

# A SET OF AS-BUILT PLANS FOR PENNIAL PARK PHASE 3

A TRACT OF LAND BEING PART OF FRACTIONAL SECTION 4,  
TOWNSHIP 46 NORTH, RANGE 3 EAST  
OF THE FIFTH PRINCIPAL MERIDIAN,  
CITY OF O'FALLON, ST. CHARLES COUNTY, MISSOURI



## DEVELOPMENT NOTES

- Area of Tract: 54.17 Acres
- Existing Zoning: R-1 (City of O'Fallon)
- Proposed User: Single Family Homes
- Number of Lots Proposed: 153 Lots
- The proposed height and lot setbacks are as follows:  
Minimum Front Yard: 25 feet  
Minimum Side Yard: 6 feet  
Minimum Rear Yard: 25 feet  
Minimum Lot Area: 10,000 square feet  
Maximum Height of Building: 2 1/2 stories or 35 feet
- Current Owner/Developer: Kaplan Development Company  
5140 North Service Road  
St. Peters, MO 63376
- Site is served by:  
Duckett Creek Sanitary District  
AmerenUE  
St. Charles Gas Company  
St. Charles County Public Water District No. 2  
Verizon Telephone Company  
Fort Zumwalt School District  
O'Fallon Fire Protection District
- No Flood Plain exists on this site per F.I.R.M. #29183 C 0243E, dated Aug. 2, 1996.
- Topographic information is per Walker and Associates Topo on U.S.G.S. Datum.
- Boundary information is per Bax Engineering during August, 1999.
- All streets will be constructed to City of O'Fallon standards.  
Streets will consist of 26 foot wide concrete pavement with integral rolled curb centered in a 50 foot right-of-way.
- All cul-de-sacs and bubbles will have pavement radii of 42 feet with right-of-way radii of 54 feet. Street intersections shall have a minimum rounding radius of 25 feet with pavement radii of 37 feet.
- Minimum street grades shall be 1%.
- A 4 foot wide concrete sidewalk shall be constructed on one side of streets where indicated.
- All homes shall have a minimum of 2 off-street parking places with 2-car garages.
- All utilities must be located underground.
- The following lots are susceptible to street movement:  
45, 46, 47, 48, 49, 55, 57, 58, 59, 60, 61, 65, 66, 67, 68, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 89, 90, 91, 92, 93, 94, 95, 96, 97, 183, 184, 185, 186, 187, 188, 189, 190, 191, 192, 193, 194, 195, 196, 198, 199, 200, 201, 203, 206, 207, 208, 209, 241, 244, 245, 246, 247, 300, 301, 308, 318, 319, 320, 321, 322, 324, 331, 332, 333, 336, 337, 338, 344, 345, 346.
- Calculations in accordance to the Tree Preservation Ordinance:  
Existing trees x 20% = 0.28 acres  
Save trees = 0.06 acres  
Trees to be Replaced = 0.07 acres  
No trees to be replaced

Street Tree Requirements: 1 per lot/2 per corner lot = 177 trees  
Total Required 177 trees

Note: Proposed trees shall be hardwood varieties with a 2" minimum diameter and a minimum height of 8'. Trees to be planted on the individual lots shall be planted after home construction and yard finish grading by the homeowner, as required by the covenants and restrictions.

Street trees shall be centered within the area between back of curb and sidewalk or back of curb and property line.

## SHEET INDEX

1	COVER SHEET
2-4	SITE PLAN
5-8	SANITARY SEWER PROFILES
9-12	STORM SEWER PROFILES

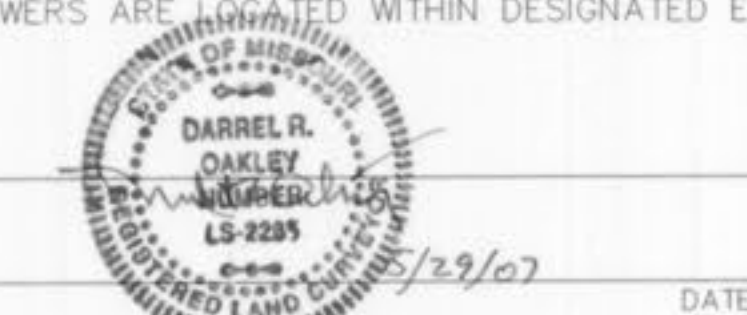
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## SEWER MEASUREMENTS

THE EXISTING SEWER LENGTHS, SIZES, FLOWLINES, DEPTHS OF STRUCTURES AND SEWERS AND LOCATIONS WITH RESPECT TO EXISTING OR PROPOSED EASEMENTS HAVE BEEN MEASURED. THE RESULTS OF THOSE MEASUREMENTS ARE SHOWN ON THIS SET OF FINAL MEASUREMENT PLANS.

ALL PUBLIC SEWERS ARE LOCATED WITHIN DESIGNATED EXISTING OR PROPOSED EASEMENTS EXCEPT AS FOLLOWS:

SIGNED: P.E./L.S.



DATE

AS-BUILT NOTE:  
ALL DISTANCE AND SLOPE CALCULATIONS ARE FROM  
CENTER OF STRUCTURE TO CENTER OF STRUCTURE.

## GRADING NOTES

- A Geotechnical Engineer shall be employed by the owner and be on site during grading operations. All soils tests shall be verified by the Geotechnical Engineer concurrent with the grading and backfilling operations.
- The grading contractor shall perform a complete grading and compaction operation as shown on the plans, stated in these notes, or reasonably implied there from, all in accordance with the plans and notes as interpreted by the Geotechnical Engineer.
- The Contractor shall notify the Soils Engineer at least two days in advance of the start of the grading operation.
- All areas shall be allowed to drain. All low points shall be provided with temporary ditches.
- A sediment control plan that includes monitored and maintained sediment control basins and/or straw bales should be implemented as soon as possible. No graded area is to be allowed to remain bare without being seeded and mulched. Care should be exercised to prevent soil from damaging adjacent property and silting up existing downstream storm drainage system.
- Debris and foundation material from any existing on-site building or structure which is scheduled to be razed for this development must be disposed of off-site.
- All trash and debris on site, either existing or from construction, must be removed and properly disposed of off-site.
- Soft soil in the bottom and banks of any existing or former pond sites or tributaries or on any sediment basins or traps should be removed, spread out and permitted to dry sufficiently to be used as fill. None of this material should be placed in proposed public right-of-way locations or on any storm sewer locations.
- Site preparation includes the clearance of all stumps, trees, bushes, shrubs, and weeds; the grubbing and removal of roots and other surface obstructions from the site; and the demolition and removal of any man-made structures. The material shall be properly disposed of off-site. Topsoil and grass in the fill areas shall be thoroughly disced prior to the placement of any fill. The Soils Engineer shall approve the discing operation.
- Compaction equipment shall consist of tamping rollers, pneumatic-tired rollers, vibratory roller, or high speed impact type drum rollers acceptable to the Soils Engineer. The roller shall be designed so as to avoid the creation of a layered fill without proper blending of successive fill layers.
- The Soils Engineer shall observe and test the placement of the fill to verify the specifications are met. A series of fill density tests will be determined on each lift of fill. Interim reports showing fill quality will be made to the Owner at regular intervals.
- The Soils Engineer shall notify the Contractor of rejection of a lift of fill or portion thereof. The Contractor shall rework the rejected portion of fill and obtain notification from the Soils Engineer of its acceptance prior to the placement of additional fill.
- All areas to receive fill shall be scarified to a depth of not less than 6 inches and then compacted in accordance with the specifications given below. Natural slopes steeper than 1 vertical to 5 horizontal to receive fill shall have horizontal benches, cut into the slopes before the placement of any fill. The width and height to be determined by the Soils Engineer. The fill shall be loosely placed in horizontal layers not exceeding 8 inches in thickness and compacted in accordance with the specifications given below. The Soils Engineer shall be responsible for determining the acceptability of soils placed. Any unacceptable soils placed shall be removed at the Contractor's expense.
- The sequence of operation in the fill areas will be fill, compact, verify acceptable soil density, and repetition of the sequence. The acceptable moisture contents during the filling operation are those at which satisfactory dry densities can be obtained. The acceptable moisture contents during the filling operation in the remaining areas are from 2 to 8 percent above the optimum moisture control.
- The surface of the fill shall be finished so that it will not impound water. If at the end of a days work it would appear that there may be rain prior to the next working day, the surface shall be finished smooth. If the surface has been finished smooth for any reason, it shall be scarified before proceeding with the placement of succeeding lifts. Fill shall not be placed on frozen ground, nor shall filling operations continue when the temperature is such as to permit the layer under placement to freeze.
- Fill and backfill should be compacted to the criteria specified in the following table:

CATEGORY	MINIMUM PERCENT COMPACTION
Fill in building areas below footings	90%
Fill under slabs, walks, and pavement	90%
Fill other than building areas	88%
Natural subgrade	88%
Pavement subgrade	90%
Pavement base course	90%

Measured as a percent of the maximum dry density as determined by modified Proctor Test (ASTM-D-1557).

Moisture content must be within 2 percent below or 4 percent above optimum moisture content if fill is deeper than 10 feet.

## GENERAL NOTES

- Underground utilities have been plotted from available information and therefore their locations shall be considered approximate only. The verification of the location of all underground utilities, either shown or not shown on these plans shall be the responsibility of the contractor, and shall be located prior to any grading or construction of the improvements.
- All manhole tops & flowlines built without elevations furnished by the Engineer will be the responsibility of the sewer contractor.
- 8" P.V.C. sanitary sewer pipe shall meet the following standards. A.S.T.M.-D-3034 SDR-35, with wall thickness compression joint A.S.T.M.-D-3212. An appropriate rubber seal waterstop as approved by the sewer district shall be installed between P.V.C. pipe and masonry structures.
- All filled places, including trench backfills, under buildings, proposed storm and sanitary sewer lines and/or paved areas, shall be compacted to 90% maximum density as determined by the "Modified AASHTO T-180 Compaction Test," (A.S.T.M.-D-1557). All filled places within public roadways shall be compacted to 95% of maximum density as determined by the "Standard Proctor Test AASHTO T-99, Method C" (A.S.T.M.-D-698).
- All trench backfills under paved areas shall be granular backfill, and shall be compacted to 90% of the maximum density as determined by the "Modified AASHTO T-180 Compaction Test," (A.S.T.M.-D-1557). All other trench backfills may be earth material (free of large clods or stones). All trench backfills shall be water jetted.
- All sanitary house connections have been designed so that the minimum vertical distance from the low point of the basement to the flow line of a sanitary sewer at the corresponding house connection is not less than the diameter of the pipe plus the vertical distance of 2 1/2 feet.
- No area shall be cleared without the permission of the Project Engineer.
- All P.V.C. sanitary sewer is to be SDR-35 or equal with clean 1/2" to 1" granular stone bedding uniformly graded. This bedding shall extend from 4" below the pipe to the springline of the pipe. Immediate backfill over pipe shall consist of same size "clean" or minus stone from springline of pipe to 6" above the top of pipe.
- All soils test shall be verified by a Soils Engineer concurrent with the grading and backfilling operations.
- Easements shall be provided for sanitary sewers, and all utilities on the Record Plat. See Record Plat for location and size of easements.
- Maintenance and upkeep of the common ground area shall be the responsibility of the developer and/or successors.
- A 25' building line shall be established along all Public Rights-Of-Way.
- All water lines shall be laid at least 10 feet horizontally, from any sanitary sewer, storm sewer, or manhole. 18" vertical clearance from outside of pipe to outside of pipe shall be maintained wherever water lines must cross sanitary sewers, laterals, or storm drains the water line shall be laid at such an elevation that the bottom of the water line is above the top of the drain or sewer. A full length of water pipe shall be centered over the sewer line to be crossed so that the joints will be equally distant from the sewer and as remote therefrom as possible. This vertical separation shall be maintained for that portion of the water line located within 10 feet horizontally, of any sewer or drain it crosses.
- All PVC water pipe shall conform to A.S.T.M.-D-2241, SDR 21 Standard Specification for P.V.C. Pressure Pipe, 200 P.S.I. working pressure for water, with approved joint.
- Water lines, valves, sleeves, meters, and fittings shall meet all specifications and installation requirements of Public Water Supply District No. 2 of St. Charles County.
- All water hydrants and valves shall be ductile iron and installed in accordance with plans and details. All ductile iron pipe for water mains shall conform to A.W.W.A. Specifications C-106 and/or C-108. The ductile iron fittings shall conform to A.W.W.A. Specification CC-110. All rubber gasket joints for water ductile iron pressure pipe and fittings shall conform to A.W.W.A. Specification C-111.
- All sanitary manholes shall be waterproofed on the exterior in accordance with Missouri Department of Natural Resources specifications 10 CSR-B120 (7)E.
- Brick will not be used in the construction of sanitary sewer manholes.
- All pipes shall have positive drainage through manholes. No flat base structures are allowed.
- The City of O'Fallon and Duckett Creek Sanitary District shall be notified 48 hours prior to construction for coordination and inspection.
- Gas, water and other underground utilities shall not conflict with the depth or horizontal location of existing or proposed sanitary or storm sewers, including house laterals.
- All existing site improvements disturbed, damaged or destroyed shall be repaired or replaced to closely match pre-construction conditions.
- The contractor shall prevent all storm, surface water, mud and construction debris from entering the existing sanitary sewer system.
- All construction and materials shall conform to the current construction standards of the City of O'Fallon and Duckett Creek Sanitary District.
- All sanitary and storm sewer trench backfills shall be water jetted. Granular backfill will be used under pavement areas.
- All existing areas disturbed during construction of the off-site sanitary sewer line shall be seeded and mulched to prevent erosion.
- All sanitary sewer laterals shall be a minimum of 4" in diameter per City of O'Fallon.
- Concrete pipe for storm sewers shall be Class III, A.S.T.M. C-76 with a minimum diameter of 12" except in the R.O.W. it shall be 15".
- The ADS N-12 pipe shall have a smooth interior wall.
- Concrete pipe joints shall be MSD type "A" approved compression-type joints and shall conform to the requirements of the specifications for joints for circular concrete sewer and culvert pipe, using flexible, watertight, rubber-type gaskets (A.S.T.M.-C-443). Band-type gaskets depending entirely on cement for adhesion and resistance to displacement during jointing shall not be used.
- When HDPE pipe is used, City of O'Fallon specifications or manufacturers specifications, which ever are more stringent, shall be followed.
- The use of High Density Polyethylene Corrugated pipe, ADS N-12 or equal will be permitted as an acceptable alternative to reinforced concrete pipe, ADS N-12 HC shall be used for all ADS pipe greater than 36". Pipe shall meet A.S.T.M.-D-2321 and A.A.S.H.T.O. M-294-291.
- All flared end sections and inlet structures will be concrete.
- All storm sewer pipe installed in the Public Right-of-Way shall be Reinforced concrete Class III pipe.
- All concrete pipe or ADS N-12 pipe shall be installed with "O-Ring" Rubber type gaskets per M.S.D. standard construction specifications or manufacturer.
- Blow-off hydrants and water meters shall not be located in any pavement or hard-surfaced area including, but not limited to, driveways, sidewalks, and streets. Since the location of all such areas is not shown on this plan all costs to relocate any blow-off hydrants and water meters from any pavement or hard-surfaced areas shall be borne by the Developer or the Builders.
- All creek crossings shall be grouted rip-rap as directed by District inspectors. (All grout shall be high slump ready-mix concrete.)
- Existing sanitary sewer service shall not be interrupted.
- Pre-manufactured adapters shall be used at all PVC to DIP connections. Rubber boot / Mission-type couplings will not be allowed.
- Any permits, licenses, easements, or approvals required to work on public or private properties or roadways are the responsibility of the developer.
- No slopes shall exceed 3(H) : 1(V).
- Driveway locations shall not interfere with the sidewalk handicap ramps in the cul-de-sacs.
- All sign posts and hardware to be painted black, per City of O'Fallon requirements.
- No parking on one side of Fallon parkway, as shown on Sheets 2, per City of O'Fallon requirements. City of O'Fallon to verify locations.

NOTE:  
INSPECTION MUST BE 90% COMPLETED  
AND PASSED ON EXISTING SEWERS PRIOR  
TO NEW DEVELOPMENT CONNECTION.

## REFERENCE BENCHMARK

R.M. #74 - ELEV=493.07 (U.S.G.S. DATUM)  
CHISELED SQUARE ON TOP OF EAST CONCRETE HEADWALL  
OF BIRDIE HILLS ROAD BRIDGE OVER TRIBUTARY NO. 2  
(APPROXIMATELY 500 FEET SOUTH OF EISENHOWER DRIVE)

## SITE BENCHMARK

ELEV=572.28 (U.S.G.S. DATUM)  
OLD CROSS CL-CL SWEETBAY DRIVE AND CHERRYWOOD PARC DRIVE  
CHERRYWOOD PARK SUBDIVISION



PENNIAL PARK, PHASE 3  
KAPLAN DEVELOPMENT COMPANY  
5140 NORTH SERVICE ROAD  
ST. PETERS, MISSOURI 63376  
PREPARED FOR:  
636-397-4471

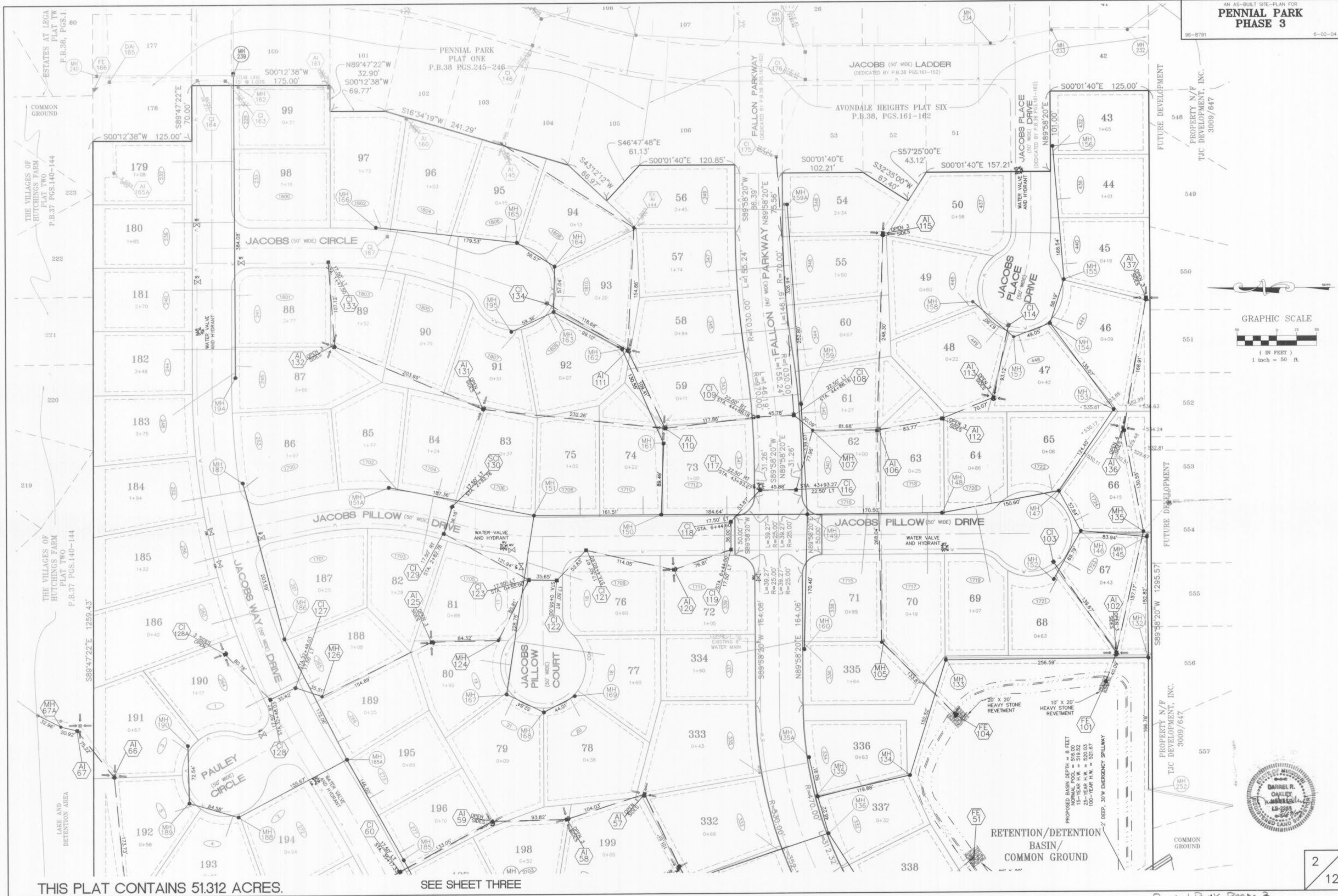
DISCLAIMER OF RESPONSIBILITY  
I hereby certify that the documents intended to be authorized by my seal are limited to this sheet, and I hereby disclaim any responsibility for all other drawings, specifications, estimates, reports or other documents or instruments relating to or intended to be used for any part or parts of the architectural or engineering project or survey.

REVISIONS	DATE	CITY COMMENTS
3-9-07		

ENGINEERING  
PLANNING  
SURVEYING  
221 Point West Blvd.  
St. Charles, MO 63301  
636-928-5552  
FAX 928-1718

6-02-04	DATE
97-8791	PROJECT NUMBER
1 OF 12	SHEET OF
8791ASBP3.DWG	FILE NAME
ECF	DRAWN
DRO	DESIGNED
CHECKED	

Pennial Park Phase 3  
As built 1/12



THIS PLAT CONTAINS 51.312 ACRES.

SEE SHEET THREE

Pennial Park Phase 3  
As built 2/12

AN AS-BUILT SITE-PLAN FOR  
**PENNIAL PARK  
 PHASE 3**

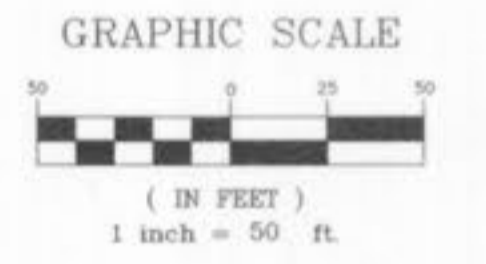
96-8791 8-02-04

SEE SHEET TWO

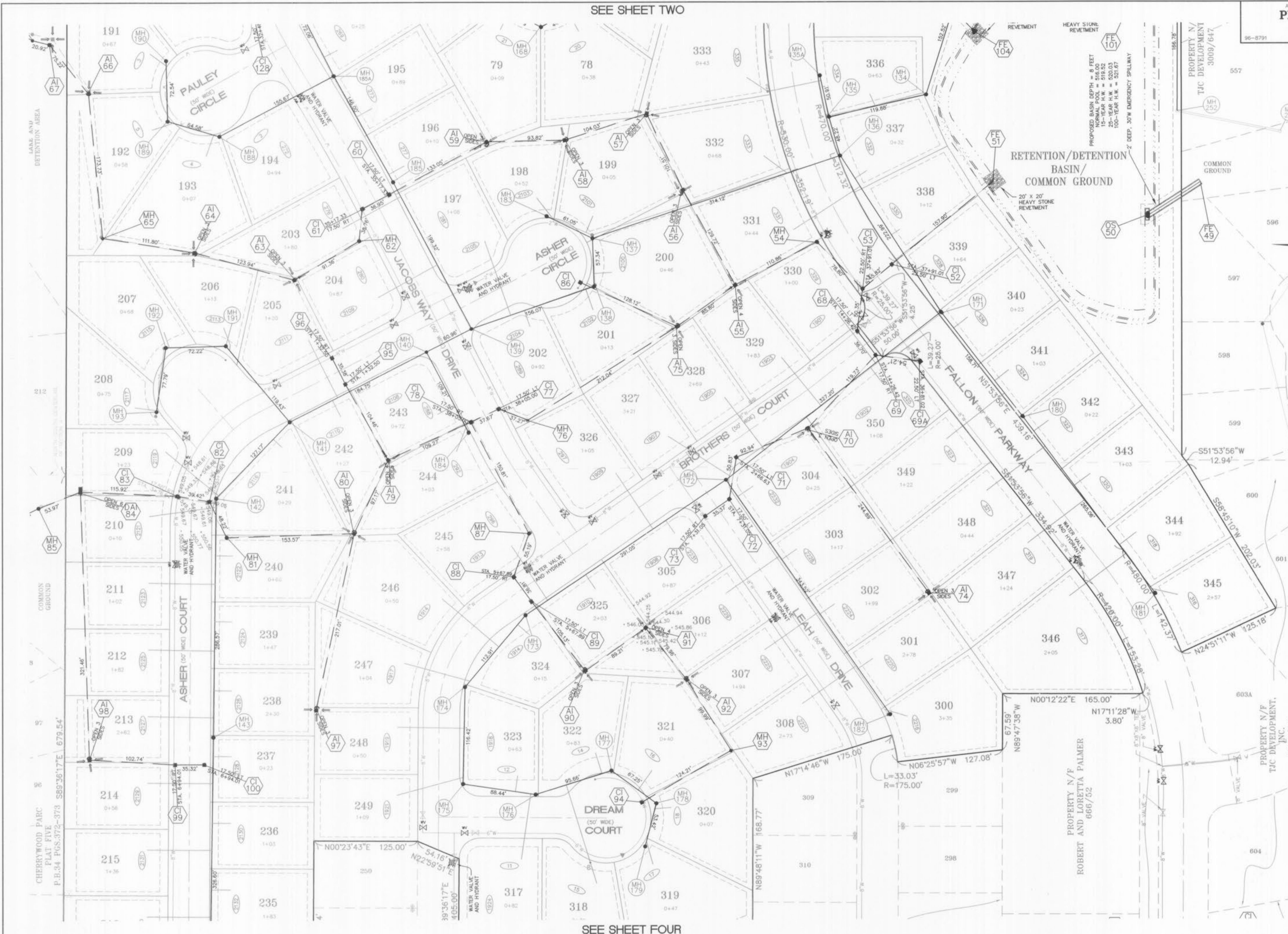
SEE SHEET FOUR

PROPOSED BASIN DEPTH = 6 FEET  
 NORMAL POOL = 516.00  
 25-YEAR H.W. = 520.03  
 100-YEAR H.W. = 521.87  
 2' DEEP, 30"W EMERGENCY SPILLWAY

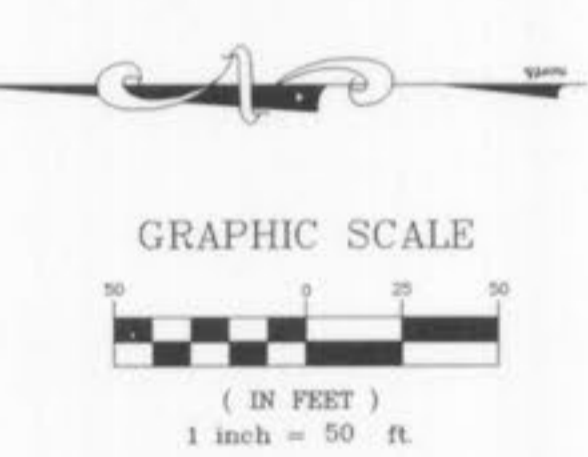
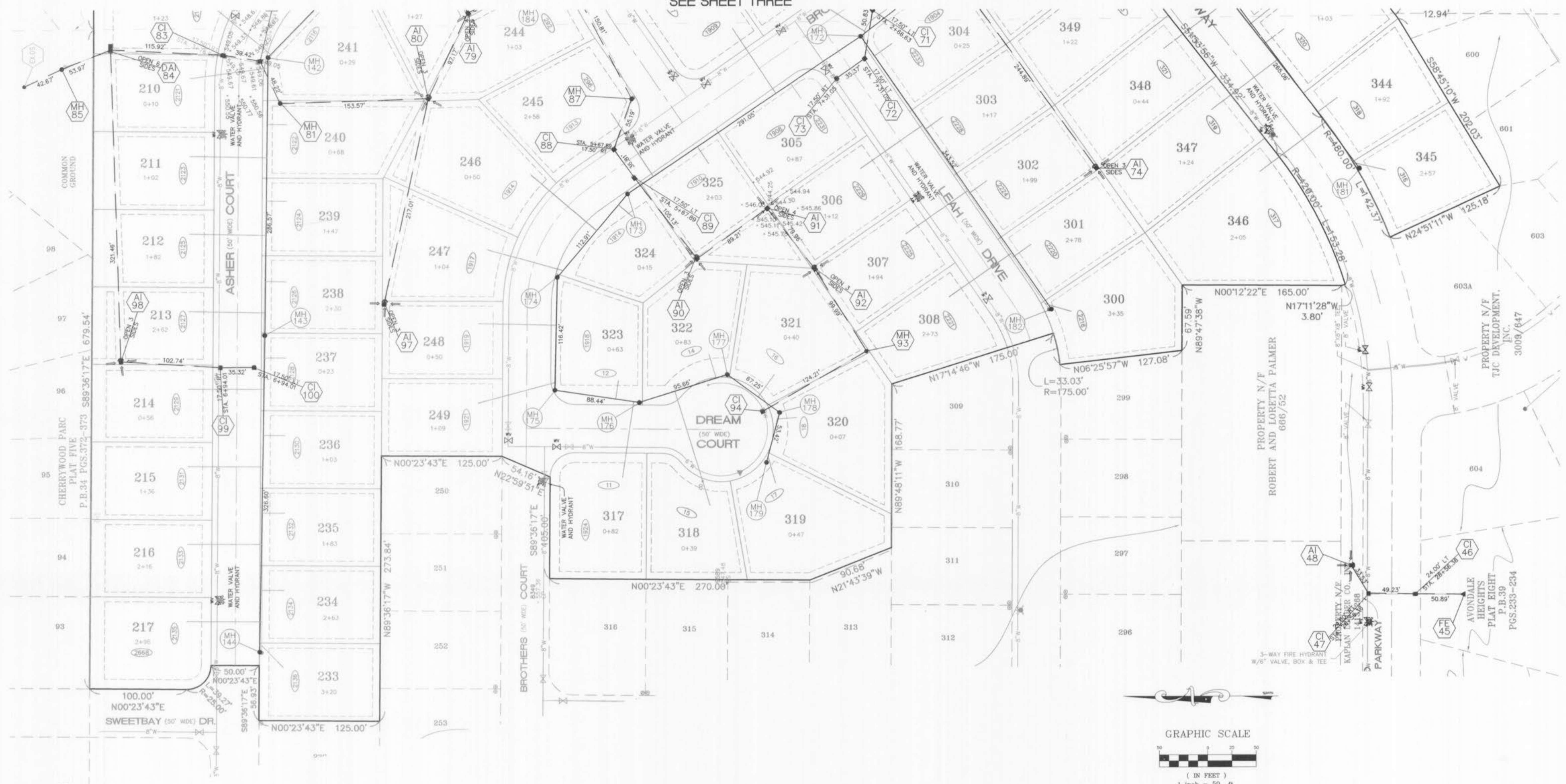
RETENTION/DETENTION  
 BASIN/  
 COMMON GROUND

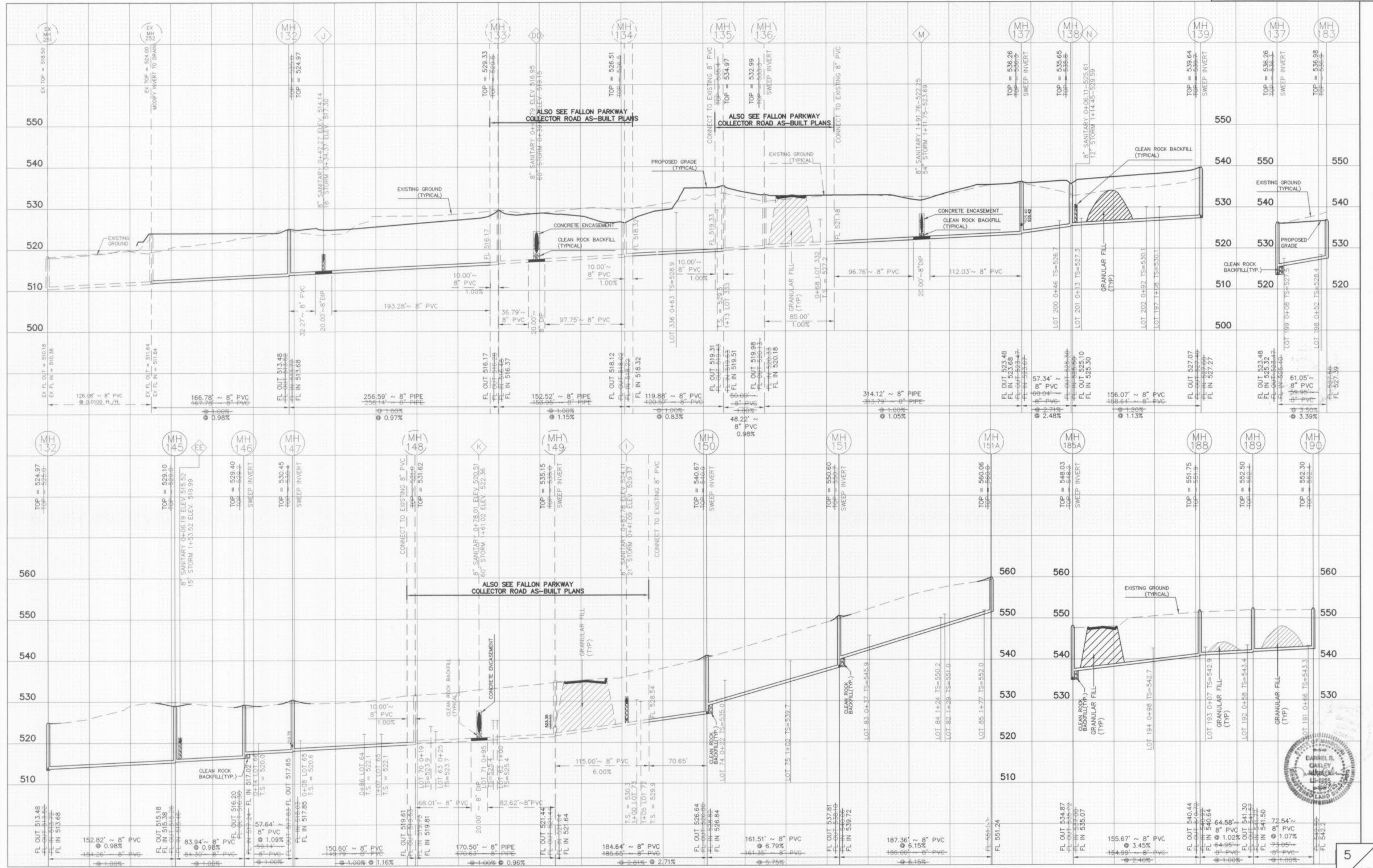


Pennial Park Phase 3  
 Asbuilt 3/12



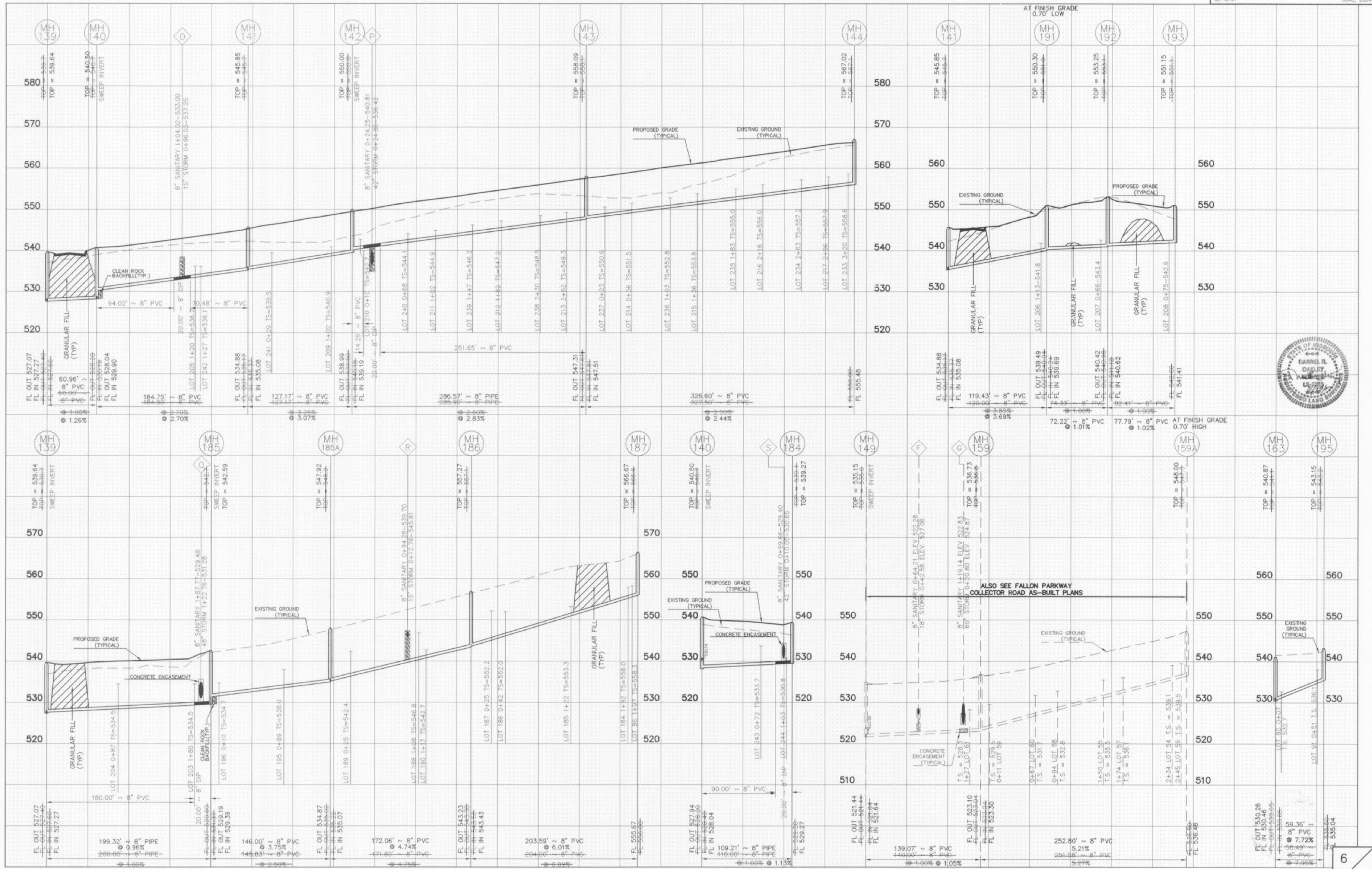
SEE SHEET THREE





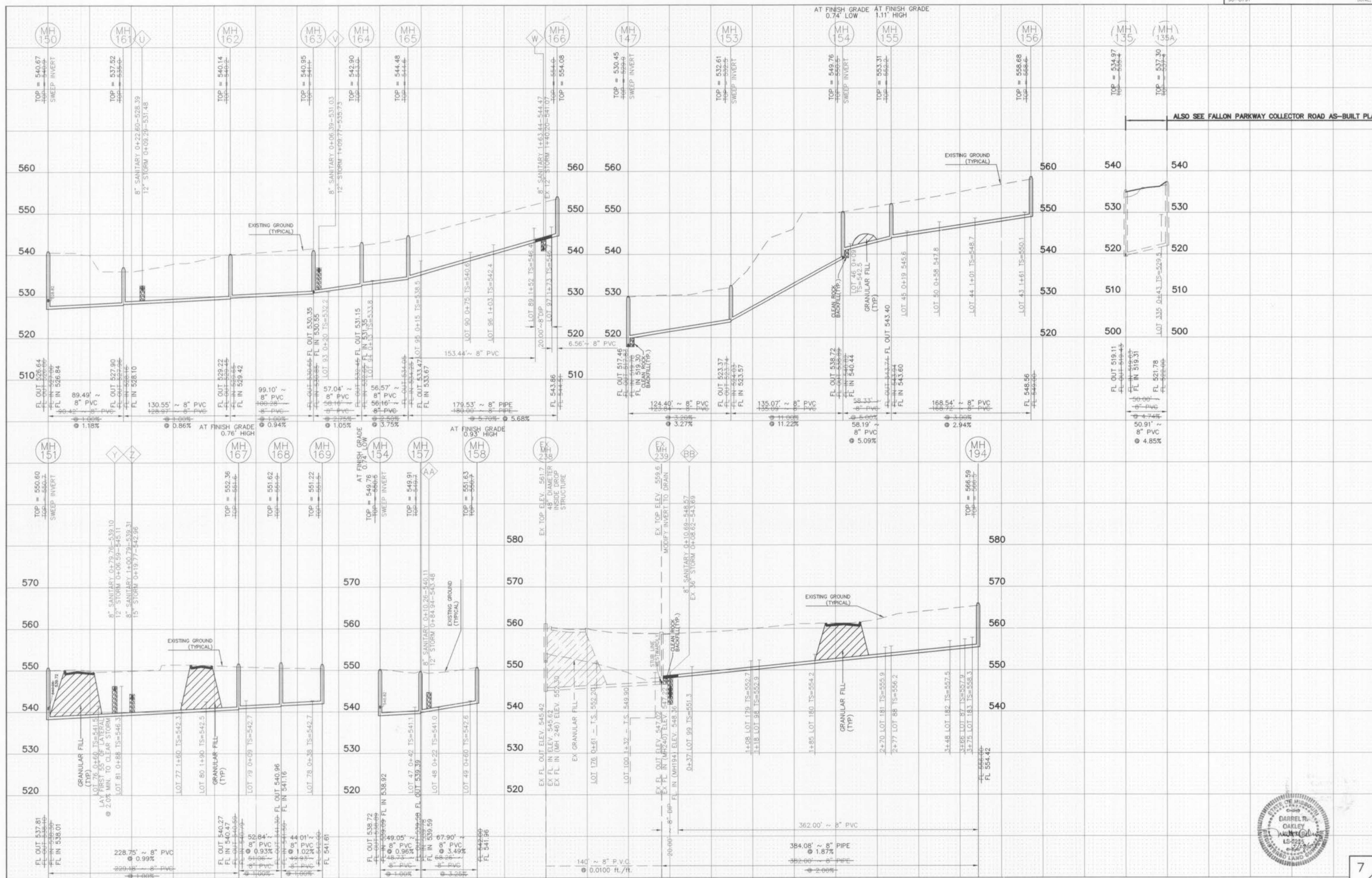
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Pennial Park Phase 3  
 As built 5/12



SCALE: VERTICAL = 10  
HORIZONTAL = 50

Pennial Park Phase 3  
As built 6/12



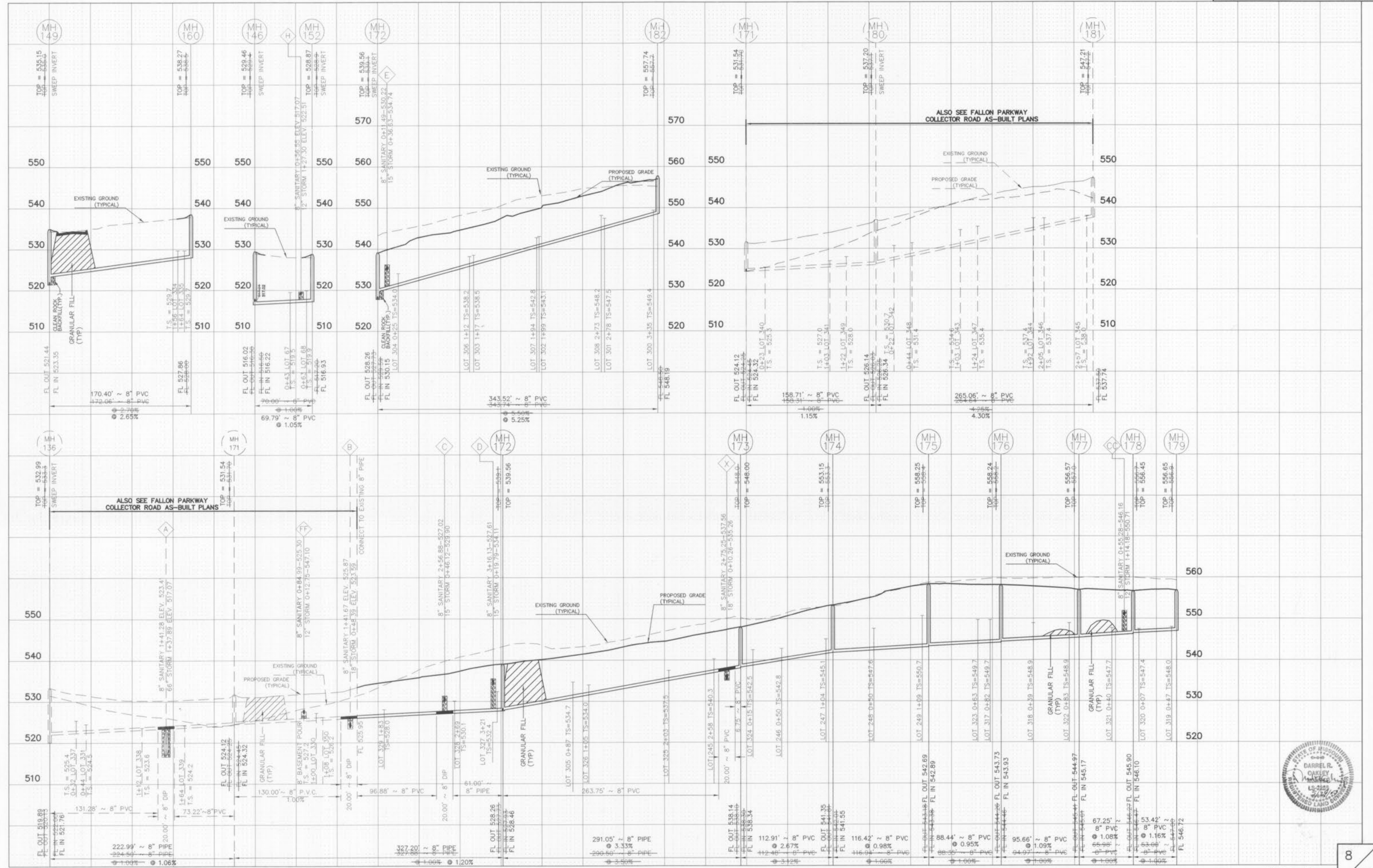
ALSO SEE FALLON PARKWAY COLLECTOR ROAD AS-BUILT PLANS



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 HORIZONTAL = 50

7  
 12

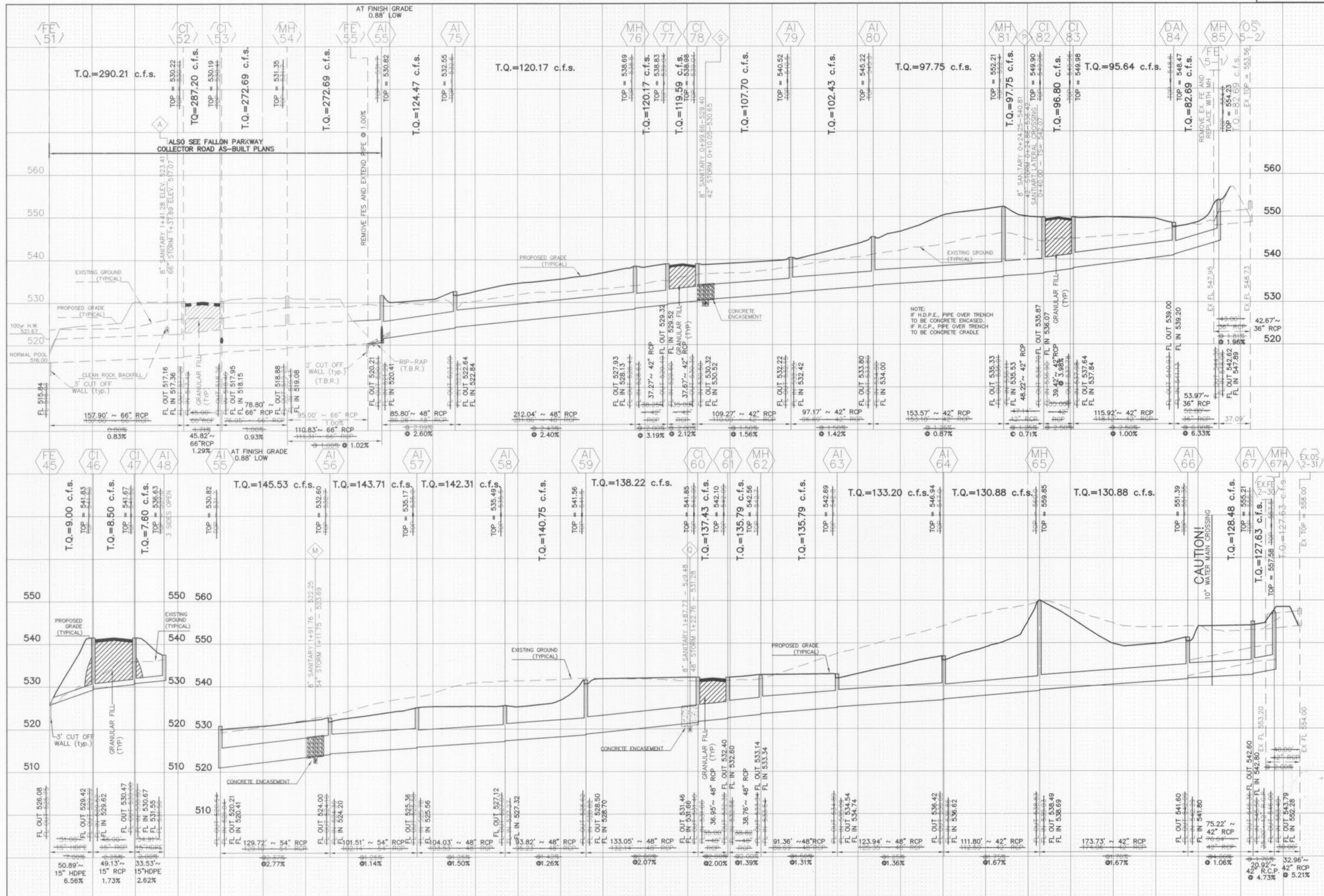
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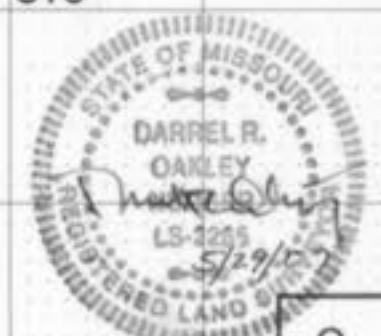
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 As bu lts

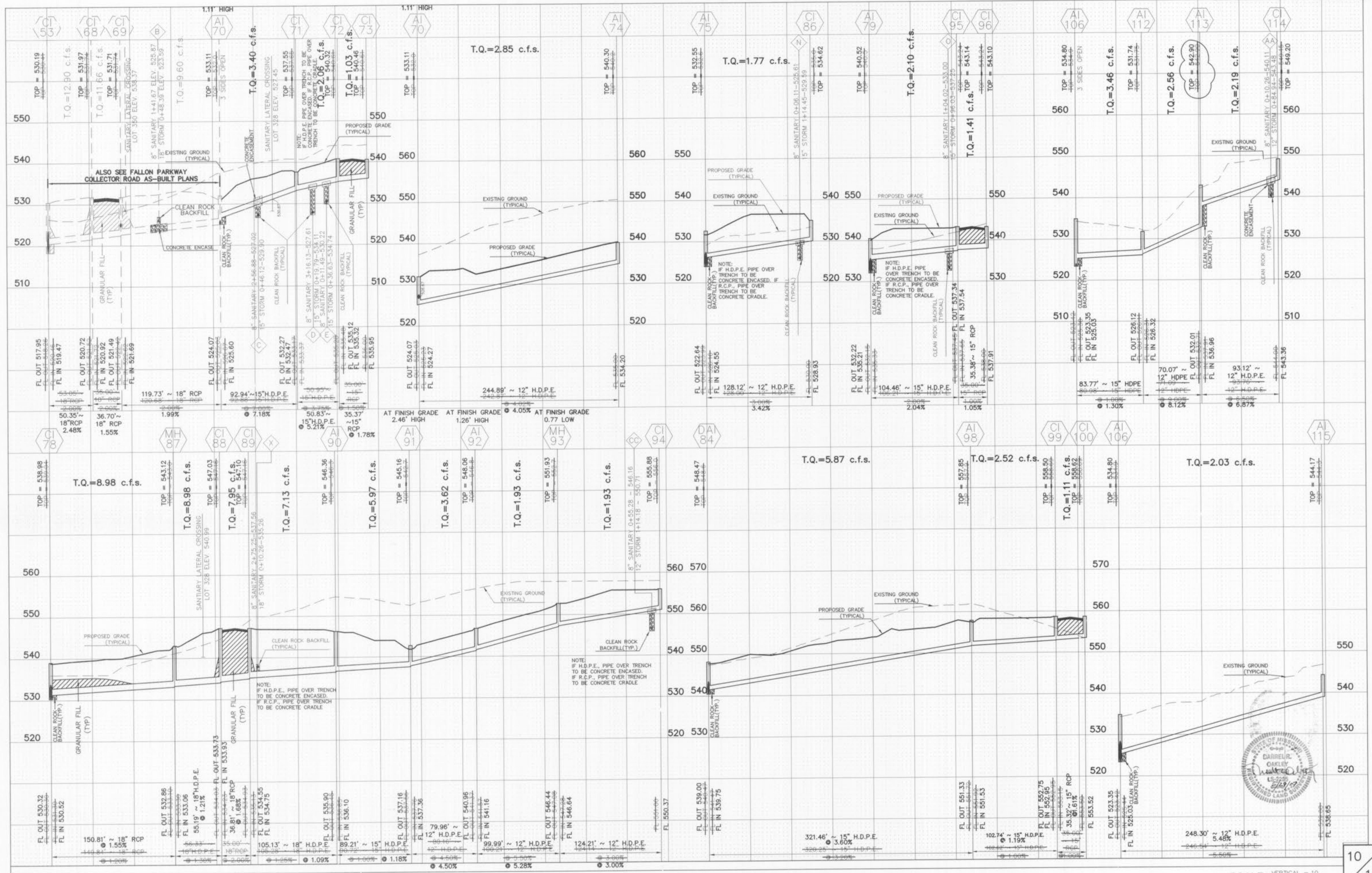




Pennial Park Phase 3  
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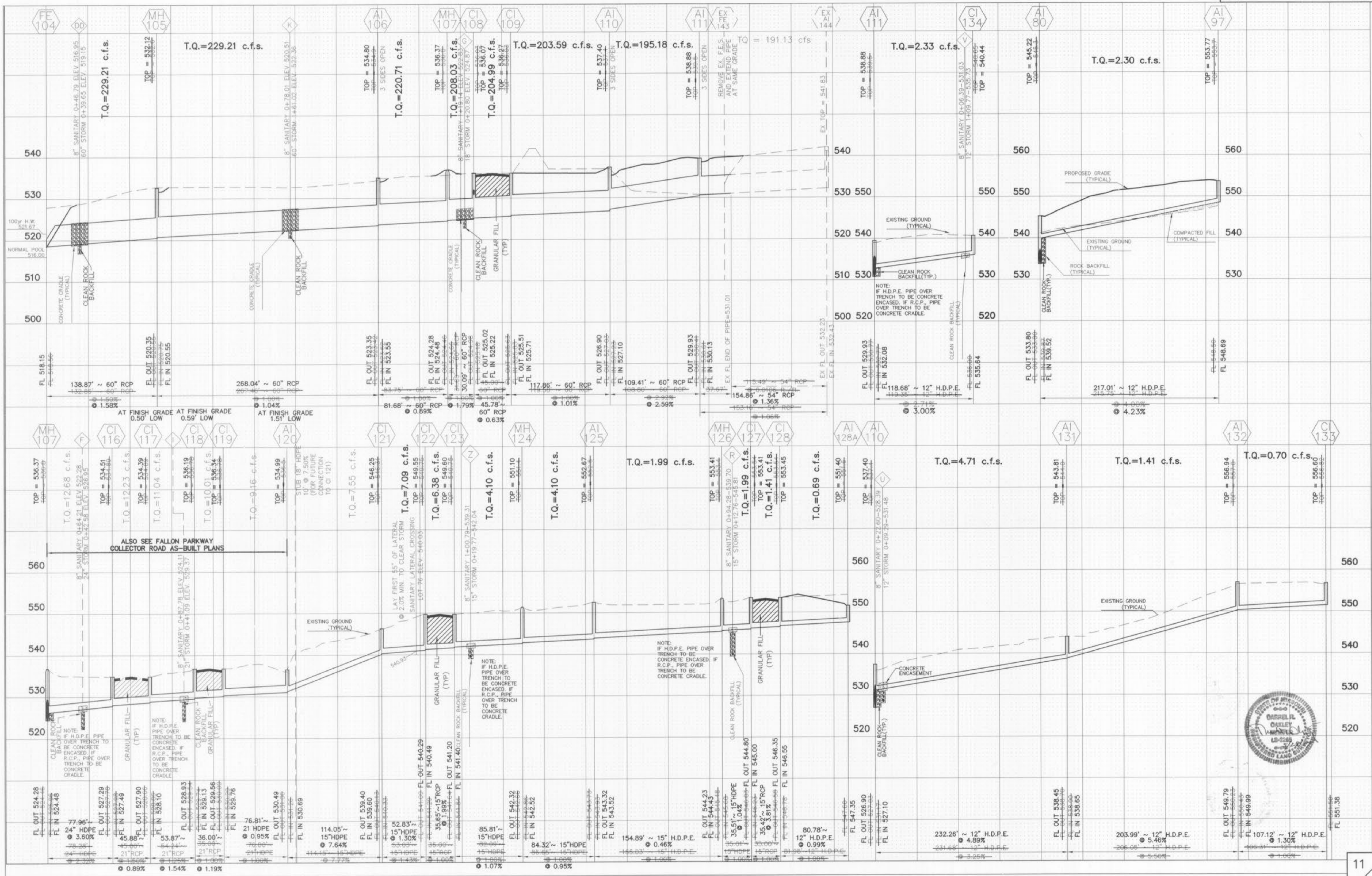
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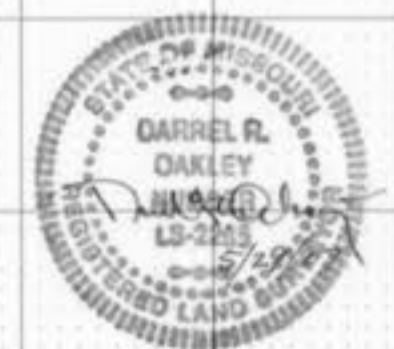
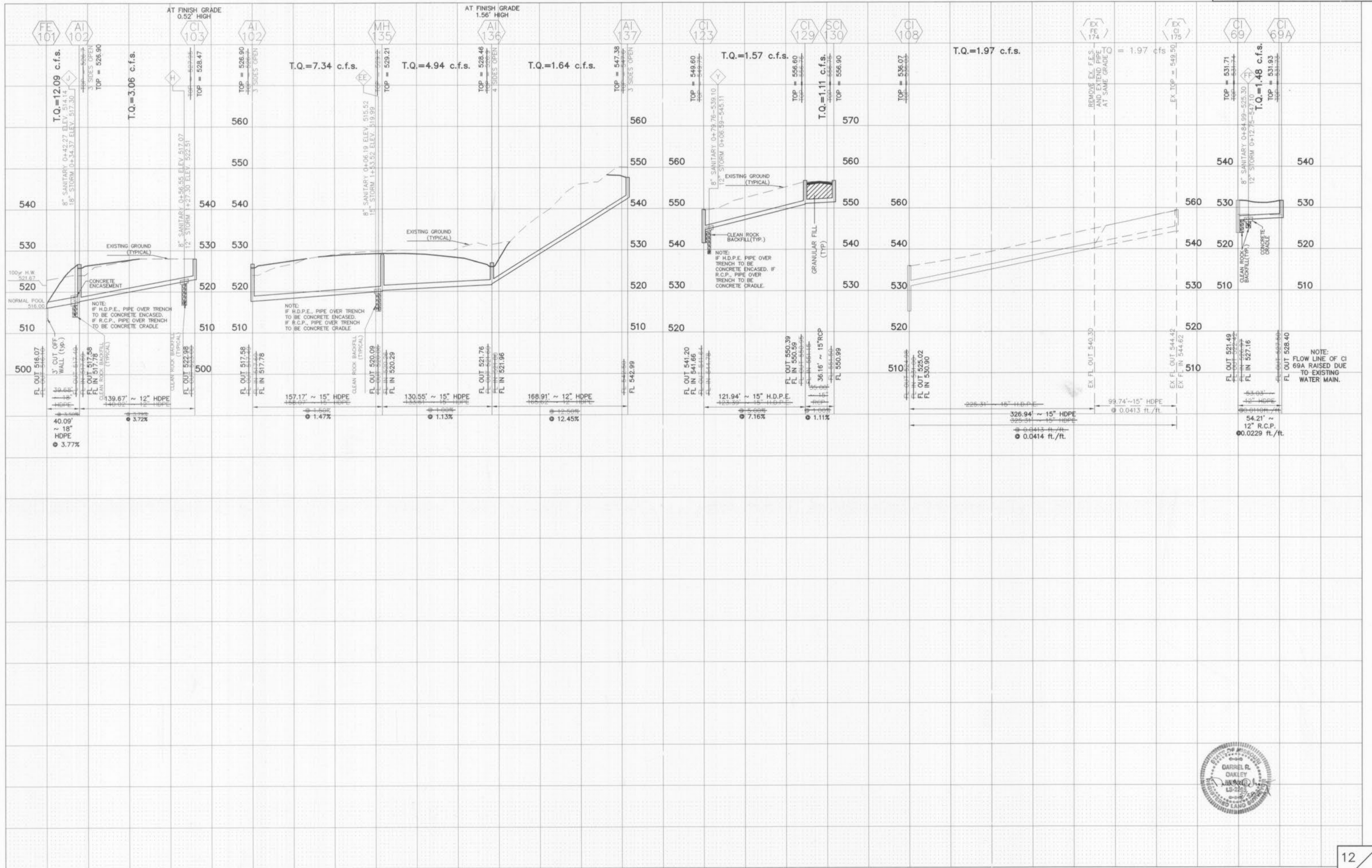
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Pennial Park Phase 3  
 As built 10/12



SCALE: VERTICAL = 10  
 HORIZONTAL = 50

Pennial Park Phase 3  
 As built 11/12



SCALE: VERTICAL = 10  
 HORIZONTAL = 50

Pennial Park Phase 3  
 As built 12/12