and grass in the fill areas shall be thoroughly disc'd prior to the placement of any fill. The Soils Engineer shall approve the discing operation.
10. Compaction equipment shall consist of tamping rollers, pneumatic-tired rollers, vibratory roller, or high speed impact type drum rollers acceptable to the Soils Engineer. The roller shall be designed so as to avoid the creation of a layered fill without proper blending of successive fill layers.
11. The Soils Engineer shall observe and test the placement of the fill to verify that specifications are met. A series of fill density tests will be determined on each lift of fill. Interim reports showing fill quality will be made to the Owner at regular intervals.
12. The Soils Engineer shall notify the Contractor of rejection of a lift of fill or portion thereof. The Contractor shall rework the rejected portion of fill and obtain notification from the Soils Engineer of its acceptance prior to the placement of additional fill.
13. All areas to receive fill shall be scarified to a depth of not less than 6 inches and then compacted in accordance with the specifications given below. Natural slopes steeper than 1 vertical to 5 horizontal to receive fill shall have horizontal benches, cut into the slopes before the placement of any fill. The width and height to be determined by the Soils Engineer. The fill shall be loosely placed in horizontal layers not exceeding 8 inches in thickness and compacted in accordance with the specifications given below. The Soils Engineer shall be responsible for determining the acceptability of soils placed. Any unacceptable soils placed shall be removed at the Contractor's expense.
14. The sequence of operation in the fill areas will be fill, compact, verify acceptable soil density, and repetition of the sequence. The acceptable moisture contents during the filling operation are those at which satisfactory dry densities can be obtained. The acceptable moisture contents during the filling operation in the remaining areas are from 2 to 8 percent above the optimum moisture control.
15. The surface of the fill shall be finished so that it will not impound water. If at the end of a days work it would appear that there may be rain prior to the next working days, the surface shall be finished smooth. If the surface has been finished smooth for any reason, it shall be scarified before proceeding with the placement of succeeding lifts. Fill shall not be placed on frozen ground, nor shall filling operations continue when the temperature is such as to permit the layer under placement to freeze.
16. Fill and backfill should be compacted to the criteria specified in the following table.

CATEGORY	MINIMUM PERCENT COMPACTION
Fill in building areas below footings	90%
Fill under slabs, walks, and pavement	90%
Fill other than building areas	88%
Natural subgrade	88%
Pavement subgrade	90%
Pavement base course	90%

GENERAL NOTES

1. Underground utilities have been plotted from available information and therefore their locations shall be considered approximate only. The verification of the location of all underground utilities, either shown or not shown on these plans shall be the responsibility of the contractor, and shall be located prior to any grading or construction of the improvements.
2. All manhole tops & flowlines built without elevations furnished by the Engineer will be the responsibility of the sewer contractor.
3. 8" P.V.C. sanitary sewer pipe shall meet the following standards. A.S.T.M.-D-3034 SDR-35, with wall thickness compression joint A.S.T.M.-D-3212. An appropriate rubber seal waterstop as approved by the sewer district shall be installed between P.V.C. pipe and masonry structures.
4. All filled places, including trench backfills, under buildings, proposed storm and sanitary sewer lines and/or paved areas, shall be compacted to 90% maximum density as determined by the "Modified AASHTO T-180 Compaction Test," (A.S.T.M.-D-1557). All filled places within public roadways shall be compacted to 95% of maximum density as determined by the "Standard Proctor Test AASHTO T-99, Method C" (A.S.T.M.-D-698).
5. All trench backfills under paved areas shall be granular backfill, and shall be compacted to 90% of the maximum density as determined by the "Modified AASHTO T-180 Compaction Test," (A.S.T.M.-D-1557). All other trench backfills may be earth material (free of large clods or stones). All trench backfills shall be water jetted.
6. All sanitary house connections have been designed so that the minimum vertical distance from the low point of the basement to the flow line of a sanitary sewer at the corresponding house connection is not less than the diameter of the pipe plus the vertical distance of 2 1/2 feet.
7. No area shall be cleared without the permission of the Project Engineer.
8. All P.V.C. sanitary sewer is to be SDR-35 or equal with clean 1/2" to 1" granular stone bedding uniformly graded. This bedding shall extend from 4" below the pipe to the springline of the pipe. Immediate backfill over pipe shall consist of same size "clean" or minus stone from springline of pipe to 6" above the top of pipe.
9. All soils test shall be verified by a Soils Engineer concurrent with the grading and backfilling operations.
10. Easements shall be provided for sanitary sewers, and all utilities on the Record Plat. See Record Plat for location and size of easements.
11. Maintenance and upkeep of the common ground area shall be the responsibility of the developer and/or successors.
12. A 25' building line shall be established along all Public Rights-Of-Way.
13. All water lines shall be laid at least 10 feet horizontally, from any sanitary sewer, storm sewer, or manhole. 18" vertical clearance from outside of pipe to outside of pipe shall be maintained wherever water lines must cross sanitary sewers, laterals, or storm drains. The water line shall be laid at such an elevation that the bottom of the water line is above the top of the drain or sewer. A full length of water pipe shall be centered over the sewer line to be crossed so that the joints will be equally distant from the sewer and as remote therefrom as possible. This vertical separation shall be maintained for that portion of the water line located within 10 feet horizontally, of any sewer or drain it crosses.
14. All PVC water pipe shall conform to A.S.T.M.-D-2241, SDR 21 Standard Specification for P.V.C. Pressure Pipe, 200 P.S.I. working pressure for water, with approved joint.
15. Water lines, valves, sleeves, meters, and fittings shall meet all specifications and installation requirements of Public Water Supply District No. 2 of St. Charles County.
16. All water hydrants and valves shall be ductile iron and installed in accordance with plans and details. All ductile iron pipe for water mains shall conform to A.W.W.A. Specifications C-106 and/or C-108. The ductile iron fittings shall conform to A.W.W.A. Specification CC-110. All rubber gasket joints for water ductile iron pressure pipe and fittings shall conform to A.W.W.A. Specification C-111.
17. All sanitary manholes shall be waterproofed on the exterior in accordance with Missouri Department of Natural Resources specifications 10 CSR-8.120 (7)E.
18. Brick will not be used in the construction of sanitary sewer manholes.
19. All pipes shall have positive drainage through manholes. No flat base structures are allowed.
20. The City of O'Fallon and Duckett Creek Sanitary District shall be notified 48 hours prior to construction for coordination and inspection.
21. Gas, water and other underground utilities shall not conflict with the depth or horizontal location of existing or proposed sanitary or storm sewers, including house laterals.
22. All existing site improvements disturbed, damaged or destroyed shall be repaired or replaced to closely match pre-construction conditions.
23. The contractor shall prevent all storm, surface water, mud and construction debris from entering the existing sanitary sewer system.
24. All construction and materials shall conform to the current construction standards of the City of O'Fallon and Duckett Creek Sanitary District.
25. All sanitary and storm sewer trench backfills shall be water jetted. Granular backfill will be used under pavement areas.



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30. All existing areas disturbed during construction of the off-site sanitary sewer line shall be seeded and mulched to prevent erosion.
31. All sanitary sewer laterals shall be a minimum of 4" in diameter per City of O'Fallon.
32. No flushing hydrants or water meters shall be located in driveways and/or walkways.
33. Concrete pipe for storm sewers shall be Class III, A.S.T.M. C-76 with a minimum diameter of 12" except in the R.O.W. it shall be 15".
34. The ADS N-12 pipe shall have a smooth interior wall.
35. Concrete pipe joints shall be MSD type "A" approved compression-type joints and shall conform to the requirements of the specifications for joints for circular concrete sewer and culvert pipe, using flexible, watertight, rubber-type gaskets (A.S.T.M.-C-443). Band-type gaskets depending entirely on cement for adhesion and resistance to displacement during jointing shall not be used.
36. When HDPE pipe is used, City of O'Fallon specifications or manufacturer's specifications, which ever are more stringent, shall be followed.
37. The use of High Density Polyethylene Corrugated pipe, ADS N-12 or equal will be permitted as an acceptable alternative to reinforced concrete pipe, ADS N-12 HC shall be used for all ADS pipe greater than 36". Pipe shall meet A.S.T.M.-D-2321 and A.A.S.H.T.O. M-294-291.
38. All fired end sections and inlet structures will be concrete.
39. All storm sewer pipe installed in the Public Right-of-Way shall be Reinforced concrete Class III pipe.
40. All concrete pipe or ADS N-12 pipe shall be installed with "O-Ring" Rubber type gaskets per M.S.D. standard construction specifications or manufacturer.
41. All Fire Hydrants and Water Meters shall not be located in driveways and/or sidewalks.
42. All creek crossings shall be grouted rip-rap as directed by District inspectors. (All grout shall be high slump ready-mix concrete.)
43. Existing sanitary sewer service shall not be interrupted.
44. Pre-manufactured adapters shall be used at all PVC to DIP connections. Rubber boot / Mission-type couplings will not be allowed.
45. Any permits, licenses, easements, or approvals required to work on public or private properties or roadways are the responsibility of the developer.
46. No slopes shall exceed 3(H) : 1(V).
47. Driveway locations shall not interfere with the sidewalk handicap ramps in the cul-de-sacs.
48. No parking on one side of Fallon parkway per City of O'Fallon requirements. City of O'Fallon to verify locations.
49. Developer must supply City construction inspectors with soil reports prior to or during site soil testing if reports have been prepared for the project.
50. All storm inlets must be installed with a 5/8" trash bar. to or during site soil testing if reports have been prepared for the project.
51. City approval of the construction site plans does not mean that single family and two family dwellings units can be constructed on the lots without meeting the building setbacks as required by the Zoning Code.
52. All signposts, sign backs and bracket arms shall be painted black using Carboline Rustbond penetrating sealer SG and Carboline 133 HB paint (or equivalent as approved by the City). Proposed rip rap pads will be evaluated for effectiveness at reducing erosion in the creek area, additional pad area may be required where erosion is observed.

VEGETATIVE ESTABLISHMENT
For Urban Development Sites
APPENDIX A

Seeding Rates:	
Permanent:	
Tall Fescue - 30 lbs./ac.	
Smooth Brome - 20 lbs./ac.	
Combined Fescue @ 15 lbs./ac. and Brome @ 10 lbs./ac.	
Temporary:	
Wheat or Rye - 150 lbs./ac. (3.5 lbs. per square foot)	
Oats - 120 lbs./ac. (2.75 lbs. per square foot)	
Seeding Periods:	
Fescue or Brome - March 1 to June 1	
August 1 to October 1	
Wheat or Rye - March 15 to November 1	
Oats - March 15 to September 15	
Mulch Rates: 100 lbs. per 1,000 sq. feet (4,356 lbs. per acre)	
Fertilizer Rates:	
Nitrogen 30 lbs./ac.	
Phosphate 30 lbs./ac.	
Potassium 30 lbs./ac.	
Lime 600 lbs./ac. ENM*	

* ENM = effective neutralizing material as per State evaluation of quarried rock.

REFERENCE BENCHMARK

R.M. #74 - ELEV.=493.07 (U.S.G.S. DATUM)
CHISELED SQUARE ON TOP OF EAST CONCRETE HEADWALL
OF BRIDGE HILLS ROAD BRIDGE OVER TRIBUTARY NO. 2
(APPROXIMATELY 500 FEET SOUTH OF EISENHOWER DRIVE)

SITE BENCHMARK

ELEV.=507.68 (U.S.G.S. DATUM)
OLD CROSS CL-CL CHRISTINA MARIE DRIVE AND
JACQUELINE CIRCLE KENSINGTON PLACE SUBDIVISION

SEWER MEASUREMENTS

THE EXISTING SEWER LENGTHS, SIZES, FLOWLINES, DEPTHS OF STRUCTURES AND SEWERS AND LOCATIONS WITH RESPECT TO EXISTING OR PROPOSED EASEMENTS HAVE BEEN MEASURED. THE RESULTS OF THOSE MEASUREMENTS ARE SHOWN ON THIS SET OF FINAL MEASUREMENT PLANS.

ALL PUBLIC SEWERS ARE LOCATED WITHIN DESIGNATED EXISTING OR PROPOSED EASEMENTS EXCEPT AS FOLLOWS:

SIGNED: [Signature]
P.E./A.S.

DATE: 10-05-03

ASBUILTS NOTE:
ALL DISTANCE AND SLOPE CALCULATIONS ARE FROM CENTER OF STRUCTURE TO CENTER OF STRUCTURE.

AVONDALE HEIGHTS-PLAT EIGHT

TJC DEVELOPMENT, INC.
631 AVONDALE DRIVE
ST. PETERS, MO 63376
(636) 978-9200

PREPARED FOR:

DISCLAIMER OF RESPONSIBILITY
I hereby agree that the documents intended to be authorized by my seal are limited to the site shown, and I hereby disclaim any responsibility for all other Drawings, Specifications, Estimates, Reports or other documents or instruments relating to or intended to be used for any part or parts of the architectural or engineering project or survey.

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REVISIONS

NO.	DATE	DESCRIPTION

BAX

**ENGINEERING
PLANNING
SURVEYING**

1502 South Cloverleaf Drive
St. Peters, MO. 63376-6445
636-928-5552
FAX 928-1718

FEBRUARY 2003
DATE

95-7230S
PROJECT NUMBER

1 OF 8
SHEET OF

7230SASB.DWG.
FILE NAME

BCC
DRAWN

DESIGNED CHECKED

Avondale Heights Plat 8 App. 10/11/03 BCC



BLOW-OFF HYDRANTS AND WATER METERS SHALL NOT BE LOCATED IN ANY PAVEMENT OR HARD-SURFACED AREAS INCLUDING, BUT NOT LIMITED TO, DRIVEWAYS, SIDEWALKS, WALKWAYS, AND STREETS. SINCE THE LOCATION OF ALL SUCH AREAS IS NOT SHOWN ON THIS PLAN ALL COSTS TO RELOCATE ANY BLOW-OFF HYDRANTS AND WATER METERS FROM ANY PAVEMENT OR HARD-SURFACED AREAS SHALL BE BORNE BY THE DEVELOPER OR THE BUILDER.

AS-BUILTS ADDED JANUARY 2003

UNDERGROUND UTILITIES HAVE BEEN PLOTTED FROM AVAILABLE INFORMATION AND THEREFORE THEIR LOCATIONS SHALL BE CONSIDERED APPROXIMATE ONLY. THE VERIFICATION OF THE LOCATION OF ALL UNDERGROUND UTILITIES, EITHER SHOWN OR NOT SHOWN ON THESE PLANS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR, AND SHALL BE LOCATED PRIOR TO ANY GRADING OR CONSTRUCTION OF THE IMPROVEMENTS.

Approved: [Signature] Date: 1/13/03

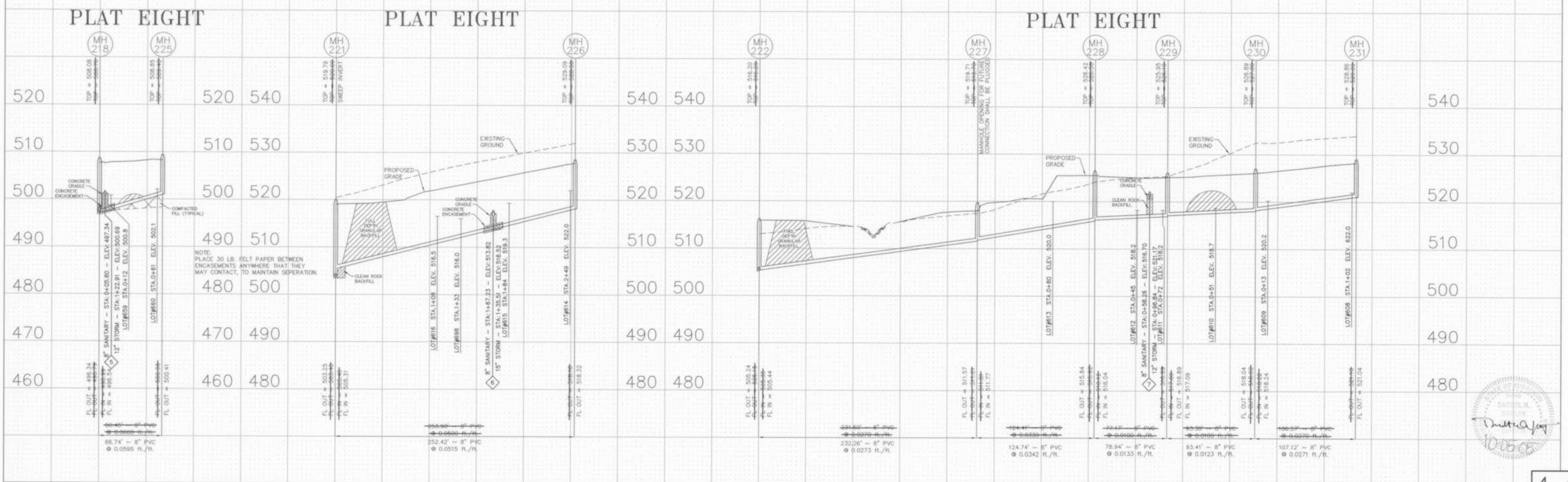
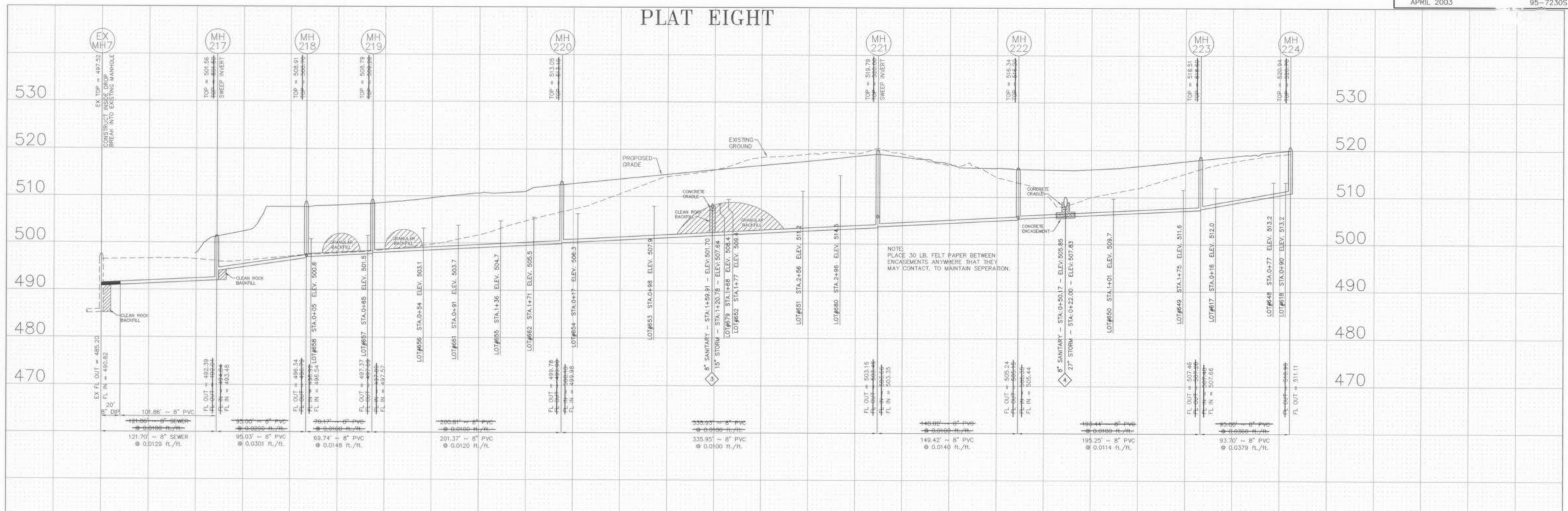


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Avondale Heights Plat 8 Jan 10/11/03 442



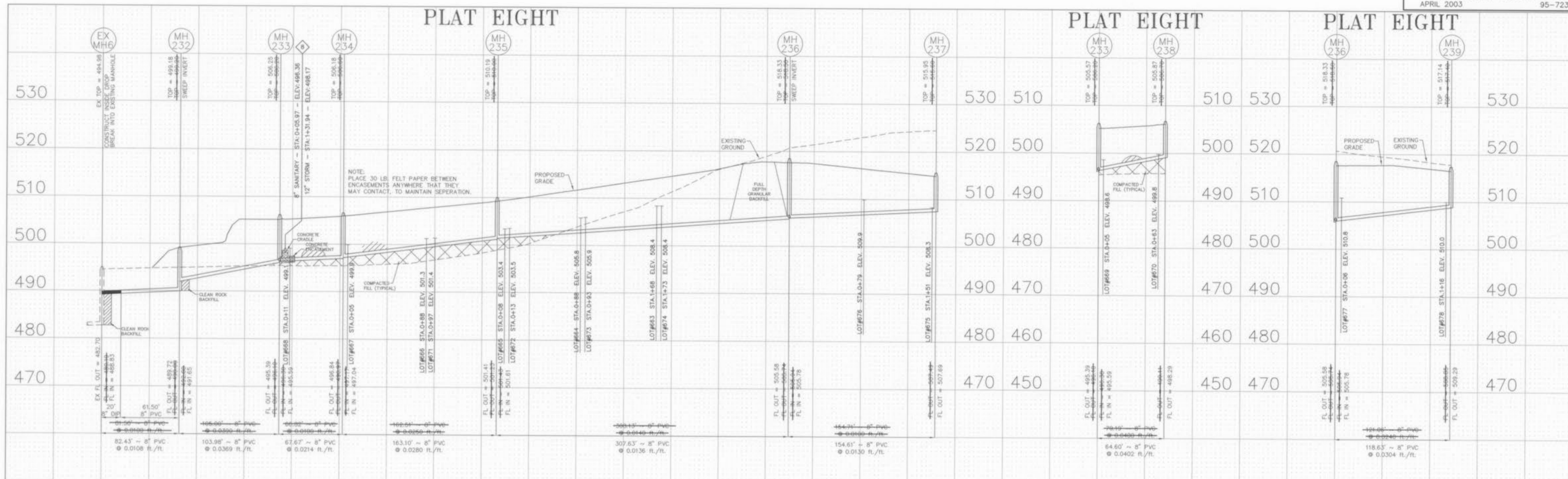
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AS-BUILTS ADDED APRIL 2003

SCALE:
 VERTICAL = 1:10
 HORIZONTAL = 1:50

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Avondale Heights Plat 8 April 10/11/03 48'

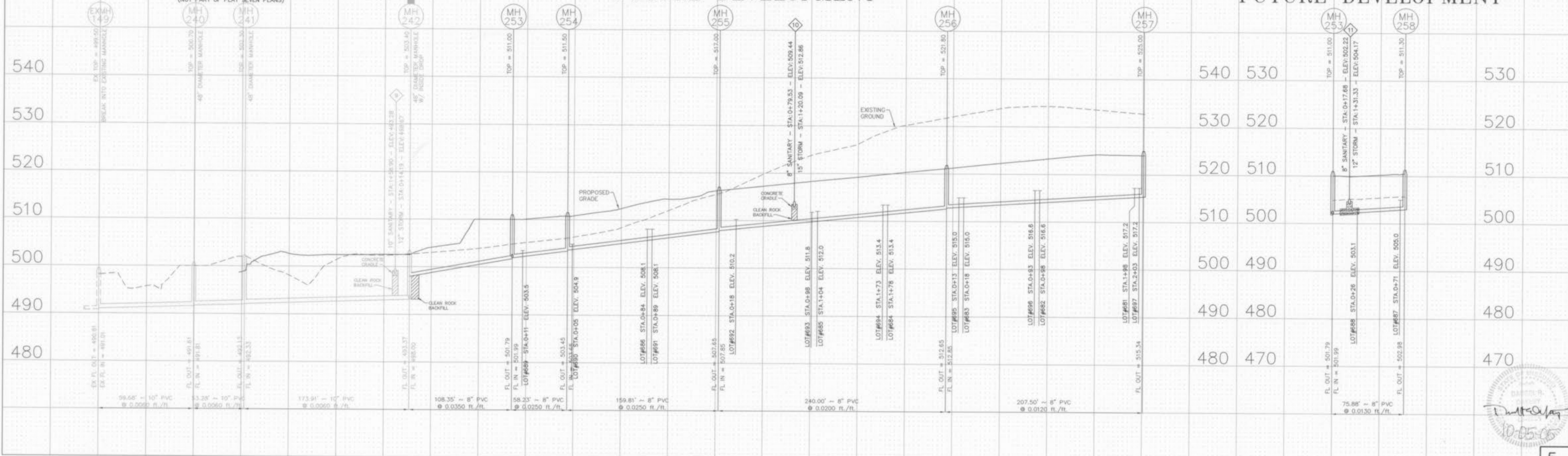


**AVONDALE HEIGHTS
 TRUNKLINE PLANS**
 (NOT PART OF PLAT SEVEN PLANS)



FUTURE DEVELOPMENT

FUTURE DEVELOPMENT



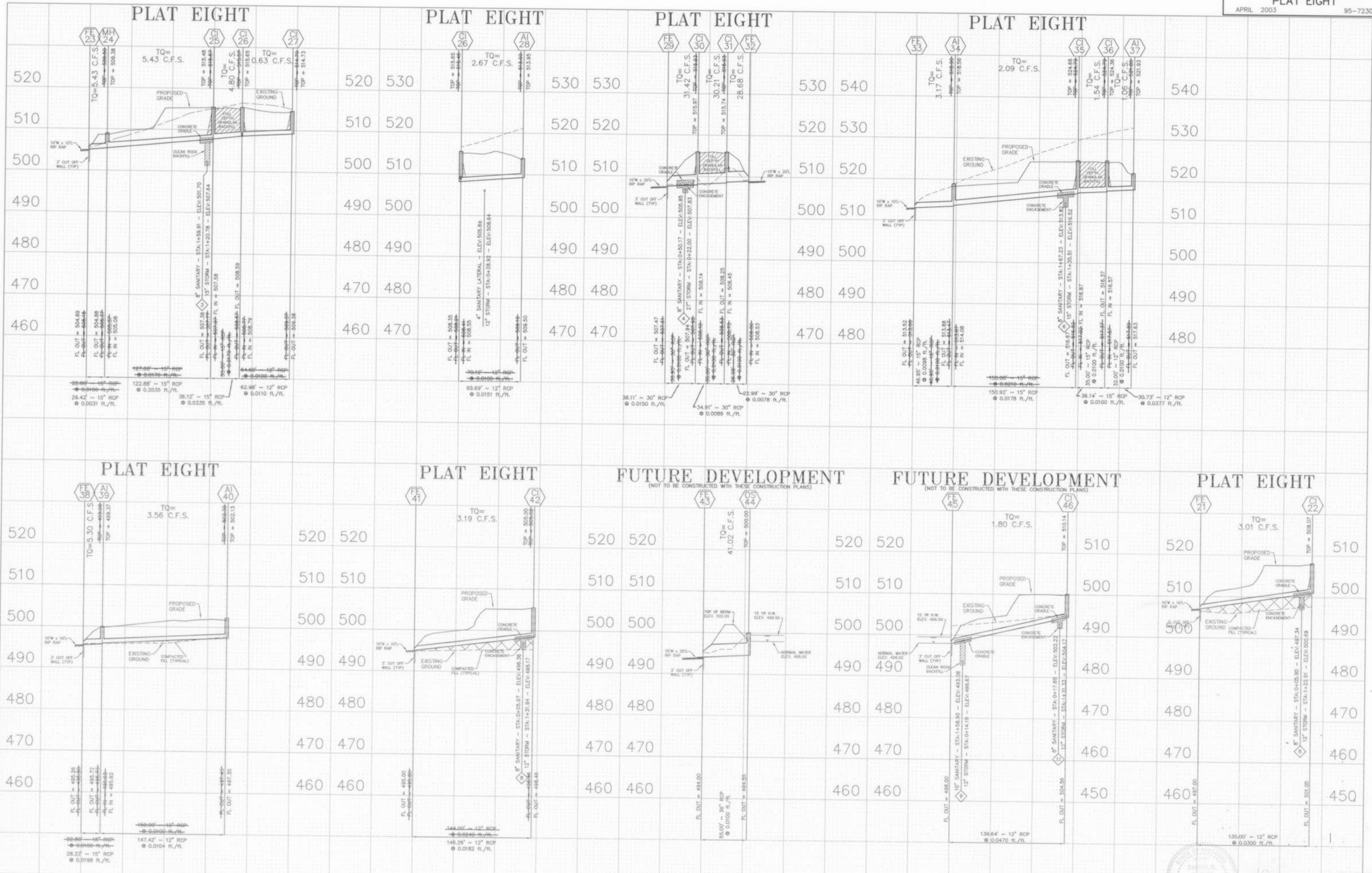
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AS-BUILTS ADDED APRIL 2003

SCALE:
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 HORIZONTAL = 1:50

Avondale Heights Plat 8 App 10/1/03 MRC

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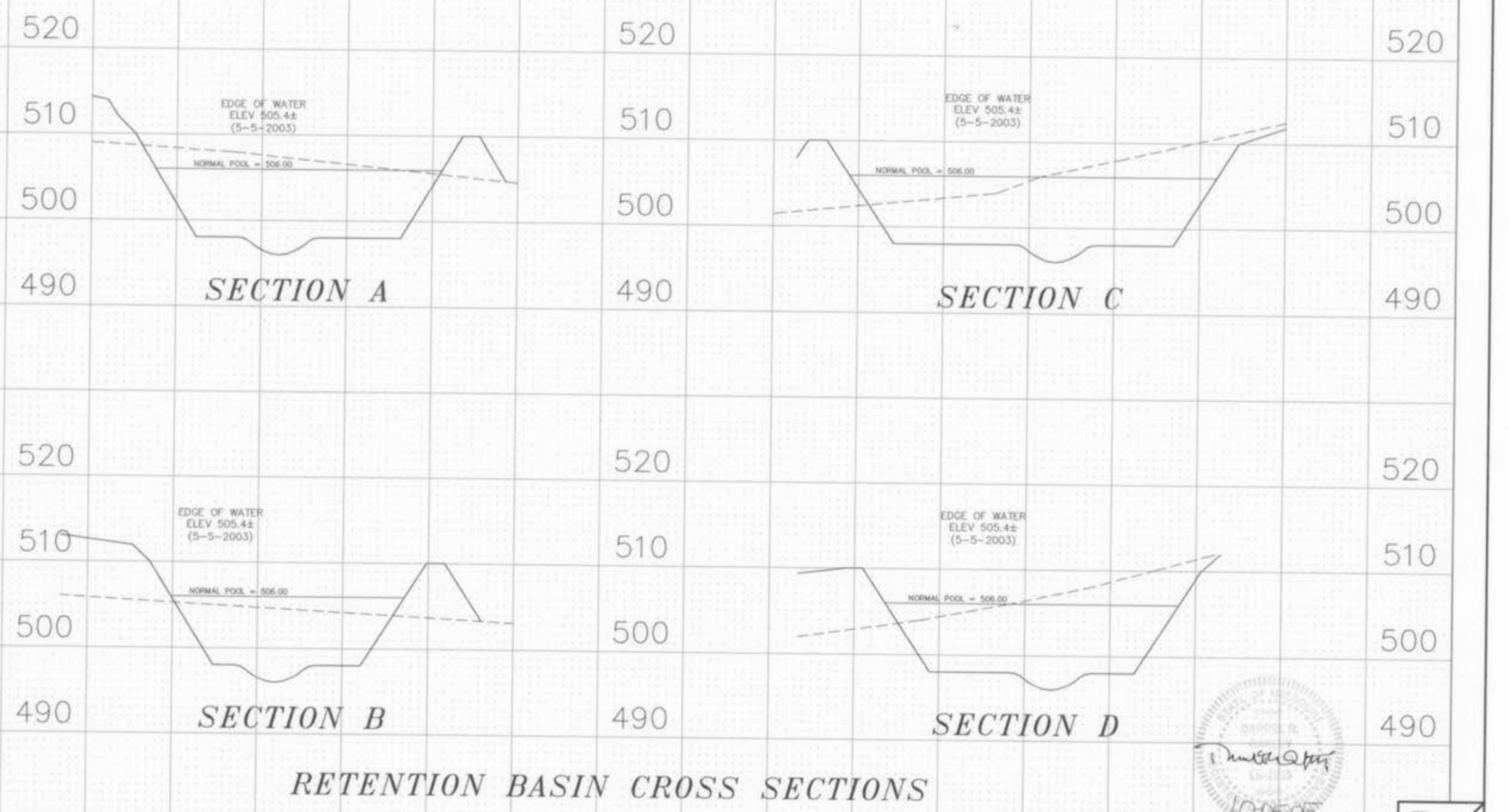
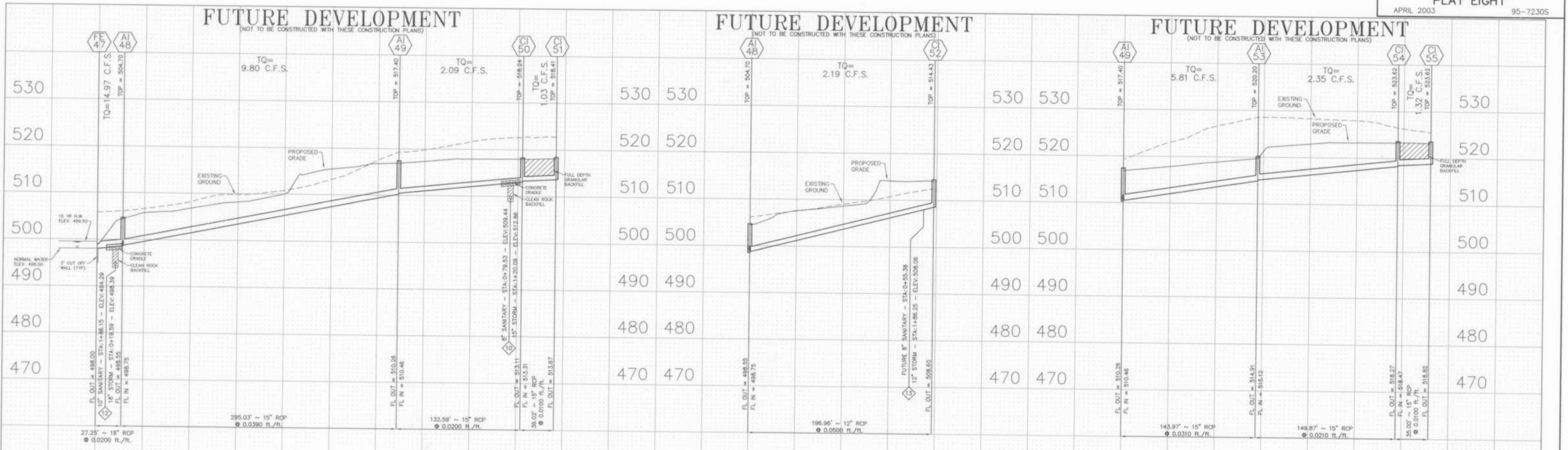
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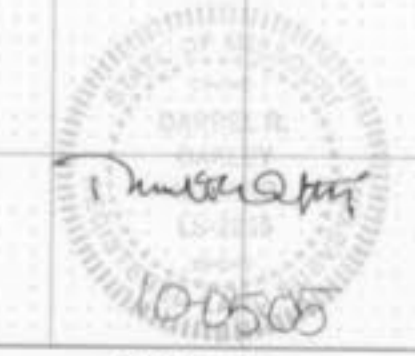
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UNDERGROUND UTILITIES HAVE BEEN PLOTTED FROM AVAILABLE INFORMATION AND THEREFORE THEIR LOCATIONS SHALL BE CONSIDERED APPROXIMATE ONLY. THE VERIFICATION OF THE LOCATION OF ALL UNDERGROUND UTILITIES, EITHER SHOWN OR NOT SHOWN ON THESE PLANS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR, AND SHALL BE LOCATED PRIOR TO ANY GRADING OR CONSTRUCTION OF THE IMPROVEMENTS.

AS-BUILTS ADDED APRIL 2003



SCALE:
 VERTICAL = 1:10
 HORIZONTAL = 1:50

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Avondale Heights plat 8 April 10/03/03

UPP STR	LOW STR	L	DIA	UPPER FL LN	LOWER FL LN	PS	UPPER ST EL	DEPTH HY GR	UPPER HY EL	LOWER HY EL	HYDR GRADE	FR HEAD	VEL	VEL HEAD	JUNC LOSS	TURN LOSS	STR GRADE	INL CAP	DR AREA	PI	Q	TQ	PIPE CAP	REMARKS	
C13	MH2	112.80	12	502.71	498.42	3.80	508.06	8.00	500.06	499.42	0.0420	0.47	2.93	0.13	0.17	0.00	LP	4.00	0.00	0.00	0.00	2.30	6.95	1	
MH2	FE1	22.00	12	498.22	498.00	1.00	501.80	2.65	499.15	499.00	0.0420	0.09	2.93	0.13	0.00	0.06		0.00	0.00	0.00	0.00	2.30	3.56	2	HW=499.00
DSS	FE4	74.00	36	501.24	500.50	1.00	508.00	2.52	505.48	503.50	0.0770	0.57	8.27	1.06	1.41	0.00		0.00	0.00	0.00	0.00	58.47	66.70	3	HW=503.50
C19	C18	35.00	15	508.67	508.32	1.00	513.05	3.46	509.59	509.57	0.0030	0.01	0.88	0.01	0.01	0.00	1.5	2.11	0.00	0.00	0.00	1.08	6.46	4	
MH7	MH7	155.08	15	508.12	506.57	1.00	513.05	5.02	508.03	507.82	0.0100	0.16	1.68	0.04	0.05	0.00	1.5	2.11	0.00	0.00	0.00	2.06	6.46	5	
MH7	FE6	37.28	15	506.37	506.00	0.99	511.50	3.94	507.56	507.50	0.0100	0.04	1.68	0.04	0.00	0.02		0.00	0.00	0.00	0.00	2.06	6.44	6	HW=507.50
C120	C114	35.00	15	515.70	515.35	1.00	520.21	3.50	516.71	516.60	0.0110	0.04	1.76	0.05	0.07	0.00	2	1.93	0.00	0.00	0.00	2.16	6.46	7	
EX119	A118	33.00	30	521.30	520.97	1.00	529.88	3.74	526.14	524.33	0.1020	0.34	8.43	1.10	1.47	0.00	2.0	1.93	0.00	0.00	0.00	41.38	41.02	8	
A118	C117	185.15	30	520.77	518.92	1.00	527.00	2.67	524.33	521.70	0.1080	2.01	8.70	1.17	0.14	0.48	4S	11.00	0.00	0.00	0.00	42.70	41.00	9	
C117	C116	35.99	30	518.72	518.36	1.00	526.21	4.51	521.70	520.86	0.1120	0.40	8.86	1.22	0.09	0.35	3	1.57	0.00	0.00	0.00	43.47	41.02	10	
C116	A115	153.68	30	518.16	512.53	3.66	525.96	8.87	517.09	515.03	0.1170	1.80	9.06	1.27	0.10	0.16	3	1.57	0.00	0.00	0.00	44.45	78.51	11	
A115	C114	74.68	36	512.33	511.58	1.00	518.00	3.03	514.97	514.62	0.0470	0.35	6.50	0.66	-0.16	0.16	4S	11.00	0.00	0.00	0.00	45.95	66.85	12	
C114	C113	66.13	36	511.38	510.72	1.00	520.21	5.59	514.62	513.73	0.0540	0.36	6.94	0.75	0.17	0.36	2	1.93	0.00	0.00	0.00	49.03	66.63	13	
C113	C112	35.09	36	510.52	510.17	1.00	519.38	5.65	513.73	513.17	0.0550	0.19	6.98	0.76	0.02	0.25	2	1.93	0.00	0.00	0.00	49.32	66.61	14	
C112	A111	127.59	36	509.97	507.04	2.30	519.33	8.15	511.18	510.36	0.0550	0.71	7.02	0.76	0.01	0.10	2	1.93	0.00	0.00	0.00	49.61	101.07	15	
A111	FE10	139.64	36	506.84	506.00	0.60	513.10	2.74	510.36	509.00	0.0570	0.80	7.13	0.79	0.06	0.50	4S	11.00	0.00	0.00	0.00	50.43	51.73	16	HW=509.00
C122	FE21	135.00	12	501.05	497.00	3.00	508.07	8.80	499.27	498.00	0.0710	0.96	3.83	0.23	0.31	0.00	LP	4.00	0.00	0.00	0.00	3.01	6.17	17	HW=498.00
A128	C126	56.59	12	509.16	508.59	1.01	513.90	3.75	510.15	509.59	0.0560	0.32	3.40	0.18	0.24	0.00	4S	11.00	0.00	0.00	0.00	2.67	3.58	18	
C127	C126	77.98	12	509.37	508.59	1.00	514.79	5.17	509.62	509.59	0.0030	0.02	0.80	0.01	0.01	0.00		0.00	0.00	0.00	0.00	0.63	3.56	19	
C126	C125	35.00	15	508.39	507.79	1.71	515.88	6.37	509.51	509.04	0.0550	0.19	3.91	0.24	0.18	0.10		0.00	0.00	0.00	0.00	4.80	8.46	20	
C125	MH24	127.59	15	507.59	505.42	1.70	515.88	8.18	507.70	506.67	0.0700	0.89	4.41	0.30	0.12	0.02		0.00	0.00	0.00	0.00	5.41	8.42	21	
MH24	FE23	22.00	15	505.22	505.00	1.00	509.50	2.97	506.53	506.25	0.0700	0.15	4.41	0.30	0.00	0.13		0.00	0.00	0.00	0.00	5.41	6.46	22	HW=506.25
FE23	C131	26.98	30	509.00	508.73	1.00	512.07	0.00	512.07	511.23	0.0490	0.13	5.84	0.53	0.71	0.00	LP	4.00	0.00	0.00	0.00	28.68	41.03	23	
C131	C130	35.00	30	508.53	508.18	1.00	515.93	4.81	511.12	510.68	0.0540	0.19	6.15	0.59	0.12	0.13	LP	4.00	0.00	0.00	0.00	30.21	41.02	24	
C130	FE29	26.95	30	507.98	507.61	1.37	515.93	5.35	510.58	510.11	0.0590	0.16	6.40	0.64	0.10	0.21	LP	4.00	0.00	0.00	0.00	31.42	48.06	25	HW=510.11
A137	C136	32.00	12	517.89	517.57	1.00	521.80	3.16	518.64	518.57	0.0090	0.03	1.25	0.03	0.04	0.00	4S	11.00	0.00	0.00	0.00	1.06	3.56	26	
C136	C135	35.00	15	517.37	517.02	1.00	524.79	6.49	518.30	518.27	0.0060	0.02	1.25	0.02	0.00	0.01	4	1.21	0.00	0.00	0.00	1.54	6.46	27	
C135	A134	150.08	15	516.82	513.67	2.10	524.79	9.66	515.13	514.92	0.0100	0.16	1.70	0.05	0.05	0.00	4	1.21	0.00	0.00	0.00	2.09	9.36	28	
A134	FE33	46.95	15	513.47	513.00	1.00	518.90	4.43	514.47	514.25	0.0240	0.11	2.58	0.10	0.09	0.02	2S	5.50	0.00	0.00	0.00	3.17	6.46	29	HW=514.25
A140	A139	150.00	12	497.43	495.93	1.00	502.30	3.37	498.93	497.00	0.1000	1.50	4.53	0.32	0.43	0.00	3S	8.25	0.00	0.00	0.00	3.56	3.56	30	
A139	FE38	22.89	15	495.73	495.50	1.00	499.90	2.90	497.00	496.75	0.0670	0.15	4.32	0.29	0.10	0.00	3S	8.25	0.00	0.00	0.00	5.30	6.48	31	HW=496.75
C142	FE41	144.00	12	498.46	495.00	2.40	505.52	8.02	497.50	496.00	0.0800	1.15	4.06	0.26	0.35	0.00	LP	4.00	0.00	0.00	0.00	3.19	5.52	32	HW=496.00
DS44	FE43	55.00	36	494.55	494.00	1.00	500.00	2.10	497.90	497.00	0.0380	0.21	5.80	0.52	0.69	0.00		0.00	0.00	0.00	0.00	41.02	66.70	33	HW=497.00
C146	FE45	139.64	12	504.56	498.00	4.70	510.14	10.17	499.97	499.50	0.0260	0.36	2.29	0.08	0.11	0.00	LP	4.00	0.00	0.00	0.00	1.80	7.72	34	HW=499.50
C155	C154	35.00	15	518.82	518.47	1.00	523.62	3.86	519.76	519.72	0.0040	0.01	1.08	0.02	0.03	0.00	2.0	1.93	0.00	0.00	0.00	1.32	6.46	35	
C154	A153	149.87	15	518.27	515.12	2.10	523.62	6.98	516.64	516.37	0.0130	0.20	1.91	0.06	0.07	0.00	2.0	1.93	0.00	0.00	0.00	2.35	9.37	36	
A153	A149	143.97	15	514.92	510.46	3.10	520.20	6.86	513.34	511.71	0.0810	1.16	4.73	0.35	0.43	0.04	3S	8.25	0.00	0.00	0.00	5.81	11.37	37	
C152	A148	196.96	12	508.60	498.75	5.00	514.43	12.52	501.91	501.01	0.0380	0.74	2.79	0.12	0.16	0.00	LP	4.00	0.00	0.00	0.00	2.19	7.97	38	
C151	C150	36.02	15	513.67	513.31	1.00	518.41	3.83	514.58	514.56	0.0030	0.01	0.84	0.01	0.01	0.00	2.0	1.93	0.00	0.00	0.00	1.03	6.46	39	
C150	A149	132.59	15	513.11	510.46	2.00	518.24	6.33	511.91	511.71	0.0100	0.14	1.70	0.05	0.06	0.00	2.0	1.93	0.00	0.00	0.00	2.09	9.13	40	
A149	A148	295.03	15	510.26	498.75	3.90	517.40	8.54	508.86	501.01	0.0230	6.79	7.99	0.99	1.03	0.03	3S	8.25	0.00	0.00	0.00	9.80	12.76	41	
A148	FE47	27.25	18	498.55	498.00	2.02	504.70	3.69	501.01	499.50	0.0200	0.55	8.47	1.11	0.59	0.37	3S	8.25	0.00	0.00	0.00	14.97	14.92	42	HW=499.50
C158	MH57	105.10	12	521.47	520.42	1.00	525.21	3.24	521.97	521.42	0.0370	0.39	2.75	0.12	0.16	0.00	LP	4.00	0.82	2.64	2.16	2.16	3.56	43	
MH57	FE56	22.00	12	520.22	520.00	1.00	524.30	3.16	521.14	521.00	0.0370	0.08	2.75	0.12	0.00	0.06		0.00	0.00	0.00	0.00	2.16	3.56	44	HW=521.00
A160	FE59	343.94	12	497.94	494.50	1.00	503																		