

GRADING NOTES

1. A Geotechnical Engineer shall be employed by the owner and be on site during grading operations. All soils tests shall be verified by the Geotechnical Engineer concurrent with the grading and backfilling operations.
2. The grading contractor shall perform a complete grading and compaction operation as shown on the plans, stated in these notes, or reasonably implied there from, all in accordance with the plans and notes as interpreted by the Geotechnical Engineer.
3. The Contractor shall notify the Soils Engineer at least two days in advance of the start of the grading operation.
4. All areas shall be allowed to drain. All low points shall be provided with temporary ditches.
5. A sediment control plan that includes monitored and maintained sediment control basins and/or straw bales should be implemented as soon as possible. No graded area is to be allowed to remain bare without being seeded and mulched. Care should be exercised to prevent soil from damaging adjacent property and silting up existing downstream storm drainage system.
6. Debris and foundation material from any existing on-site building or structure which is scheduled to be razed for this development must be disposed of off-site.
7. All trash and debris on site, either existing or from construction, must be removed and properly disposed of off-site.
8. Soft soil in the bottom and banks of any existing or former pond sites or tributaries or on any sediment basins or traps should be removed, spread out and permitted to dry sufficiently to be used as fill. None of this material should be placed in proposed public right-of-way locations or on any storm sewer locations.
9. Site preparation includes the clearance of all stumps, trees, bushes, shrubs, and weeds; the grubbing and removal of roots and other surface obstructions from the site; and the demolition and removal of any man-made structures. The material shall be properly disposed of off-site. Topsoil and grass in the fill areas shall be thoroughly disced prior to the placement of any fill. The Soils Engineer shall approve the discing operation.
10. Compaction equipment shall consist of tamping rollers, pneumatic-tired rollers, vibratory roller, or high speed impact type drum rollers acceptable to the Soils Engineer. The roller shall be designed so as to avoid the creation of a layered fill without proper blending of successive fill layers.
11. The Soils Engineer shall observe and test the placement of the fill to verify that specifications are met. A series of fill density tests will be determined on each lift of fill. Interim reports showing fill quality will be made to the Owner at regular intervals.
12. The Soils Engineer shall notify the Contractor of rejection of a lift of fill or portion thereof. The Contractor shall rework the rejected portion of fill and obtain notification from the Soils Engineer of its acceptance prior to the placement of additional fill.

GENERAL NOTES

1. Underground utilities have been plotted from available information and therefore their locations shall be considered approximate only. The verification of the location of all underground utilities, either shown or not shown on these plans shall be the responsibility of the contractor, and shall be located prior to any grading or construction of the improvements.
2. All manhole tops built without elevations furnished by the Engineer will be the responsibility of the sewer contractor.
3. 8" P.V.C. sanitary sewer pipe shall meet the following standards. A.S.T.M.-D-3034 SDR-35, with wall thickness compression joint A.S.T.M.-D-3212. An appropriate rubber seal waterstop as approved by the sewer district shall be installed between P.V.C. pipe and masonry structures.
4. All filled places, including trench backfills, under buildings, proposed storm and sanitary sewer lines and/or paved areas, shall be compacted to 90% maximum density as determined by the "Modified AASHTO T-180 Compaction Test," (A.S.T.M.-D-1557). All filled places within public roadways shall be compacted to 95% of maximum density as determined by the "Standard Proctor Test AASHTO T-99, Method C" (A.S.T.M.-D-698).
5. All trench backfills under paved areas shall be granular backfill, and shall be compacted to 90% of the maximum density as determined by the "Modified AASHTO T-180 Compaction Test," (A.S.T.M.-D-1557). All other trench backfills may be earth material (free of large clods or stones). All trench backfills shall be water jetted.
6. No area shall be cleared without the permission of the Project Engineer.
7. All P.V.C. sanitary sewer is to be SDR-35 or equal with clean 1/2" to 1" granular stone bedding uniformly graded. This bedding shall extend from 4" below the pipe to the springline of the pipe. Immediate backfill over pipe shall consist of same size "clean" or minus stone from springline of pipe to 6" above the top of pipe.
8. All soils test shall be verified by a Soils Engineer concurrent with the grading and backfilling operations.
9. Easements shall be provided for sanitary sewers, and all utilities on the Record Plat. See Record Plat for location and size of easements.
10. Maintenance and upkeep of the common ground area shall be the responsibility of the developer and/or successors.

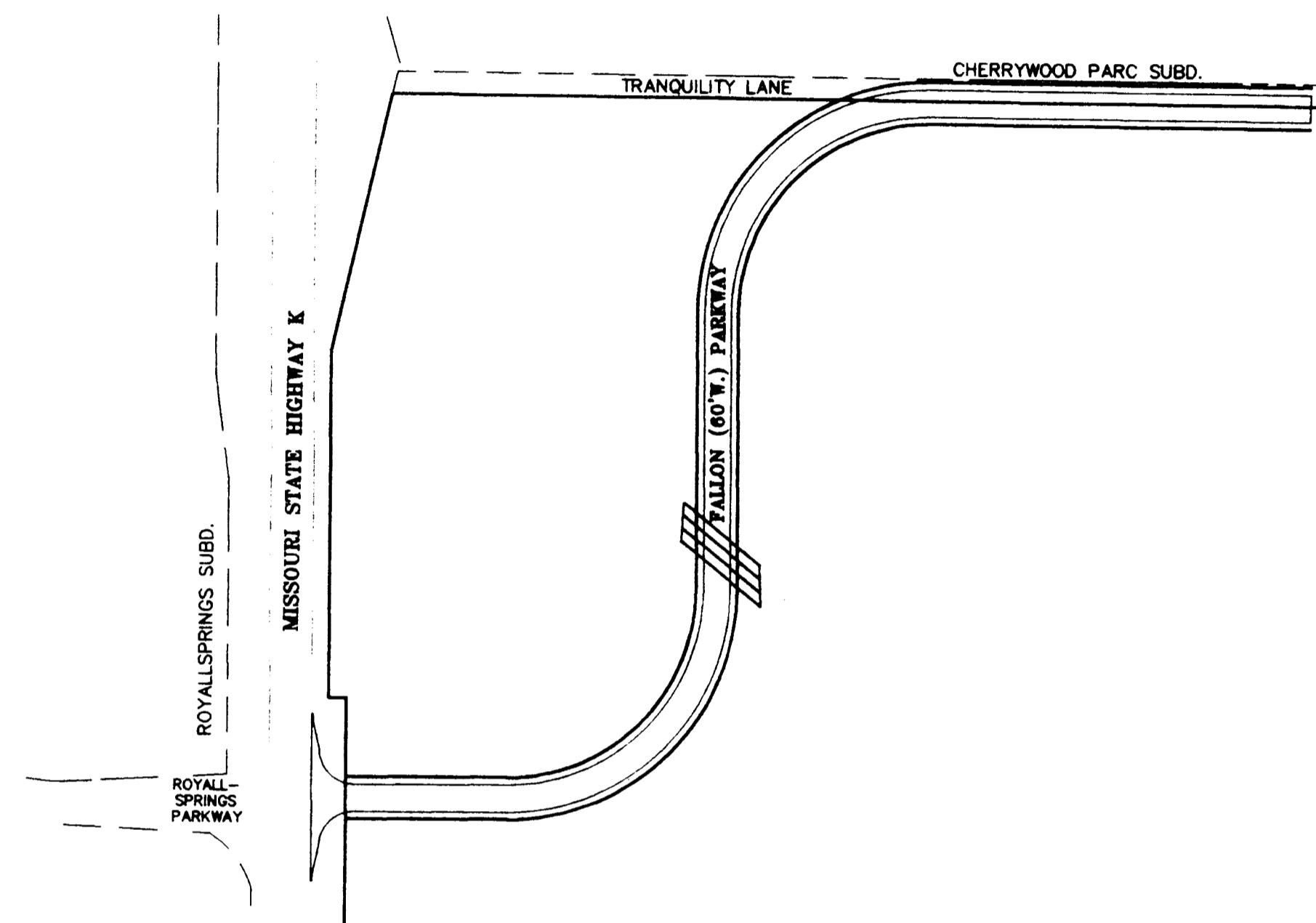
CATEGORY	MINIMUM PERCENT COMPACTION
Natural subgrade	88%
Pavement subgrade	90%
Pavement base course	90%

Measured as a percent of the maximum dry density as determined by modified Proctor Test (ASTM-D-1557).

Moisture content must be within 2 percent below or 4 percent above optimum moisture content if fill is deeper than 10 feet.

A SET OF AS-BUILTS FOR
FALLON PARKWAY COLLECTOR ROAD

A TRACT OF LAND BEING PART OF FRACTIONAL SECTION 4,
TOWNSHIP 46 NORTH, RANGE 3 EAST
OF THE FIFTH PRINCIPAL MERIDIAN,
CITY OF O'FALLON, ST. CHARLES COUNTY, MISSOURI



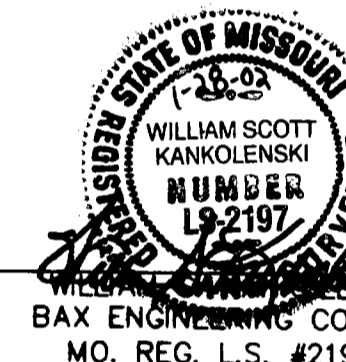
REFERENCE BENCHMARK

R.M. #74 - ELEV.=493.07 (U.S.G.S. DATUM)
CHISELED SQUARE ON TOP OF EAST CONCRETE HEADWALL,
OF BIRDIE HILLS ROAD BRIDGE OVER TRIBUTARY NO. 2
(APPROXIMATELY 500 FEET SOUTH OF EISENHOWER DRIVE)

SITE BENCHMARK

ELEV.=572.28 (U.S.G.S. DATUM)
OLD CROSS CL-CL SWEETBAY DRIVE AND CHERRYWOOD PARC DRIVE
CHERRYWOOD PARK SUBDIVISION

THIS IS TO CERTIFY THAT WE HAVE DURING THE MONTH OF JANUARY, 2002, BY ORDER OF KAPLAN DEVELOPMENT COMPANY, EXECUTED AN AS-BUILT SURVEY OF STORM SEWERS WITHIN "FALLON PARKWAY-COLLECTOR ROAD", A SUBDIVISION ACCORDING TO THE PLAT THEREOF RECORDED IN PLAT BOOK _____ PAGE(S) _____ OF THE ST. CHARLES COUNTY RECORDS. ALL SEWERS SHOWN LIE WITHIN THE EASEMENTS AS SHOWN ON SAID RECORDED SUBDIVISION PLAT UNLESS OTHERWISE NOTED. THE SANITARY LATERALS IF ANY WERE SUPPLIED TO BAX ENGINEERING BY THE CONTRACTOR, THEREFORE THEIR LOCATION IS ASSUMED APPROXIMATE. ALL SEWERS SHOWN LIE WITHIN THE EASEMENTS AS SHOWN UNLESS OTHERWISE NOTED. THE RESULTS OF THIS AS-BUILT SURVEY ARE SHOWN ON THIS PLAT TO THE BEST OF MY KNOWLEDGE AND BELIEF.



23. All construction and materials shall conform to the current construction standards of the City of O'Fallon and Duck Creek Sanitary District.
24. All sanitary and storm sewer trench backfills shall be water jetted. Granular backfill will be used under pavement areas.
25. All existing areas disturbed during construction of the off-site sanitary sewer line shall be seeded and mulched to prevent erosion.
26. All sanitary sewer laterals shall be a minimum of 4" in diameter per City of O'Fallon.
27. No flushing hydrants or water meters shall be located in driveways and or walkways.
28. Concrete pipe for storm sewers shall be Class III, A.S.T.M. C-76 with a minimum diameter of 12" except in the R.O.W. it shall be 15".
- ~~29. The ABC-N-12 pipe shall have a smooth interior wall.~~
30. Concrete pipe joints shall be MSD type "A" approved compression-type joints and shall conform to the requirements of the specifications for joints for circular concrete sewer and culvert pipe, using flexible, watertight, rubber-type gaskets (A.S.T.M.-C-443). Band-type gaskets depending entirely on cement for adhesion and resistance to displacement during jointing shall not be used.
- ~~31. When HDPE pipe is used, City of O'Fallon specifications or manufacturer specifications, which ever are more stringent, shall be followed.~~
- ~~32. The use of High Density Polyethylene Corrugated pipe, ABC-N-12 or equal will be permitted as an acceptable alternative to reinforced concrete pipe, ABC-N-12 HG shall be used for all ABC pipe greater than 36". Pipe shall meet A.S.T.M. D-2321 and A.S.T.M. M-204-201.~~
33. All flared end sections and inlet structures will be concrete.
34. All storm sewer pipe installed in the Public Right-of-Way shall be Reinforced concrete Class III pipe.
- ~~35. All concrete pipe or ABC-N-12 pipe shall be installed with "O-Ring" Rubber-type gaskets per M.C.D. standard construction specifications or manufacturer.~~
36. All Fire Hydrants and Water Meters shall not be located in driveways and/or sidewalks.
37. All creek crossings shall be grouted rip-rap as directed by District inspectors. (All grout shall be high slump ready-mix concrete.)
38. Existing sanitary sewer service shall not be interrupted.
39. Pre-manufactured adapters shall be used at all PVC to DIP connections. Rubber boot / Mission-type couplings will not be allowed.
40. Any permits, licenses, easements, or approvals required to work on public or private properties or roadways are the responsibility of the developer.
41. No slopes shall exceed 3(H) : 1(V).

FALLON PARKWAY COLLECTOR ROAD
 PREPARED FOR: KAPLAN DEVELOPMENT COMPANY
 5140 NORTH SERVICE ROAD
 ST. PETERS, MISSOURI 63376
 (636) 397-4471

DISCLAIMER OF RESPONSIBILITY
I hereby certify that the documents intended to be authorized by my seal are limited to the sheets and I hereby disclaim any responsibility for all other drawings, specifications, instruments relating to or intended to be used for any part or parts of the architectural or engineering project or survey.

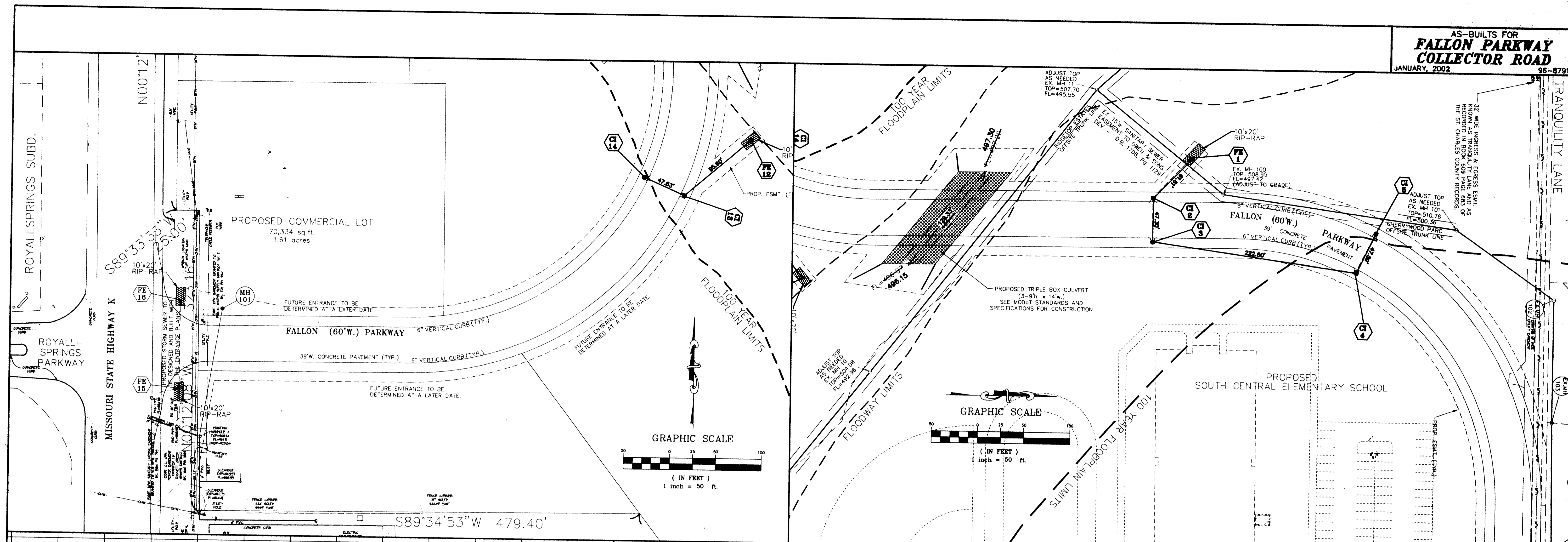
RECEIVED
JAN 29 2002
CITY OF O'FALLON, MO

REVISIONS

NO.	DESCRIPTION

BAX
ENGINEERING
PLANNING
SURVEYING
1052 South Cloverleaf Drive
St. Peters, MO. 63378-6445
314-928-5552
FAX 928-1718

JANUARY, 2002
DATE
96-8791e
PROJECT NUMBER
1 of 3
SHEET OF
8791eASB.dwg
FILE NAME
MROSE WSK
DRAWN CHECKED



Station	Elevation	Notes
0+00	530.18	
1+00	530.43	
2+00	530.68	
3+00	529.93	
4+00	528.68	
5+00	527.93	
6+00	524.18	
7+00	518.93	
8+00	514.68	
9+00	513.18	
10+00	510.67	
11+00	510.36	
12+00	511.17	
13+00	512.17	
14+00	513.42	
15+00	516.23	
16+00	518.17	

HIGH POINT ELEV = 530.93
 HIGH POINT STA = 1+50
 P.V.I. STA = 2+00
 P.V.I. ELEV = 531.68
 A = -4.00
 K = 50.00
 200.00' VC

P.V.I. STA = 7+00
 P.V.I. ELEV = 516.68
 A = 1.00
 K = 200.00
 200.00' VC

LOW POINT ELEV = 510.34
 LOW POINT STA = 10+83.67
 P.V.I. STA = 10+50.33
 P.V.I. ELEV = 509.67
 A = 3.00
 K = 56.67
 200.00' VC

P.V.I. STA = 15+00
 P.V.I. ELEV = 514.17
 A = 3.00
 K = 50.00
 150.00' VC

CL HIGHWAY K - 137+87.68
 0+00 - 530.52
 EX. EDGE OF PAVEMENT
 0+33.00 - 530.01
 EX. EDGE OF SHOULDER
 0+41.00 - 530.09

P.V.C. 1+00
 ELEV. 530.68
 P.V.T. 3+00
 ELEV. 528.68

P.V.C. 6+00
 ELEV. 519.68
 P.V.T. 8+00
 ELEV. 514.68

P.V.C. 9+50.33
 ELEV. 511.67
 P.V.T. 11+50.33
 ELEV. 510.67

P.V.C. 14+26
 ELEV. 513.42
 P.V.T. 15+75
 ELEV. 517.17

CI 13, CI 14
 6+50.00 - TOP=518.35

CI 2, CI 3
 10+83.33 - TOP=510.45

CI 4, CI 5
 13+19.19 - TOP=512.47

9'x14' BOX CULVERT

SCALES:
 1"=50' HORIZONTAL
 1"=10' VERTICAL

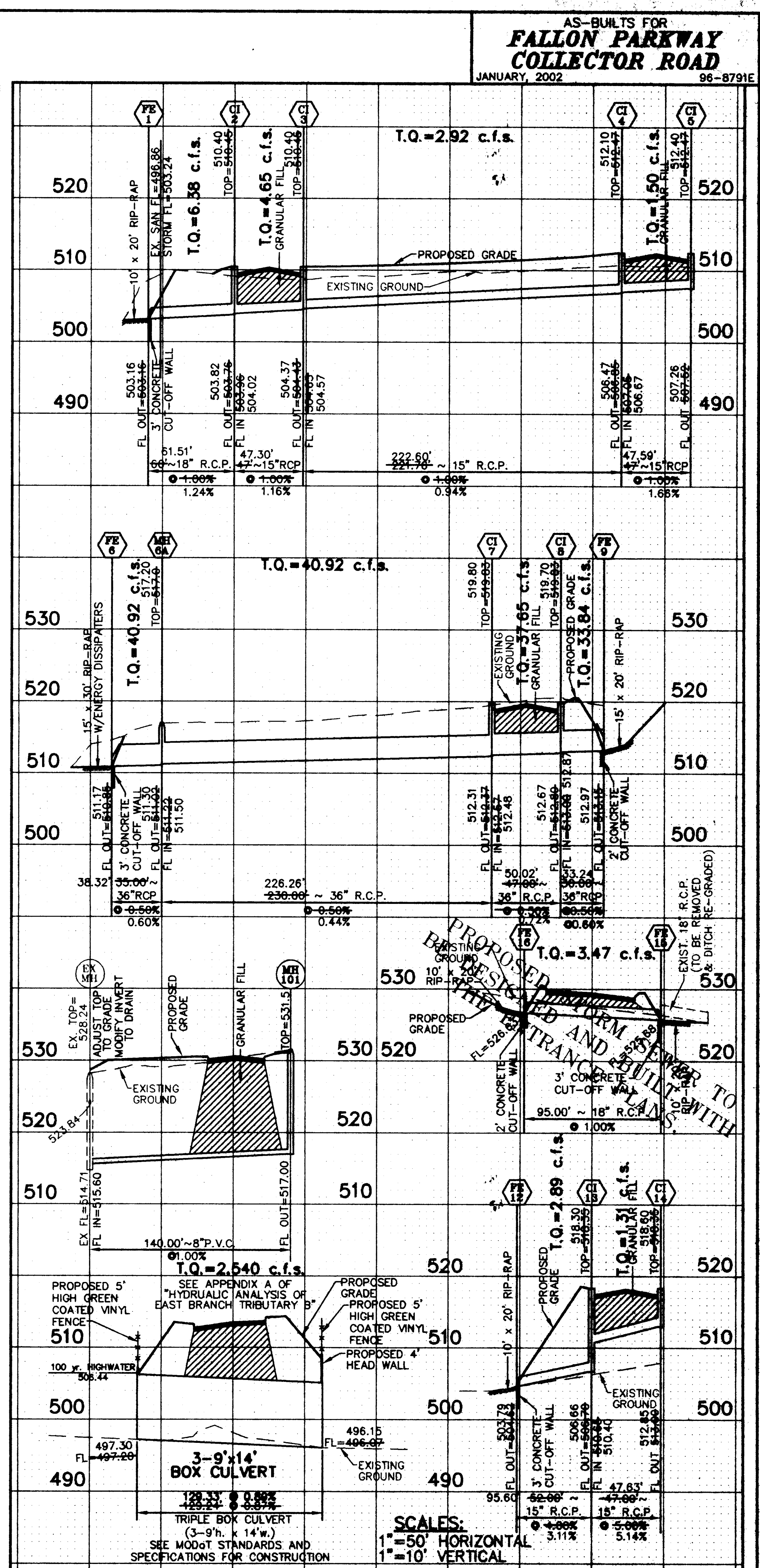
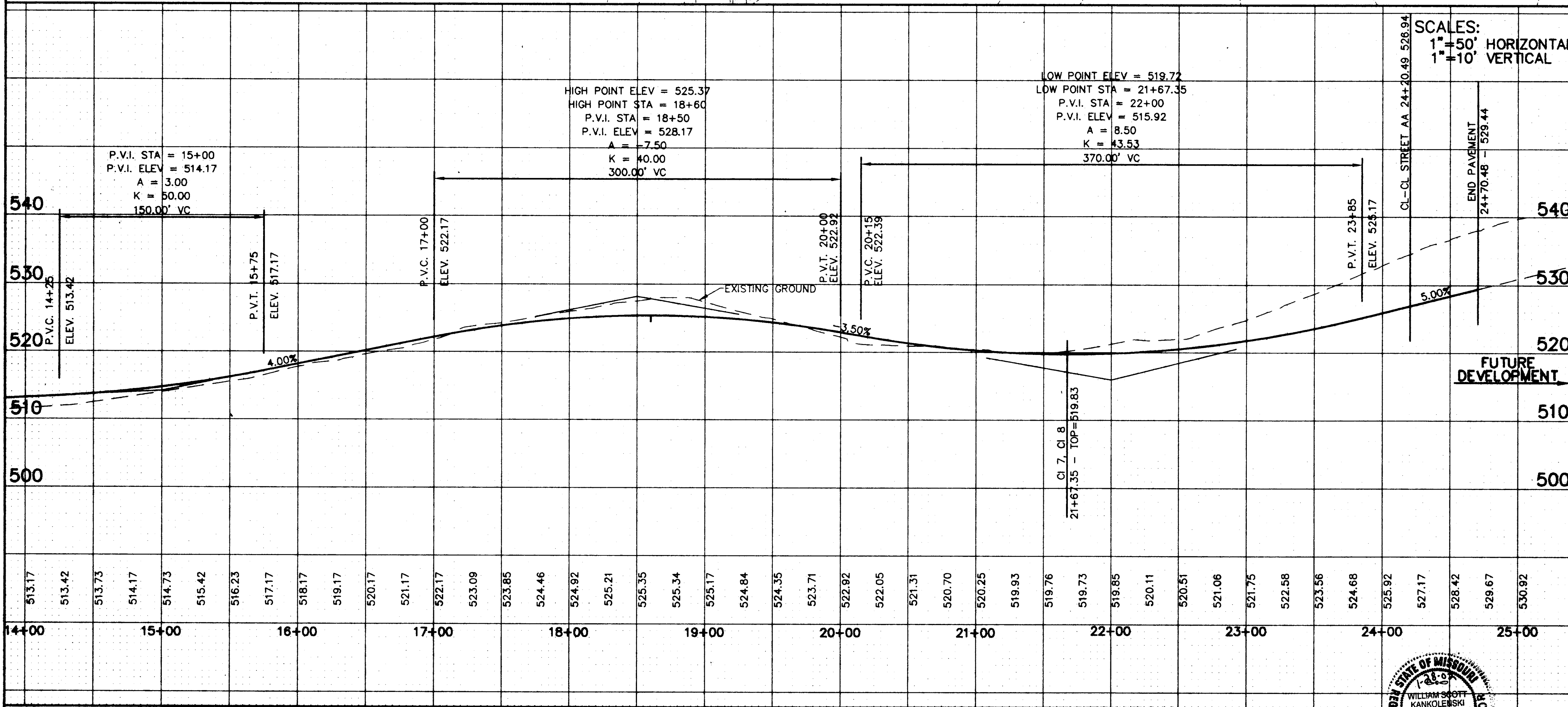
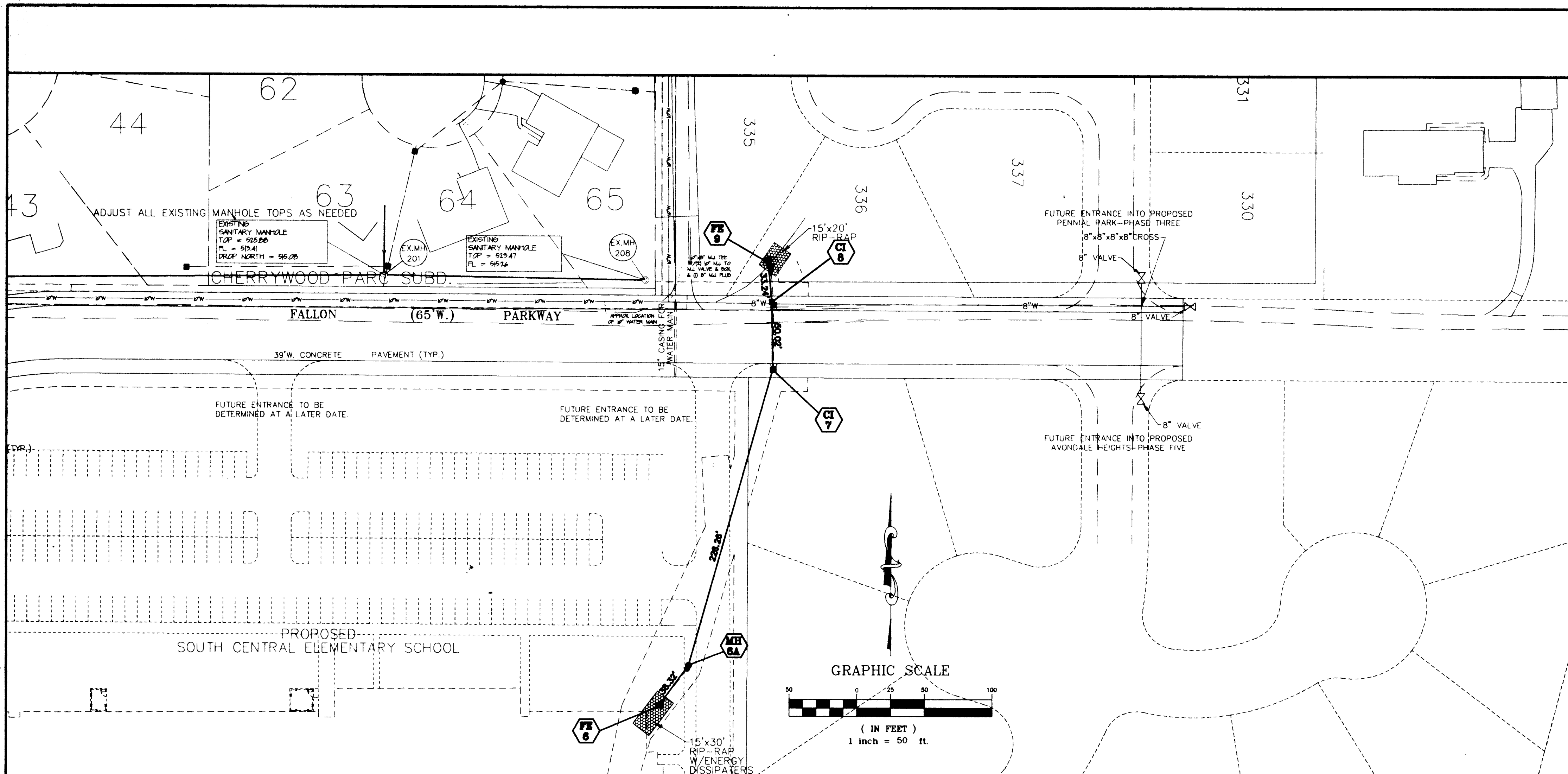


AS-BUILT NOTE:
 ALL DISTANCE AND SLOPE CALCULATIONS ARE FROM
 CENTER OF STRUCTURE TO CENTER OF STRUCTURE.

AS-BUILTS ADDED JANUARY, 2002

All underground utilities have been plotted from available information, and therefore, their locations must be considered approximate only. The verification of the location of all underground utilities, either shown or not shown on these plans, shall be the responsibility of the contractor and shall be located prior to any grading or construction of improvements.

E:\VMS\8609E\Fallon Parkway\CONSTRUCTION\8791E\ASBUILTS.dwg P1 Jan 25 10:46:41 2002 STA 18 WR



AS-BUILT NOTE:
 All DISTANCE AND SLOPE CALCULATIONS ARE FROM
 CENTER OF STRUCTURE TO CENTER OF STRUCTURE.

AS-BUILTS ADDED JANUARY, 2002

All underground utilities have been plotted from available information, and therefore, their locations must be considered approximate only. The verification of the location of all underground utilities, either shown or not shown on these plans, shall be the responsibility of the contractor and shall be located prior to any grading or construction of improvements.