## GRADING NOTES

- A Geotechnical Engineer shall be employed by the awner and be on site during grading operations. All soils tests shall be verified by the Geotechnical Engineer concurrent with the grading and backfilling operations.
- 2. The grading contractor shall perform a complete grading and compaction operation as shawn on the plans, stated in these notes, or reasonably implied there from, all in accordance with the plans and notes as interpreted by the Geotechnical Engineer.
- The Contractar shall notify the Soils Engineer at least two days in advance of the stort of the grading operation.
- 4. All areas shall be allowed to drain. All low points shall be provided with temporary ditches.
- 5. All filled places, including trench backfills, under buildings, proposed storm and sanitary sewer lines, proposed roads ond/or paved areas, shall be compacted to 90% of maximum density as determined by the "Modified AASHTO T-180 Compaction Test," (A.S.T.M.-D-1557), or 95% of maximum density as determined by the Stondard Proctor Test AASHTO -99. All filled places within public roadways shall be compacted from the bottom of the fill up to 90% maximum density os determined by the Modified AASHTO T-180 Compaction Test or 95% of maximum density as determined by the Standard Practor Test AASHTO T-99, Method "C" (A.S.T.M.-D-698). All test shall be verified by a soils engineer concurrent with grading and backfilling operations. Ensure the moisture content of the soil in fill areas is to correspond to the compactive effort as defined by the Standard or Modified Proctor Test. Optimum moisture content shall be determined using the same test that was used for compaction. Soil compaction curves shall be submitted to the City of O'Fallon prior to the placement of fill. Proof rolling may be required to verify sail stability at the discretion of the City Of O'Fallon.
- 6. A sediment control plan that includes monitored ond maintoined sediment control basins and/or straw boles should be implemented as soon as possible. No graded area is to be allowed to remain bare without being seeded and mulched. Care should be exercised to prevent soil from damaging adjacent property and silting up existing downstream storm drainage systems. All erosion control systems shall be inspected and necessary corrections made within 24 hours of any rain storm resulting in 1/2 inch of rain or more.
- Debris and foundation material from any existing on-site building ar structure which is scheduled to be razed for this development must be disposed of off-site.
- 8. All trash and debris on site, either existing or from construction, must be removed and properly disposed af off-site.
- 9. Soft soil in the bottom and banks of any existing or former pond sites or tributaries ar on any sediment basins or traps should be remaved, spread out and permitted to dry sufficiently to be used as fill. Name of this material should be placed in proposed public right—of—way locations or on any storm sewer locations.
- 10. Site preparation includes the clearance of all stumps, trees, bushes, shrubs, and weeds; the grubbing and remaval of roots and other surface abstructions from the site; and the demolitian and remaval of ony man-made structures. The material shall be properly disposed af off-site. Topsoil and grass in the fill areas shall be thoroughly disced prior to the placement of any fill. The Soils Engineer shall approve the discing operation.
- Compaction equipment shall cansist of tamping rollers, pneumatic-tired rollers, vibratory roller, or high speed impact type drum rollers acceptable to the Soils Engineer. The roller shall be designed so as to avoid the creatian of a layered fill without proper blending of successive fill layers.
- 12. The Soils Engineer shall observe and test the placement of the fill to verify that specifications are met. A series of fill density tests will be determined on each lift of fill. Interim reports showing fill quality will be made to the Owner at regular intervals.
- 13. The Soils Engineer shall notify the Contractor of rejection of a lift of fill ar portion thereof. The Contractor sholl rework the rejected portion of fill and obtain notification from the Soils Engineer of its acceptance prior to the placement of additional fill.
- 14. All areas ta receive fill shall be scarified to a depth of nat less than 6 inches and then compacted in accordance with the specifications given below. Natural slopes steeper than 1 vertical to 5 horizontal to receive fill shall have horizontal benches cut into the slopes before the placement of any fill. The width and height to be determined by the Soils Engineer. The fill shall be loosely placed in harizontal layers not exceeding 8 inches in thickness and compacted in accordance with the specifications given below. The Sails Engineer shall be responsible for determining the acceptability of sails placed. Any unacceptable soils placed shall be removed at the Contractor's expense.
- 15. The sequence of operation in the fill areas will be fill, compoct, verify acceptable soil density, and repetition of the sequence. The occeptable moisture contents during the filling operation are those at which satisfactory dry densities can be abtained.
- 16. The surface of the fill sholl be finished so that it will not impound water. If at the end of a days work it would appear that there may be rain prior to the next working day, the surface shall be finished smooth. If the surface has been finished smooth for any reason, it shall be scarified before praceeding with the placement of succeeding lifts. Fill shall not be placed on frozen ground, nor shall filling operations cantinue when the temperature is such as to permit the layer under placement to freeze.
- Developer must supply City construction inspectors with soil reports prior to or during site soil testing.

18. Fill and backfill should be compacted to the criteria specified in the fallowing table:
 MINIMUM
 CATEGORY
 Fill in building areas below footings
 90%

Fill in building areas belaw footings90%Fill under slabs, walks, and pavement90%Fill other than building areas88%Natural subgrade88%Pavement subgrade90%Pavement base course90%

## GENERAL NOTES

- . Underground utilities have been plotted fram available information and therefore their locations shall be considered approximate only. The verification of the location of all underground utilities, either shown or not shown on these plans shall be the responsibility of the contractor, and shall be located prior to any grading or construction of the improvements.
- All manhole tops & flowlines built without elevations furnished by the Engineer will be the responsibility of the sewer contractor.
- 3. Sanitary sewer pipe shall meet the fallowing standords. A.S.T.M.—D— 3034 SDR—35, with wall thickness compression joint A.S.T.M.—D—3212. An appropriate rubber seal waterstop as approved by the City of O'Fallon sewer district shall be installed between P.V.C. pipe and masonry structures.
- 4. All trench backfills under paved areas shall be granular backfill, and shall be Modified compacted to 90% of the maximum density as determined by the "AASHTO T-180 Compaction Test," (A.S.T.M.-D-1557). All other trench backfills may be earth material (free of large clods or stones). All trench backfills shall be water jetted.
- 5. All sanitary house connections have been designed so that the minimum vertical distance from the low point of the basement to the flow line of a sanitary sever at the corresponding house connection is not less than the diameter of the pipe plus the vertical distance of 2 1/2 feet.
- 6. No area shall be cleared without the permission of the Project Engineer.

## GENERAL NOTES

- 7. All P.V.C. sanitary sewer is to be SDR-35 or equal with clean 1/2" to stone bedding uniformly graded. This bedding shall extend from 4" belo to the springline of the pipe. Immediate backfill over pipe shall consist size "clean" or minus stone fram springline of pipe to 12" above the to
- All soils test shall be verified by a Soils Engineer concurrent with the g backfilling operations.
- Eosements shall be provided for sanitary sewers, and all utilities on the Plat. See Record Plat for location and size of easements.
- Mointenance and upkeep of the common ground area shall be the resp the developer and/or successors.
- 11. All water lines shall be laid at least 10 feet horizontally, from any sani storm sewer, or manhole. 18" vertical clearance from outside of pipe of pipe shall be maintained wherever water lines must crass sanitary s laterals, or storm drains the water line shall be loid at such an elevat the bottom of the water line is obove the top of the drain or sewer. length of water pipe shall be centered over the sewer line to be cross the joints will be equally distant from the sewer and as remote theref possible. This vertical separation shall be maintoined for that portion water line located within 10 feet harizontally, of any sewer or drain it
- 12. All water mains should be 2 inches in diameter, or larger. The pipe sh Minimum Pressure Rating (PR) of 200 psi or SDR-21 for 2" thru 10" 13.5 Class 130 for 12" and larger pipe with blue stripe to identify as All water mains of PVC material shall be certified by NSF and listed is 61. NSF stands for NSF International which is on agency that certifies such as pipe, valves, etc. for use in potable water systems among ot Standards 61 is the (ANSI/NSF Standard 61) is a listing of certified or system components. The Missouri DNR requires that product which can with drinking water be listed in NSF Standard 61. If the pipe is NSF have a stamp on the pipe that says "NSF-pw".
- 13. Disinfection and Bacteriological testing shall be per A.W.W.A. C 651-86
- 14. Pressure testing shall be performed immediately following disinfection, be pumped to a pressure (at the lowest point in the project) of 150 where the working pressure is higher than 150 PSI as determined by such cases, the test pressure shall be as specified by the District an tests shall be canducted. The first test shall be with the fire hydror open and be to 150 PSI. The second test shall be with the fire hyd valves closed and be to the higher pressure as directed by the Distric equipment and pressure gauges shall be provide by the contractor. the test pressure, the piping shall be left closed for a period of two the end of this time the pressure drap shall not exceed 2 PSI. In a pressure appears, in the judgment of the District's representative, to drop, the test shall be cantinued for another two (2) haurs and if an accur, the test shall be considered a failure. If the pressure test fo will be required to find and correct the source of the leakage. If this drainage of the pipeline, when the leakage is corrected, the piping mu disinfected and the pressure tested again until satisfactory results are
- 15. Water lines, valves, sleeves, meters, and fittings shall meet all specific installation requirements of The City of O'Fallon.
- 16. All water hydrants and valves shall be ductile iron and installed in acc with plans and details. All ductile iron pipe for water mains shall cor A.W.W.A. Specifications C-108 and/ar C-108. The ductile iran fittings conform to A.W.W.A. Specification CC-110. All rubber gasket joints for ductile iran pressure pipe and fittings shall conform to A.W.W.A. Speci
- 17. All sanitary manhetes shall be waterproafed on the exterior in accordant Missouri Department of Notural Resources specifications 10 CSR-8.120
- 18. Brick will not be used in the construction of sanitary sewer manholes.19. All pipes shall have positive drainage through manholes. No flat base
- are allowed. 20. The City of O'Fallon shall be natified 48 hours prior to construction for coordination and inspection.
- Gas, water and other underground utilities shall not conflict with the a horizantal location of existing or proposed sanitary or storm sewers, i house laterols.
- All existing site improvements disturbed, damaged or destroyed shall l or replaced to closely match pre-construction conditions.
- 23. The contractor shall prevent all storm, surface water, mud and constru from entering the existing sanitary sewer system.
- 24. All construction and materials shall conform to the current construction af the City of O'Fallon.
- All sanitory and storm sewer trench backfills shall be water jetted. C backfill will be used under pavement areas.
- 26. All existing areas disturbed during construction of the off-site sanitar shall be seeded and mulched to prevent erasion.
  27. All sanitary sewer (aterals shall be a minimum of 4" in diometer per (
- O'Fallan.
- 28. All storm inlets must be installed with a 5/8" trash bar ocross the a
- Cancrete pipe for storm sewers shall be Class III, A.S.T.M. C-76 with diameter of 12" except in the R.O.W. it shall be 15".
- 30. The ADS N-12 pipe shall have a smooth interior wall.31. Concrete pipe joints shall be MSD type "A" approved compression-typ
- shall conform to the requirements of the specifications for joints for concrete sewer and culvert pipe, using flexible, watertight, rubber—type (A.S.T.M.—C—443). Band—type gaskets depending entirely on cement for resistance to displacement during jointing shall not be used.
- 32. When HDPE pipe is used. City of O'Fallon specifications or manufacture specifications, which ever are more stringent, shall be followed.
- 33. The use of High Density Polyethylene Corrugated pipe, ADS N-12 WT of be permitted as an acceptable alternative to reinforced concrete pipe, shall be used for all ADS pipe greater than 36". Pipe shall meet A.S.T A.A.S.H.T.O. M-294-291.
- 34. All flared end sections and inlet structures will be concrete.
- 35. All storm sewer pipe installed in the Public Right-of-Way shall be Rei concrete Class III pipe.
- 36. All concrete pipe or ADS N-12 pipe shall be installed with "O-Ring" R gaskets per M.S.D. standard construction specifications or manufacture
   37. Blow-off hydrants and water meters shall not be located in any paver
- surfoced area including, but not limited to, driveways, sidewalks, and Since the locatian of all such areas is not shown an this plan all cas relocate any blow—off hydrants and water meters from any pavement surfaced areas shall be barne by the Developer or the Builders.
- All creek crossings shall be grauted rip-rap as directed by District ins grout shall be high slump ready-mix concrete.)
- 39. Existing sanitary sewer service shall not be interrupted.
  40. Pre-manufactured adapters shall be used at all PVC to DIP connection Rubber boot/Mission-type couplings will not be allowed.
- 41. All utilities shall be located underground.
- Storm and sanitary sewer pipe place at less than 1% slope shall have of pipe slope before backfilling.
- 43. Any permits, licenses, easements, or approvals required to work on pul private properties or roadways are the responsibility of the developer.
- 44. No slopes shall exceed 3(H):1(V).

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	A SET OF AS-BUILT PLANS	FOR CDDD
TE	ENCLAVE AT DE	ER CREEK
	QUARTER OF SECTION 19. TOWNSHIP	47 NORTHWEST 47 NORTH,
1" granular	RANGE 3 EAST OF THE FIFTH PR	
ow the pipe of same top of pipe.	MERIDIAN, ST. CHARLES COUNTY, N	MISSOURI
proding and		
Record		1 COVER SHEET 2 SITE PLAN
itary sever	15 16	3 SITE PLAN 4 SANITARY SEWER PF
to outside sewers, tion that		5 STORM SEWER PROF
A full sed so that from as	13 19 19 1008 VALLET	
of the crosses.		
and C906 DR water pipe.		
ther things. drinking water	7 VARMA VIAN LANG	
ome in contact certified, it will	6 5 4 3	
the piping shall		
PSI or higher the District. In d two pressure	COMMON GROUND	
t auxiliary valves rant auxiliary ct. All pumping		DEVELOPMENT
After achieving (2) hours. At ddition, if the		1. Area of tract: 41.48 Acres 2. Existing Zoning: "R-1" Single Family Resid
be continuing to ny further drops ils, the contractor s requires		4. Minimum Lot Size - "R-1": 10,000 Squa 5. Area in Lots: 12.38 Acres 6. Area in Right-of-Way: 2.04 Acres
e achieved.		7. Area in Cammon Ground: 29.10 Acres 8. Area Reserved far Future Development: 0.2 9. Yard Requirements — "R—1"
ations and	KEY MAP	A. Front Yard: 25 Feet B. Side Yard: 6 Feet C. Rear Yard: 25 Feet
nform to s shall ar water	<u>GENERAL NOTES</u>	11. Maximum Lot Coverage: 30% 12. Enclosed decks must mointain a 25 foot 13. Current Owner/Developer: Vantage He
ification C-111. nce with	<ul> <li>45. Driveway locations shall not interfere with the sidewalk curb ramps.</li> <li>46. City approval of the Construction plans does not mean that Single Family dwelling units can be constructed on lats without meeting the minimum.</li> </ul>	P.O. Box St. Peters 14. The site is served by the following:
) (7)E.	47. Sidewalks and sidewalk curb ramps shall be constructed in accordance with the	A. Sanitary Sewers: City of O'Fallon B. Water: City of O'Fallon C. Electric: AmerenUE Electric Compo D. Talephone: CenturyTel, Inc.
structures	current approved "Americans with Disabilities Act Accessibility Guidelines" (A.D.A.A.G.). If any conflict occurs between the above information and the plans the A.D.A.A.G. shall take precedence and the contractar prior to any construction shall patify the Design Englisher.	E. Gas: Laclede Gas Company F. School: Fort Zumwalt School Distri G. Fire: O'Fallon Fire Pratection Distri
or	48. Contractor shall assume complete responsibility for controlling all siltation and erosion of the project area. The contractor shall use whatever means necessary	<ol> <li>Fload plain exists on this site per F.I.R.M. Dated: March 17, 2003.</li> <li>Topographic information is an U.S.G.S. Date: 10.1000 (2000)</li> </ol>
including	to cantrol erosion ond siltation including, but not limited to, staked straw bales and/or siltation fabric fences (possible methods of control are detailed in the plan). Cantrol shall commence with grading and be maintained throughout the	17. Boundary information provided per bounda 18. All lacal streets will be constructed to Ci Streets will consist af 26 foot wide con colled ourb centered in a 50 foot right
be repaired	project until acceptance af the work by the awner and/or the City of O'Fallon. The contractors responsibilites include all design and implementation as required to prevent erasian and the depositing of silt. The Owner and/or the City of	feet. 19. All cul—desacs and bubbles will have o with a minimum right—of—way radii of 5-
on standards	O'Fallon may at their option direct the contractar in his methods as deemed fit to protect property and improvements. Any depositing of silts or mud on new or existing pavement or in new or existing storm sewers or swales shall be removed after each sein and affected group cleaned to the patiefaction of the	have a minimum rounding radius of 25 t 20. Minimum street grades shall be 1% 21. A 4 foot wide concrete sidewalk shall be
Franular	Owner and/or City of O'Fallon. 49. Only wes are to be used for lateral cannection to sanitary mains. Tees may be	as indicated. 22. All homes shall have a minimum of 2 of garages 23. All utilities must be located underground
y sewer line	used only if approved by the City of O'Fallon. 50. All paving to be in accordance with St. Charles County Standards and Specifications	24. The developer realizes that they will com Ordinance Number 1689 and provide land the City of O'Fallon Zoning Ordinances.
City of	except as madified by the City of O'Fallon ordinances. 51. All sign post, backs, bracket arms, street signs and traffic signals shall be painted	25. Additional lighting may be required by th 26. The following lots are susceptible to stre 27. Calculations in accordance to the "Tree I
pening.	or equivalent as approved by The City of O'Fallon and/or MoDOT.)	Existing Trees = 7.84 Acres x 20% = 1.57 Acres n Saved Trees = 2.14 Acres (27%)
a minimum	53. The maximum lot coverage by buildings or structures shall not exceed 30% of the lot	2.14 Acres of Saved Trees is greater saved, so therefore no additional trees Street trees requirements = 1 Tree/Lo
e joints and	area. 54. Any proposed pavilians or playground areas will need separate permit from the Building	Total Street Trees to be planted = 27 Total trees to be planted with this de 28. Maximum slope on yard slapes will be 3:
circular e gaskets or adhesion and	55. Any retaining walls on site will be maintained by the Homeowners Association.	29. Common graund areas will be maintained 30. All construction traffic for this developme Water View Lone. 31. The developer in grant that popitary per
ers	56. St. Charles County Highway Department shall be notified 24 hours prior to the start af construction. Contact Vance Gribble, Chief Inspector, at 636-949-7305.	until a new Lift Station is completed. 32. A Geological Engineer will be needed to r construction.
or equal will , ADS N-12 HC	57. A Special Use Permit must be obtained from St. Charles County Highway Department before any work can be performed on Bridal Poth Lane right—of—way. Please contact Donna C. Ray ot St. Charles County Highway Department at 636—949—7305.	<ul> <li>33. The roadway connection to Bridal Path L County.</li> <li>34. The roadway connection to Bridal Path L</li> </ul>
1.MD-2321 and		percent (50%) of the Building Permits fo the City of O'Fallon. 35. Stormwoter detention is not required for
nforced		to the valume of detention normally requ
Rubber type er.	VEGETATIVE ESTABLISHMENT For Urban Development Sites Seedina Rates: APPENDIX A	LEGEND
ment or hard streets. sts to	Permanent: Tall Fescue — 80 lbs./ac. Smooth Brome — 100 lbs./ac	C.1. CURB INLET Ø STREET LIGHT
or hard	Combined Fescue @ 40 lbs./ac. and Brome @ 50 lbs./ac. Temporary:	D.C.I. DOUBLE CURB INLET A.I. AREA INLET
spectors. (All	Wheat or Rye — 90/120 lbs./ac. (2.0/2.5 lbs. per 1000 square feet) Oats — 80 lbs./ac. (2 lbs. per 1000 square feet) Seeding Periods:	F.E.     FLARED END SECTION     SOL =     FRAF OSED CONTOUR       E.P.     END PIPE     S X S     STREET SIGN       C.P.     CONCRETE PIPE     S X S     STREET SIGN
ns.	Fescue or Brome — February 1 to June 1 August 1 to November 1 Wheat or Rive — Japuary 1 to June 1 July 15 to November 15	C.M.P. CORRUGATED METAL PIPE WY C.I.P. CAST IRON PIPE X WATER VALVE P.V.C. POLY VINYL CHLORIDE (PLASTIC)
	Oats - February 1 to June 1, August 1 to October 1 Mulch Rates: 70-115 lbs. per 1,000 sq. feet (3000-5000 lbs. per acre)	C.O. CLEAN OUT BLOW OFF ASSEMBL
ileia verification	Fertilizer Rates: Nitrogen 30 Ibs./ac. Phosphate 60 Ibs./ac. Potassium 30 Ibs./ac.	
	Lime 600 lbs./ac. ENM* * ENM = effective neutralizing material as per State	
	evaluation of quarried rock.	







EXH HW GONO	MH 101 83			MH 102	<
				P C C C C C C C C C C C C C C C C C C C	
BREAK I					
490					
<b>48</b> 0					
470			PROPOSED GRADE		
460	GRANULAR		EXISTING GROUND-	GRAN FI	
450	61.49' ~ 8" P.V.C.		0.2 0	.4	4.
	20.00'~ 3" D.I.P. "		-25-T.S.=451		۲ FIRST % +91–T.S.=46
FL OUT=440 FL OUT=440 FL IN=454.6	FL OUT=455 FL OUT=455 FL IN=455.4 FL IN=455.4		LOT 7-1+	FL OUT=457 FL OUT=457 FL OUT=457 FL N=4578 FL N=4578	NOTE: LA 55' @ 1.7 LOT 5-0-
82 81	21' ~ 8' PIPE <u>49' ~ 8' PIPE</u> <del>1.00%</del> 0.95%	203.30', ~ 8" <del>205.79' ~ 8"</del> <del>1.00%</del> 0.91%	P.V.C. P.V.C.		
(MH 102)		MH 110		0.97 LOW	(N 1
TOP=468.60 +0P-408.70	TOP=466.90	<del>10P 467.00</del>		TOP=485.63	CV = 480.49 CV = 482.65 TOP = 490.95
480		480		510	RM-0+66.58-EL
470 PRO GRA	POSED DEEXISTING GROUND	470		500	ATER VIEW LANE 8" SANITA 24" STO
460		460		490	
450		450		480	BACKFILD BRANULAR FILL
<b>*</b> D-		9		470 E	ENCASEMENT
FL OUT=457.3 FL OUT=457.4 FL IN=457.54 FL IN=457.54	FL OUT=458.8	FL- 0UT-158.5		FL 0UT=476.0 FL 0UT=476.0 FL 0UT=479.1 FL IN=476.21	NOTE: LAY 65 @ 3.53 LOT 17-0+
	131.23' ~ 8" P.V.C. <u>131.75' ~ 8" P.V.C.</u> <del>1.00%</del> 0.99%			-	95.21' ~ 8" P.V.C. <del>94.64' ~ 8" P.V.C.</del> <del>1.75%</del> <b>4.74%</b>

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UPP STR	LOW STR	L	DIA	UPPER FL LN	LOWER FL LN	PS	UPPER ST EL	DEPTH HY GR	UPPER HY EL	LOWER HY EL	HYDR GRADE	FR HEAD	VEL	VEL HEAD	JUNC LOSS	TURN LOSS	CURVE LOSS	STR GRADE	INL CAP	DR AREA	PI	Q	TQ	PIPE CAP	REMARKS
CI30 CI29	CI29 FE28	35 65	15 15	462.36 461.79	461.99 461.02	1.05 1,19	467.45 467.59	1.65 1.79	465.80 465.80	465.80 465.78	.00010 .00020	0.00 0.01	0.39 0.72	0.00 0.01	0.00 0.01	0.00 0.00	0.00	2.34 2.34	1.53 1.53	0.18 0.15	2.64 2.64	0.48 0.40	0.48 0.88	6.61 7.04	1 2 100yrHW=465.78
A127 A126 C125 C124	AI26 CI25 CI24 FE23	127 108 36 170	12 24 24 27	465.70 464.20 463.00 462.54	464.40 463.20 462.74 460.83	1.03 0.92 0.73 1.01	470.40 467.92 468.69 468.68	3.73 1.83 2.81 2.89	466.67 466.09 465.88 465.79	466.09 465.88 465.79 465.62	.00360 .00070 .00120 .00090	0.46 0.08 0.04 0.16	2.72 1.97 2.50 2.38	0.12 0.06 0.10 0.09	0.12 0.05 0.05 0.01	0.00 0.08 0.00 0.00	$0.00 \\ 0.00 \\ 0.00 \\ 0.00 \\ 0.00$	3-S 4-S LOW LOW	7.01 9.35 3.40 3.40	0.81 1.53 0.61 0.61	2.64 2.64 2.64 2.64	2.14 4.04 1.66 1.62	2.14 6.18 7.84 9.46	3.61 21.73 19.28 31.08	3 4 5 6 100yrHW=465.62
A122 MH21	MH21 CI13	111 98	12 12	489.54 487.39	487.59 483.96	1.76 3.51	494.39 495.43	4.24 7.54	490.15* 487.89*	488.59 486.24	.00840 .00840	0.93 0.82	4.16 4.16	0.27 0.27	0.27 0.13	0.00 0.13	0.00	4-S MH	9.35 0.00	1.24 0.00	2.64 0.00	3.27 0.00	3.27 3.27	4.73 6.67	7 8
C120	CI10	36	15	472.65	472.01	1.78	477.34	4.08	473.26	473.26	.00010	0.00	0.38	0.00	0.00	0.00	0.00	5. <del>9</del> 8	0.47	0.26	2.64	1.18	0.47	8.61	9
DAI19 AI18 MH17 MH16 CI15 CI14 CI13 CI12 MH11 CI10 CI9 CI8 MH7	9 A:18 ME17 MH16 CI15 CI14 CI13 CI12 MH11 CI10 C19 CI8 MH7 CI4	54 95 82 120 35 52 35 37 133 116 35 82 188	21 21 21 21 24 24 24 24 24 24 27 27	497.85 490.69 489.41 488.20 485.84 485.25 483.76 483.16 481.26 471.81 465.99 463.72	490.89 489.61 488.40 486.04 485.45 483.96 483.96 483.36 481.46 472.01 465.14 465.29 463.92 461.27	12.87 1.13 1.23 1.80 1.10 2.50 1.14 4.61 6.96 4.91 1.83 1.43 1.31	500.50 495.29 502.05 499.47 491.65 491.15 489.56 489.57 486.36 477.23 471.60 470.86	2.13 2.65 10.79 10.04 3.52 3.93 3.32 5.34 4.32 4.54 1.79 3.00 2.82	498.38* 492.64 491.26 489.43 488.13 487.22 486.24 484.23 482.04* 472.69* 469.82 468.60 468.04	492.64 491.36 490.15 488.13 487.22 486.24 485.36 483.46 474.01 469.82 468.60 468.04 466.90	.00550 .00750 .00750 .00790 .00840 .00660 .00700 .00700 .00700 .00780 .00830 .00830 .00490 .00490	$\begin{array}{c} 0.30\\ 0.71\\ 0.61\\ 0.90\\ 0.28\\ 0.44\\ 0.23\\ 0.26\\ 0.94\\ 0.90\\ 0.30\\ 0.40\\ 0.92\end{array}$	4.89 5.69 5.69 5.807 6.086 6.04 6.57 5.45 5.45	0.37 0.50 0.50 0.54 0.57 0.57 0.57 0.62 0.67 0.46 0.46	$\begin{array}{c} 0.37 \\ 0.36 \\ 0.25 \\ 0.20 \\ 0.34 \\ 0.29 \\ 0.25 \\ 0.25 \\ 0.25 \\ 0.15 \\ 0.49 \\ 0.00 \\ 0.11 \end{array}$	0.00 0.21 0.25 0.20 0.29 0.25 0.40 0.23 0.25 0.07 0.43 0.16 0.11		4-S 3-S MH 6.00 6.00 4.00 4.00 MH 5.98 4.00 4.00 MH	9.35 7.01 0.00 0.43 0.43 1.03 1.03 0.00 0.47 1.03 1.03 0.00	$\begin{array}{c} 4.46\\ 0.73\\ 0.00\\ 0.35\\ 0.32\\ 0.22\\ 0.22\\ 0.00\\ 0.23\\ 0.07\\ 0.14\\ 0.00\\ \end{array}$	2.64 2.64 0.00 2.64	$11.77 \\ 1.92 \\ 0.00 \\ 0.92 \\ 0.84 \\ 0.58 \\ 0.58 \\ 0.00 \\ 1.02 \\ 0.73 \\ 1.08 \\ 0.00 \\$	11.77 $13.69$ $13.69$ $14.12$ $14.55$ $18.40$ $18.98$ $19.92$ $20.65$ $21.68$ $21.68$	56.85 16.87 17.59 21.25 16.65 25.05 24.20 48.56 59.66 50.11 30.62 36.99 35.39	10 11 12 13 14 15 16 17 18 19 20 21 22
FF.6 AI5 CI4 CI3 MH2 * INI	A15 CI4 C13 MH2 FE1 DICATES C	19 96 35 22 140 RITICAL [	36 36 48 54 54 DEPTH	462.77 462.34 461.14 460.63 460.25	462.54 461.34 460.83 460.45 458.84	1.24 1.04 0.88 0.81 1.01	469.15 469.15 469.09 469.19 469.82	0.83 1.71 2.19 2.69 3.61	468.32 467.44 466.90 466.50 466.21	467.44 466.90 466.50 466.21 465.74	.00560 .00560 .00250 .00140 .00140	0.10 0.54 0.09 0.03 0.19	7.09 7.09 5.76 4.59 4.59	0.78 0.78 0.51 0.33 0.33	$0.78 \\ 0.00 \\ 0.00 \\ 0.04 \\ 0.14$	0.00 0.00 0.31 0.22 0.14	0.00 0.00 0.00 0.00 0.00	LOW 3-S 1.00 1.00 MH	66.00 7.01 1.95 1.95 0.00	18.97 0.00 0.22 0.23 0.00	2.64 0.00 2.64 2.64 0.00	50.09 0.00 0.58 0.61 0.00	50.09 50.09 72.35 72.96 72.96	74.19 68.09 134.37 176.55 197.62	23 24 25 26 27 100yrHW=465.74

	AS-BUILT DETENTION CROSS-SECTIONS THE ENCLAVE AT DEER CREEK 02-11841 OCTOBER 20
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DERIGN AVAILABLE INFORMATION AND THEREFORE THEIR LOCATIONS SHALL BE CONSIDERED APPROXIMATE ON UNDERGROUND UTILITIES, EITHER SHOWN OR NOT SHOWN ON THESE PLANS SHALL BE THE RESPONSIBILITY O PRIOR TO ANY GRADING OR CONSTRUCTION OF THE IMPROVEMENTS.	DF SCALE: VERTICAL = 1"=10'