A SET OF ASBUILT PLANS FOR
WILLOW WALK ESTATES PHASE 1

TRACTS OF LAND IN U.S. SURVEY 418, AND IN
FRACTIONAL SECTION 17, TOWNSHIP 46 NORTH,
RANGE 2 EAST OF THE FIFTH PRINCIPAL
MERIDIAN ST. CHARLES COUNTY, MISSOURI

LOCATION MAP
NOT TO SCALE

GRADING NOTES

1. A Geotechnical Engineer shall be employed by the owner and be on site during grading operations. All soils tests shall be verified by the Geotechnical Engineer concurrent with the grading and backfilling operations.
2. The grading contractor shall perform a complete grading and compaction operation as shown on the plans, stated in these notes, or reasonably implied there from, all in accordance with the plans and notes as interpreted by the Geotechnical Engineer.
3. The Contractor shall notify the Soils Engineer at least two days in advance of the start of the grading operation.
4. All areas shall be allowed to drain. All low points shall be provided with temporary ditches.
5. All filled places, including trench backfills, under buildings, proposed storm and sanitary sewer lines, proposed roads and/or paved areas, shall be compacted to 90% of maximum density as determined by the "Modified AASHTO T-180 Compaction Test," (A.S.T.M.-D-1557), or 95% of maximum density as determined by the Standard Proctor Test AASHTO T-99. All filled places within public roadways shall be compacted from the bottom of the fill up to 90% maximum density as determined by the Modified AASHTO T-180 Compaction Test or 95% of maximum density as determined by the Standard Proctor Test AASHTO T-99, Method "C" (A.S.T.M.-D-698). All test shall be verified by a soils engineer concurrent with grading and backfilling operations. Ensure the moisture content of the soil in fill areas is to correspond to the compactive effort as defined by the Standard or Modified Proctor Test. Optimum moisture content shall be determined using the same test that was used for compaction. Soil compaction curves shall be submitted to the City of O'Fallon prior to the placement of fill. Proof rolling may be required to verify soil stability at the discretion of the City Of O'Fallon.
6. A sediment control plan that includes monitored and maintained sediment control basins and/or straw bales should be implemented as soon as possible. No graded area is to be allowed to remain bare without being seeded and mulched. Care should be exercised to prevent soil from damaging adjacent property and silting up existing downstream storm drainage systems. All erosion control systems shall be inspected and necessary corrections made within 24 hours of any rain storm resulting in 1/2 inch of rain or more.
7. Debris and foundation material from any existing on-site building or structure which is scheduled to be razed for this development must be disposed of off-site.
8. Trees, organic debris, rubble, foundations and other deletrious material shall be removed for the site and disposed in compliance with all applicable laws and regulations. Landfill tickets for such disposal shall be maintained on file by the developer. Burning on site shall be allowed only by permit from the local fire district. If a burn pit is proposed the location and mitigation shall be shown on the grading plan and documented by the soils engineer.
9. Soft soil in the bottom and banks of any existing or former pond sites or tributaries or on any sediment basins or traps should be removed, spread out and permitted to dry sufficiently to be used as fill. None of this material should be placed in proposed public right-of-way locations or on any storm sewer locations.
10. Site preparation includes the clearance of all stumps, trees, bushes, shrubs, and weeds; the grubbing and removal of roots and other surface obstructions from the site; and the demolition and removal of any man-made structures. The material shall be properly disposed of off-site. Topsoil and grass in the fill areas shall be thoroughly disced prior to the placement of any fill. The Soils Engineer shall approve the discing operation.
11. Compaction equipment shall consist of tamping rollers, pneumatic-tired rollers, vibratory roller, or high speed impact type drum rollers acceptable to the Soils Engineer. The roller shall be designed so as to avoid the creation of a layered fill without proper blending of successive fill layers.
12. The Soils Engineer shall observe and test the placement of the fill to verify that specifications are met. A series of fill density tests will be determined on each lift of fill. Interim reports showing fill quality will be made to the Owner at regular intervals.
13. The Soils Engineer shall notify the Contractor of rejection of a lift of fill or portion thereof. The Contractor shall rework the rejected portion of fill and obtain notification from the Soils Engineer of its acceptance prior to the placement of additional fill.
14. All areas to receive fill shall be scarified to a depth of not less than 6 inches and then compacted in accordance with the specifications given below. Natural slopes steeper than 1 vertical to 5 horizontal to receive fill shall have horizontal benches cut into the slopes before the placement of any fill. The width and height to be determined by the Soils Engineer. The fill shall be loosely placed in horizontal layers not exceeding 8 inches in thickness and compacted in accordance with the specifications given below. The Soils Engineer shall be responsible for determining the acceptability of soils placed. Any unacceptable soils placed shall be removed at the Contractor's expense.
15. The sequence of operation in the fill areas will be fill, compact, verify acceptable soil density, and repetition of the sequence. The acceptable moisture contents during the filling operation are those at which satisfactory dry densities can be obtained. The acceptable moisture contents during the filling operation in the remaining areas are from 2 to 8 percent above the optimum moisture control.
16. The surface of the fill shall be finished so that it will not impound water. If at the end of a days work it would appear that there may be rain prior to the next working day, the surface shall be finished smooth. If the surface has been finished smooth for any reason, it shall be scarified before proceeding with the placement of succeeding lifts. Fill shall not be placed on frozen ground, nor shall filling operations continue when the temperature is such as to permit the layer under placement to freeze.
17. Developer must supply City construction inspectors with soil reports prior to or during site soil testing.
18. Fill and backfill should be compacted to the criteria specified in the following table:

CATEGORY	MINIMUM PERCENT COMPACTION
Fill in building areas below footings	90%
Fill under slabs, walks, and pavement	90%
Fill other than building areas	88%
Natural subgrade	88%
Pavement subgrade	90%
Pavement base course	90%

Measured as a percent of the maximum dry density as determined by modified Proctor Test (ASTM-D-1557).

Moisture content must be within 2 percent below or 4 percent above optimum moisture content if fill is deeper than 10 feet.

GENERAL NOTES

1. Underground utilities have been plotted from available information and therefore their locations shall be considered approximate only. The verification of the location of all underground utilities, either shown or not shown on these plans shall be the responsibility of the contractor, and shall be located prior to any grading or construction of the improvements.
2. All trench backfills under paved areas shall be granular backfill, and shall be Modified compacted to 90% of the maximum density as determined by the "AASHTO T-180 Compaction Test," (A.S.T.M.-D-1557). All other trench backfills may be earth material (free of large clods or stones). All trench backfills shall be water jetted.
3. No area shall be cleared without the permission of the Project Engineer.
4. All P.V.C. sanitary sewer is to be SDR-35 or equal with clean 1/2" to 1" granular stone bedding uniformly graded. This bedding shall extend from 4" below the pipe to the springline of the pipe. Immediate backfill over pipe shall consist of same size "clean" or minus stone from springline of pipe to 12" above the top of pipe.
5. All soils test shall be verified by a Soils Engineer concurrent with the grading and backfilling operations.
6. Easements shall be provided for sanitary sewers, and all utilities on the Record Plat. See Record Plat for location and size of easements.
7. Maintenance and upkeep of the common ground area shall be the responsibility of the developer and/or successors.
8. The City of O'Fallon shall be notified 48 hours prior to construction for coordination and inspection.
9. All storm inlets must be installed with a 5/8" trash bar across the opening.
10. Concrete pipe for storm sewers shall be Class III, A.S.T.M. C-76 with a minimum diameter of 12" except in the R.O.W. it shall be 15".
11. The ADS N-12 pipe shall have a smooth interior wall.
12. Concrete pipe joints shall be MSD type "A" approved compression-type joints and shall conform to the requirements of the specifications for joints for circular concrete sewer and culvert pipe, using flexible, watertight, rubber-type gaskets (A.S.T.M.-C-443). Band-type gaskets depending entirely on cement for adhesion and resistance to displacement during jointing shall not be used.
13. When HDPE pipe is used, City of O'Fallon specifications or manufacturers specifications, which ever are more stringent, shall be followed.
14. The use of High Density Polyethylene Corrugated pipe, ADS N-12WT or equal will be permitted as an acceptable alternative to reinforced concrete pipe. Pipe shall meet meet A.S.T.M. F1417 water tight field test.
15. All flared end sections and inlet structures will be concrete.
16. All storm sewer pipe installed in the Public Right-of-Way shall be Reinforced concrete Class III pipe.
17. All concrete pipe or ADS N-12 pipe shall be installed with "O-Ring" Rubber type gaskets per M.S.D. standard construction specifications or manufacturer.
18. All utilities shall be located underground.
19. Storm and sanitary sewer pipe place at less than 1% slope shall have field verification of pipe slope before backfilling.
20. Any permits, licenses, easements, or approvals required to work on public or private properties or roadways are the responsibility of the developer.
21. No slopes shall exceed 3(H):1(V).
22. Driveway locations shall not interfere with the sidewalk curb ramps.
23. City approval of the Construction plans does not mean that Single Family dwelling units can be constructed on lots without meeting the minimum building setbacks as required by the Zoning Code.
24. Sidewalks and sidewalk curb ramps shall be constructed in accordance with the current approved "Americans with Disabilities Act Accessibility Guidelines" (A.D.A.A.G.). If any conflict occurs between the above information and the plans the A.D.A.A.G. shall take precedence and the contractor prior to any construction shall notify the Project Engineer.
25. Contractor shall assume complete responsibility for controlling all siltation and erosion of the project area. The contractor shall use whatever means necessary to control erosion and siltation including, but not limited to, staked straw bales and/or siltation fabric fences (possible methods of control are detailed in the plan). Control shall commence with grading and be maintained throughout the project until acceptance of the work by the owner and/or the City of O'Fallon. The contractors responsibilites include all design and implementation as required to prevent erosion and the depositing of silt. The Owner and/or the City of O'Fallon may at their option direct the contractor in his methods as deemed fit to protect property and improvements. Any depositing of silts or mud on new or existing pavement or in new or existing storm sewers or swales shall be removed after each rain and affected areas cleaned to the satisfaction of the Owner and/or City of O'Fallon and/or MoDOT.
26. All paving to be in accordance with St. Charles County Standards and Specifications except as modified by the City of O'Fallon ordinances.
27. All sign post, backs, bracket arms, street signs and traffic signals shall be painted black using Carboline Rustbond Penetrating Sealer SG and Carboline 133 HB paint (or equivalent as approved by The City of O'Fallon and/or MoDOT.)
28. Any proposed pavillions or playground areas will need a separate permit from the building division.
29. All sign locations and sizes must be approved separately through the Planning and Engineering Departments.
30. Brick shall not be used in the construction of storm sewer structures.
31. Provide a marking on the storm sewer inlets. The City will allow the following markers and adhesive procedures only as shown in the table below or an approved equal by Almetek Industries. "Peel and stick" adhesive pads will not be allowed.

Manufacturer	Size	Adhesive	Style	Message (Part #)	Website
ACP International	3 7/8"	Epoxy	Crystal Cap	No Dumping Drains To Waterways (SD-W-CC)	www.acpinternational.com
DAS Manufacturing, Inc.	4"	Epoxy	Standard Style	No Dumping Drains To Stream (#SDS)	www.dasmanufacturing.com

C.I. CURB INLET
D.C.I. DOUBLE CURB INLET
A.I. AREA INLET
M.H. MANHOLE
F.E. FLARED END SECTION
E.P. END PIPE
C.P. CONCRETE PIPE
R.C.P. REINFORCED CONCRETE PIPE
C.M.P. CORRUGATED METAL PIPE
C.I.P. CAST IRON PIPE
P.V.C. POLY VINYL CHLORIDE (PLASTIC)
C.O. CLEAN OUT

FIRE HYDRANT
STORM SEWER
SANITARY SEWER

STREET LIGHT
582 EXISTING CONTOUR
582 PROPOSED CONTOUR
STREET SIGN
NO PARKING SIGN
WATER VALVE
B.O. BLOW OFF ASSEMBLY
FLOWLINE ELEVATION OF HOUSE CONNECTION
FLOWLINE ELEVATION OF SEWER MAIN

ASBUILTS ADDED SEPTEMBER, 2007

NOT TO SCALE

REFERENCE BENCHMARK

REFERENCE BENCHMARK ELEVATION 544.08
CHISELED SQUARE ON THE SOUTHWEST CORNER OF RETAINING WALL ON NORTHWEST CORNER OF HOPEWELL ROAD BRIDGE OVER DARDENNE CREEK, NORTH OF THE INTERSECTION OF HOPEWELL ROAD AND HOFFMAN ROAD

SITE BENCHMARK

OLD STONE AT THE NORTHWEST BOUNDARY CORNER AND THE SOUTHWEST CORNER OF LOT 18 OF FALLING LEAF FARMS PLAT TWO PB 22, Pg. 92.
ELEV. - 621.65 (U.S.G.S.)

TYPICAL LOT SIZES
NOT TO SCALE

PUBLIC STREET

P.W.S.D. # 2 WATER NOTES

1) The St. Charles County Public Water Supply District No. 2 shall be notified at least 48 hours prior to construction for coordination of inspection.
2) All PVC water pipe shall conform to A.S.T.M.-D-2241, SDR 21 Standard Specification for P.V.C. Pressure Pipe, 200 P.S.I. working pressure for water, with approved joint. P.V.C. pipe used for waterlines is to be certified by NSF, listed in NSF Standard 61, and have the NSF logo stamped on the pipe.
3) All Water mains, valves, hydrants and related items are to be installed in accordance with the current St. Charles County Public Water Supply District No. 2 guidelines and specifications as approved by MDNR on review number 61996-04R.
4) All Water hydrants and valves shall be ductile iron and installed in accordance with the plans and details. All ductile iron pipe for water mains shall conform to A.W.W.A. Specification C-151 and be cement lined and seal coated in accordance with A.W.W.A. Specification C-104. The ductile iron fittings shall conform to A.W.W.A. Specification C-153. All joints for water ductile iron pressure pipe shall be push on type with rubber gasket conforming to A.W.W.A. Specification C-111 and fittings shall be ductile iron, Class 350, conforming to A.W.W.A. Specification C-153.
5) All water lines shall be laid at least 10 feet horizontally, from any sanitary sewer, storm sewer, or manhole. 18" vertical clearance from outside of pipe to outside of pipe shall be maintained wherever water lines must cross sanitary sewers, laterals, or storm drains the water line shall be laid at such an elevation that the bottom of the water line is above the top of the drain or sewer. A full length of water pipe shall be centered over the sewer line to be crossed so that the joints will be equally distant from the sewer and as remote therefrom as possible. This vertical separation shall be maintained for that portion of the water line located within 10 feet horizontally, of any sewer or drain it crosses.
6) Blow-off hydrants and water meters shall not be located in any pavement or hard surfaced area including, but not limited to, driveways, sidewalks, and streets. Since the location of all such areas is not shown on this plan all costs to relocate any blow-off hydrants and water meters from any pavement or hard surfaced areas shall be borne by the Developer or the Builders.

ALL WATER SYSTEM EXTENSIONS SHALL CONFORM TO THE WATER SYSTEM SPECIFICATIONS OF ST. CHARLES COUNTY PUBLIC WATER SUPPLY DISTRICT NO. 2.
DNR REVIEW #61996-04R.

DUCKETT CREEK SANITARY DISTRICT CONSTRUCTION NOTES

1. Underground utilities have been plotted from available information and, therefore, location shall be considered approximate only. The verification of the location of all underground utilities, either shown or not shown on these plans, shall be the responsibility of the contractor and shall be located prior to any grading or construction of improvements.
2. Gas, water and other underground utilities shall not conflict with the depth or horizontal location of existing or proposed sanitary and storm sewers, including house laterals.
3. All existing site improvements disturbed, damaged or destroyed shall be repaired or replaced to closely match preconstruction conditions.
4. All fill, including places under proposed storm and sanitary sewer lines and paved areas, including trench backfills within and off the road right-of-way, shall be compacted to 90 percent of maximum density as determined by the "Modified AASHTO T-180 Compaction Test (ASTM D 1557)". All tests shall be verified by a Soils Engineer concurrent with grading and backfilling operations. The compacted fill shall be free of rutting and shall be non-yielding and non-pumping during proofrolling and compaction.
5. The contractor shall prevent all storm, surface water, mud and construction debris from entering the existing sanitary sewer system.
6. All sanitary sewer flowlines and tops built without elevations furnished by the engineer will be the responsibility of the sewer contractor.
7. Easements shall be provided for all sanitary sewers, storm sewers and all utilities on the record plat.
8. All construction and materials shall conform to the current construction standards of the Duckett Creek Sanitary District.
9. The Duckett Creek Sanitary District shall be notified at least 48 hours prior to construction for coordination of inspection.
10. All sanitary sewer building connections shall be designed so that the minimum vertical distance from the low point of the basement to the flowline of a sanitary sewer at the corresponding building connection shall not be less than the diameter of the pipe plus the vertical distance of 2 1/2 feet.
11. All sanitary sewer manholes shall be waterproofed on the exterior in accordance with Missouri Department of Natural Resources specification 10 CSR-8.120(7)(E).
12. All PVC sanitary sewer pipe shall conform to the requirements of ASTM D-3034 Standard Specification for PSM Polyvinyl Chloride Sewer Pipe, SDR-35 or equal, with "clean" 1/2 inch to 1 inch granular stone bedding uniformly graded. this bedding shall extend 4 inches below the pipe to springline of pipe. Immediate backfill over pipe shall consist of same size "clean" or "minus" stone from springline of pipe to 6 inches above the top of pipe.
13. All sanitary and storm sewer trench backfills shall be water jetted. Granular backfill will be used under pavement areas.
14. All pipes shall have positive drainage through manholes. No flat invert structures are allowed.
15. All creek crossings shall be lined with rip-rap as directed by District Inspectors.
16. Brick shall not be used on sanitary sewer manholes.
17. Existing sanitary sewer service shall not be interrupted.
18. Maintain access to existing residential driveways and streets.
19. Pre-manufactured adapters shall be used at all PVC to DIP connections. Rubber boot/Mission-type couplings will not be allowed.
20. Any permits, licenses, easements or approvals to work on public or private properties or roadways are the responsibility of the developer.
21. All sanitary sewer laterals shall be a minimum of 4" in diameter.

DEVELOPMENT NOTES

1. Area of Tract: 19.27 Ac.
2. Existing Zoning: R-1 (City of O'Fallon)
3. Proposed Use: Single Family Homes
4. Number of Lots Proposed: 30 Lots
5. The proposed height and lot setbacks are as follows:
   Minimum Front Yard: 25 feet
   Minimum Side Yard: 6 feet
   Minimum Rear Yard: 25 feet
   Minimum Lot Area: 10,000 square feet
   Maximum Height of Building: 2 1/2 stories or 35 feet
6. Current Owner of Property: First Land Co.of St. Charles Cnty
   P.O. Box 176
   St. Peters, MO 63376
   (636) 928-4988
7. Site is served by: Duckett Creek Sanitary District
   Cuivre River Electric Cooperative
   Laclede Gas Company
   St. Charles County Public Water District No. 2
   Century Tel Inc.
   Wentzville School District
   New Melle Fire Protection District
8. No floodplain exists on this tract per F.I.R.M. #29183 C 0405E, dated Aug. 2, 1996.
9. Topographic information is per aerial topography by Sanborn Associates on U.S.G.S. Datum.
11. Boundary information is per survey as compiled by Bax Engineering during August, 2004.
12. One tree shall be planted for every lot. Two trees for corner lots.
13. All homes shall have a minimum of 2 off-street parking places with 2-car garages.
14. The developer realizes that they will comply with current Tree Preservation Ordinance Number 1689 and provide landscaping as set forth in Article 23 of the City of O'Fallon Zoning Ordinances.
15. Additional lighting may be required by the City of O'Fallon.
16. The following lots are susceptible to street movement: 75-81, 83, 84, 86, 87, 90-94,
17. Calculations in accordance to the Tree Preservation Ordinance:
    Existing trees 57.74 acres %
    x 20% 11.55 acres
    Trees to Remain 18.46 acres
    Trees removed 39.28 acres
    Meets the City of O'fallon requirements for tree preservation
18. Detention for this development to be provided by the basins within the common ground areas.
19. All existing creeks and streams will have an associated storm water easement that will be provided on the Record Plat.
20. All the requirements of the Planning and Zoning Commission are addressed.
21. The estimated sanitary flow for this development is 11,100 G.P.D.
22. Required shoulder improvements and walking trail along Diehr Road shall be approved by the City and installed by the Developer in a separate set of construction plans. These improvements will need to be installed at the time that 75 percent of the home permits are issued on the development.
23. The homebuilder shall be required to provide, at the City of O'fallon's discression, engineering studies on all lots with extreme changes in topography showing that homes can be built without significant danger to health, life or property per section 405.140 of The City of O'fallon's Subdivision Code. These studies must be supplied and approved prior to the issuance of Building Permits.

SITE BENCHMARK

CALL BEFORE YOU DIG!
1-800-DIG-RITE
MoDOT - (314) 340-4100

SEWER MEASUREMENTS

THE EXISTING SEWER LENGTHS, SIZES, FLOWLINES, DEPTHS OF STRUCTURES AND SEWERS AND LOCATIONS WITH RESPECT TO EXISTING OR PROPOSED EASEMENTS HAVE BEEN MEASURED. THE RESULTS OF THOSE MEASUREMENTS ARE SHOWN ON THIS SET OF FINAL MEASUREMENT PLANS.

ALL PUBLIC SEWERS ARE LOCATED WITHIN DESIGNATED EXISTING OR PROPOSED EASEMENTS EXCEPT AS FOLLOWS:

SIGNED:
P.E./L.S.

DATE

City of O'Fallon Planning and Engineering File No. 5104.01 was approved Feb. 3, 2006

PREPARED FOR:
FIRST LAND CO. OF ST. CHARLES CNTY, INC.
P.O. BOX 176
ST. PETERS, MO. 63376
636-928-4988

A SET OF AS-BUILT PLANS FOR *WILLOW WALK ESTATES PHASE 1*

DISCLAIMER OF RESPONSIBILITY:
I hereby specify that the documents intended to be authenticated by my seal are limited to this sheet, and I hereby disclaim any responsibility for all other Drawings, Specifications, Estimates, Reports or other documents or instruments relating to or intended to be used for any part or parts of the architectural or engineering project or survey.

REVISIONS

04/04/08	CITY COMMENTS
05/09/08	CITY COMMENTS

ENGINEERING
PLANNING
SURVEYING
221 Point West Blvd.
St. Charles, MO 63301
636-928-5552
FAX 928-1718

9-28-07 *DATE*
04-12901 *PROJECT NUMBER*
1 OF 4 *SHEET OF*
12901-ASB.DWG *FILE NAME*
SAZ *DRAWN*
MJT DRO *DESIGNED CHECKED*

Willow Walk Estates Phase I ASB 1/4

ASBUILT SITE PLAN FOR

WILLOW WALK ESTATES PHASE 1

04-12901 9-28-07

PROPERTY N/F
DOLORES J. CARR AND
PAUL L. JAMES
1141/1789
ZONED A:
ST. CHARLES
COUNTY

PROPERTY N/F
CHRIS WILKES AND
ANGELA WILKES
2361/705
ZONED A:
ST. CHARLES
COUNTY

FALLING LEAF FARMS
PLAT TWO
P.B.22 PG.92
ZONED A:
ST. CHARLES
COUNTY

N88°40'15"E 1401.14'

N01°57'08"W 268.24'

S08°48'48"E 1338.88'

S01°19'42"E 888.60'

N49°56'57"W 6.47'

N71°57'57"W 1110.69'

DIEHR ROAD (40' WIDE)

DESERT WILLOW (50' WIDE) COURT

WILLOW PARK DRIVE (50' WIDE)

WILLOW PARK (50' WIDE) COURT

HYDRANT AND WATER VALVE

TOP OF BERM 636.00

RIP RAP BLANKET 15' X 30'

100-YEAR HIGHWATER EASEMENT

2-YEAR H.W. ELEV=627.37
15-YEAR H.W. ELEV=628.34
25-YEAR H.W. ELEV=628.85
100-YEAR LOW FLOW ELEV. ELEV=631.02

COMMON GROUND "B"
AND DETENTION AREA & UTILITY EASEMENT

WATER LINE AS LOCATED DURING AUGUST, 2004

5' CONCRETE SWALE

4' CONCRETE SWALE

HOUSE ON LOT 100 SET 3.5' ABOVE TOP OF CURB AND 8' BASEMENT POUR

FIRE HYDRANT (TO BE RELOCATED)

RIP RAP BLANKET 10' X 20'

CLEAN-OUT

CONNECTION TO OFFSITE SANTARY

8"PVC

EX. MH 192-B
TOP = 607.76
FL = 593.42

WYNDGATE VILLAGE E
SUMMIT POINTE, L.C.
ZONED: R-1 P.U.D.
CITY OF O'FALLON

FRACTIONAL SECTION

N57°16'31"W 77.78'

L=202.76' R=251.00'
Δ=46°17'00"

N10°59'31"W 157.20'

DETAIL

COMMON GROUND "A"
AND UTILITY EASEMENT

L=142.27' R=211.00'
Δ=38°38'00"

N49°37'31"W 93.40'

L=65.20'
R=211.00'

GRAPHIC SCALE
50 0 25 50
(IN FEET)
1 inch = 50 ft.

SEE DETAIL TO LEFT

COMMON GROUND "A"
AND UTILITY EASEMENT

GRAPHIC SCALE
50 0 25 50
(IN FEET)
1 inch = 50 ft.

BAX PROJECT NAME :WILLOW WALK ESTATES PH. 1 (AS BUILTS)
BAX PROJECT NO. :04-12901
DESIGN DATE :4-4-08
DESIGNED BY :JEL FILENAME: 12901ASB

UPP STR	LOW STR	L	DIA	UPPER FL LN	LOWER FL LN	PS	UPPER ST EL	DEPTH HY GR	UPPER HY EL	LOWER HY EL	HYDR GRADE	FR HEAD	VEL	VEL HEAD	JUNC LOSS	TURN LOSS	CURVE LOSS	STR GRADE	INL CAP	DR AREA	FI	Q	TQ	PIPE CAP	REMARKS	
DCI23	MH22	85	12	623.60	617.80	6.82	631.41	7.37	624.04*	618.80	.01090	0.95	4.74	0.35	0.35	0.00	0.00	LOW	6.80	1.41	2.64	3.72	3.72	9.31	1	
MH22	MH21	57	12	609.08	600.46	15.12	625.03	15.59	609.44*	601.46	.01090	0.62	4.74	0.35	0.01	0.01	0.00		0.00	0.00	0.00	0.00	3.72	13.86	3	
MH21	FE20	28	12	598.68	597.22	5.21	605.91	6.76	599.15*	598.22	.01090	0.31	4.74	0.35	0.14	0.14	0.00		0.00	0.00	0.00	0.00	3.72	8.14	3	TP=598.22
AI25	MH24A	47	12	622.10	616.09	12.82	630.73	8.41	622.32*	617.09	.00140	0.07	1.72	0.05	0.05	0.00	0.00	3-S	7.01	0.51	2.64	1.35	1.35	12.76	4	
MH24A	FE24	79	12	615.89	612.54	4.24	629.90	13.73	616.17*	613.54	.00140	0.11	1.72	0.05	0.00	0.00	0.00	N/A	0.00	0.00	2.64	0.00	1.35	7.34	5	TP=613.54
C119	C118	36	15	642.17	641.81	1.00	647.69	4.63	643.06	643.06	.00010	0.00	0.41	0.00	0.00	0.00	0.00	2%	1.64	0.19	2.64	0.50	0.50	6.46	6	
C118	MH17	53	15	641.61	641.07	1.02	647.70	5.36	642.34	642.32	.00020	0.01	0.77	0.01	0.01	0.00	0.00	2%	1.64	0.17	2.64	0.45	0.95	6.52	7	
MH17	AI16	121	15	640.87	636.51	3.59	649.68	8.59	641.10*	637.76	.00020	0.03	0.77	0.01	0.01	0.00	0.00		0.00	0.00	0.00	0.00	0.95	12.24	8	
AI16	FE15	65	15	636.31	635.33	1.51	640.89	4.18	636.71*	636.58	.00070	0.04	1.38	0.03	0.03	0.01	0.00	3S	7.01	0.28	2.64	0.74	1.69	7.93	9	TP=636.58
C114	C113	36	15	638.08	637.61	1.31	643.38	4.52	638.86	638.86	.00010	0.00	0.50	0.00	0.00	0.00	0.00	2%	1.64	0.23	2.64	0.61	0.61	7.38	10	
C113	AI12	130	15	637.41	636.11	1.00	643.34	5.56	637.79*	637.36	.00040	0.05	0.99	0.02	0.02	0.00	0.00	2%	1.64	0.23	2.64	0.61	1.22	6.45	11	
AI12	FE11	101	18	635.91	634.90	1.00	639.21	2.83	636.38	636.34	.00930	0.23	0.99	0.02	0.01	0.01	0.00	3S	7.01	0.20	2.64	0.53	1.75	10.50	12	HW=636.34
C17	AI5	192	12	637.43	635.59	3.56	643.98	6.14	637.84*	631.59	.00470	0.89	3.09	0.15	0.15	0.00	0.00	LOW	6.80	0.92	2.64	2.43	2.43	6.72	13	
FE6	MH6A	124	18	634.80	633.22	1.27	636.30	0.24	636.06	634.72	.00750	0.93	5.16	0.41	0.41	0.00	0.00	LOW	9.12	3.45	2.64	9.12	9.12	11.86	14	
MH6A	AI5	230	18	633.02	630.59	1.06	638.68	4.92	634.08*	632.09	.00750	1.73	5.16	0.41	0.02	0.02	0.00	N/A	0.00	0.00	2.64	0.00	9.12	10.80	15	
AI5	MH4	206	24	630.39	627.94	1.19	636.95	5.53	631.42*	629.94	.00330	0.68	4.15	0.27	0.00	0.13	0.00	3S	7.01	0.56	2.64	1.48	13.03	24.67	16	
MH4	FE3	46	24	627.74	626.03	3.70	638.40	9.77	628.63	628.34	.00330	0.15	4.15	0.27	0.14	0.14	0.00		0.00	0.00	0.00	0.00	13.03	43.51	17	HW15=628.34
AI10	AI9	136	12	628.79	627.08	1.26	634.01	4.72	629.29*	628.76	.00300	0.41	2.48	0.10	0.10	0.00	0.00	4S	9.35	0.74	2.64	1.95	1.95	4.00	18	
AI9	FE8	92	12	626.88	625.70	1.28	636.46	7.70	628.76	628.34	.00400	0.36	2.82	0.13	0.04	0.02	0.00	3S	7.01	0.11	2.64	0.29	2.24	4.03	19	HW15=628.34
OS2	FE1	94	24	621.79	620.22	1.67	630.13	6.54	623.59	622.22	.00790	0.74	6.39	0.63	0.63	0.00	0.00		0.00	0.00	0.00	0.00	20.07	29.24	20	TP=622.22

* INDICATES CRITICAL DEPTH

ASBUILTS ADDED SEPTEMBER, 2007

O'FALLON FILE NUMBER 5104.01

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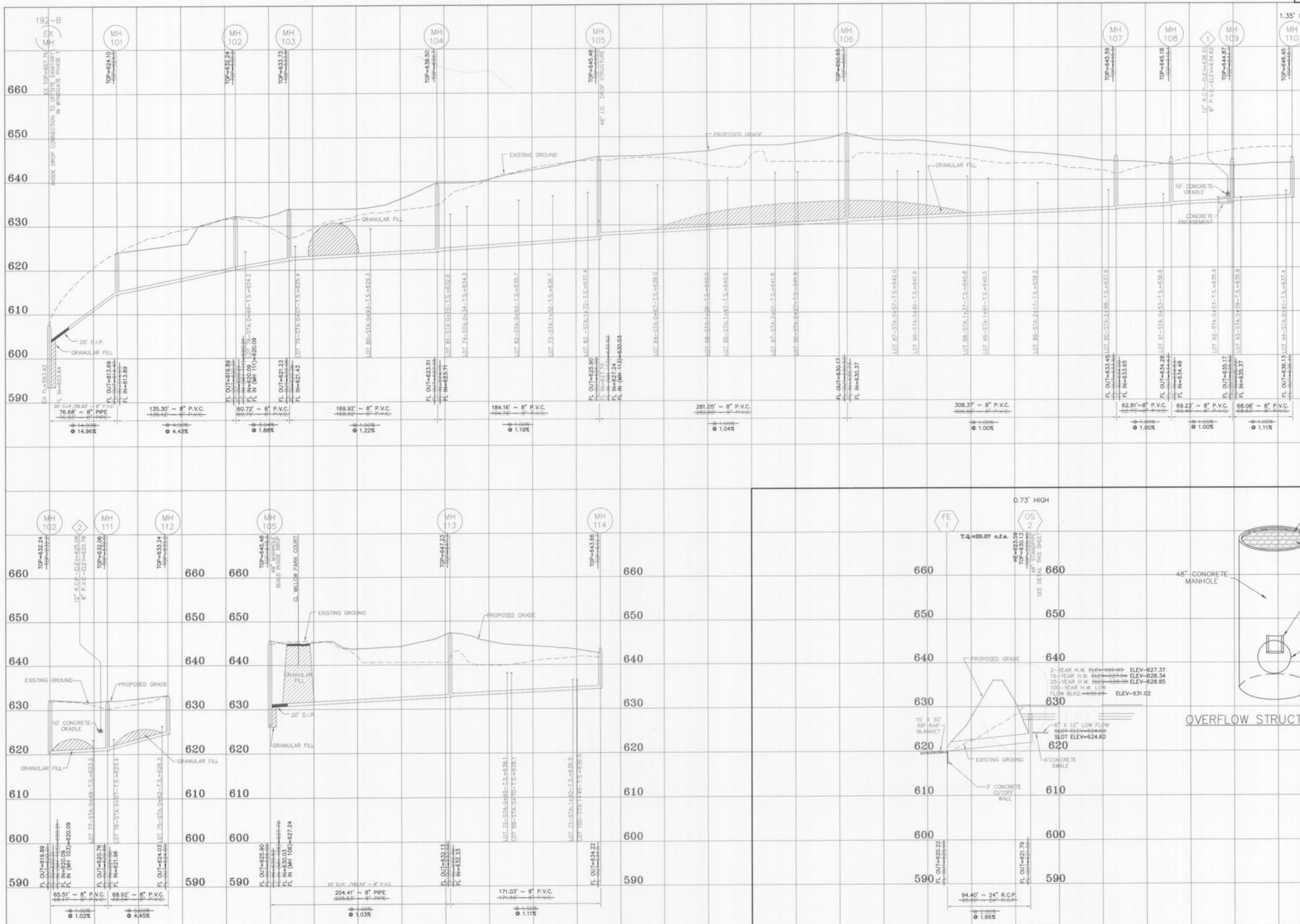
Willow Walk Estates Phase I ASB 2/4

ASBUILT SANITARY & STORM PROFILES FOR

WILLOW WALK ESTATES PHASE 1

04-12901 9-28-07



OVERFLOW STRUCTURE DETAIL

NOT TO SCALE

ASBUILTS ADDED SEPTEMBER, 2007

SCALE: VERTICAL = 10 HORIZONTAL = 50

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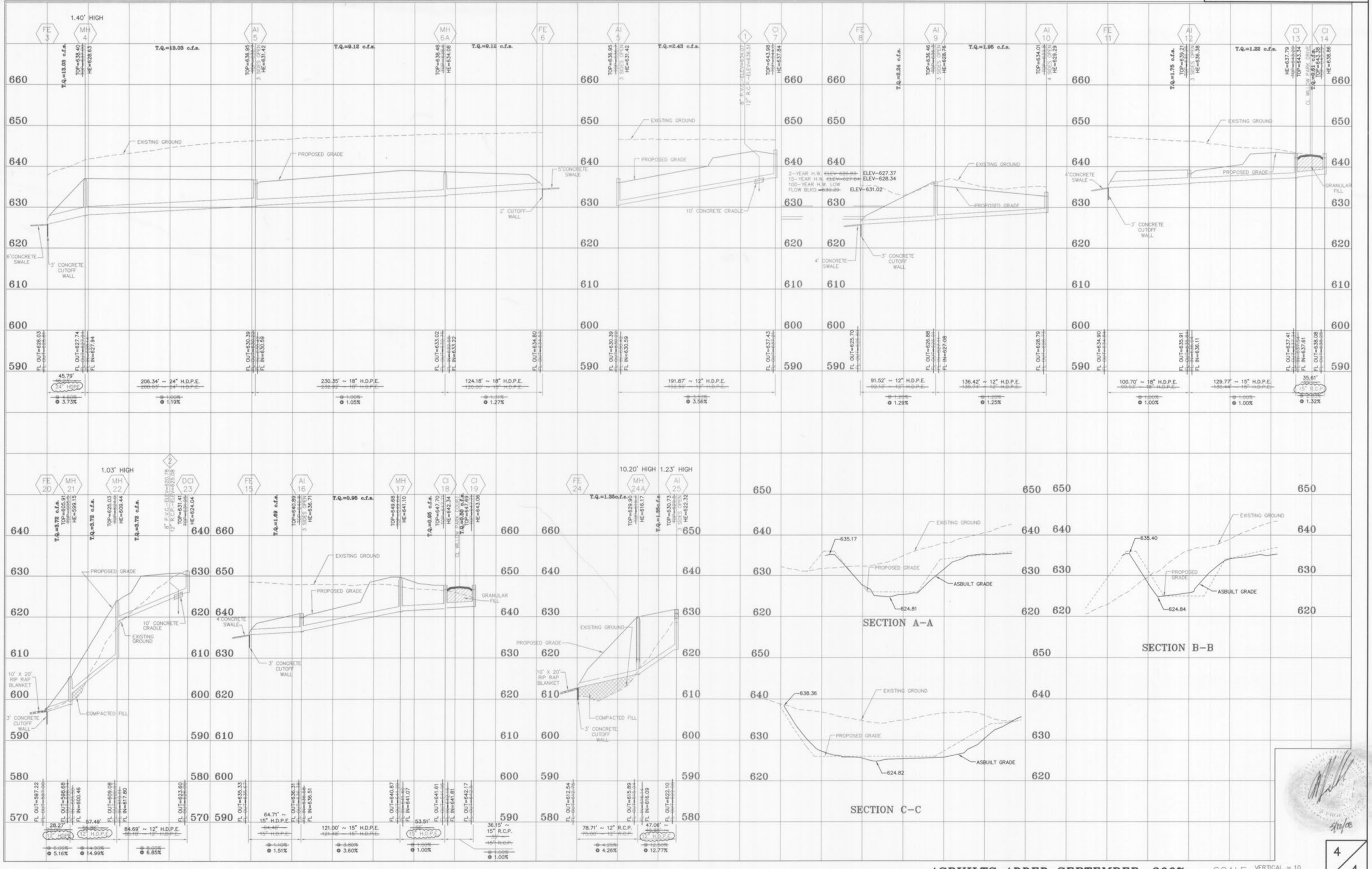
Willow Walk Estates Phase I ASB 3/4

ASBUILT STORM PROFILES FOR

WILLOW WALK ESTATES PHASE 1

04-12901 9-28-07



ASBUILTS ADDED SEPTEMBER, 2007

SCALE: VERTICAL = 10 HORIZONTAL = 50

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Willow Walk Estates Phase I ASB 4/4