A SET OF IMPROVEMENT PLANS FOR

THE CROSSINGS VILLAGE C

A TRACT OF LAND BEING PART OF SECTION 19 AND SECTION 30, TOWNSHIP 47 NORTH, RANGE 3 EAST, OF THE FIFTH PRINCIPAL MERIDIAN ST. CHARLES COUNTY, MISSOURI

GENERAL NOTES

GRADING NOTES

1. A Geotechnical Engineer shall be employed by the owner and be

on site during grading operations. All soils tests shall be verified by the Geotechnical Engineer concurrent with the

2. The grading contractor shall perform a complete grading and

3. The Contractor shall notify the Soils Engineer at least two

days in advance of the start of the grading operation.

4. All areas shall be allowed to drain. All low points shall be

sediment control basins and/or straw bales should be

5. A sediment control plan that includes monitored and maintained

implemented as soon as possible. No graded area is to be

building or structure which is scheduled to be razed for this

7. All trash and debris on site, either existing or from construction,

8. Soft soil in the bottom and banks of any existing or former pand

right-of-way locations or on any storm sewer locations.

9. Site preparation includes the clearance of all stumps, trees,

and other surface obstructions from the site; and the

10. Compaction equipment shall consist of tamping rollers,

pneumatic-tired rollers, vibratory roller, or high speed

layered fill without proper blending of successive fill

demolition and removal of any man-made structures. The

sites or tributaries or an any sediment basins or traps should be

removed, spread out and permitted to dry sufficiently to be used

as fill. None of this material should be placed in proposed public

bushes, shrubs, and weeds; the grubbing and removal of roots

material shall be properly disposed of off-site. Topsoil and grass

in the fill areas shall be thoroughly disced prior to the placement

of any fill. The Soils Engineer shall approve the discing operation.

impact type drum rollers acceptable to the Soils Engineer. The

roller shall be designed so as to avoid the creation of a

11. The Soils Engineer shall observe and test the placement of the

fill to verify that specifications are met. A series of fill

reports showing fill quality will be made to the Owner at

12. The Soils Engineer shall notify the Contractor of rejection of

from the Soils Engineer of its acceptance prior to the

13. All areas to receive fill shall be scarified to a depth of not

specifications given below. Natural slopes steeper than 1

The fill shall be loosely placed in horizontal layers not

be responsible for determining the acceptability of soils

14. The sequence of operation in the fill areas will be fill,

15. The surface of the fill shall be finished so that it will not

16. Fill and backfill should be compacted to the criteria

vertical to 5 horizontal to receive fill shall have horizontal

benches, cut into the slopes before the placement of any fill.

The width and height to be determined by the Soils Engineer.

exceeding 8 inches in thickness and compacted in accordance

placed. Any unacceptable soils placed shall be removed at the

compact, verify acceptable soil density, and repetition of the

sequence. The acceptable moisture contents during the filling

operation are those at which satisfactory dry densities can be

obtained. The acceptable moisture contents during the filling

operation in the remaining areas are from 2 to 8 percent above

impound water. If at the end of a days work it would appear

surface shall be finished smooth. If the surface has been

finished smooth for any reason, it shall be scarified before

proceeding with the placement of succeeding lifts. Fill shall

not be placed on frozen ground, nor shall filling operations

continue when the temperature is such as to permit the layer

Measured as a percent of the maximum dry density as determined

Moisture content must be within 2 percent below or 4 percent

above optimum moisture content if fill is deeper than 10 feet,

MINIMUM

90%

90%

88%

88%

90%

90%

PERCENT COMPACTION

that there may be rain prior to the next working day, the

with the specifications given below. The Soils Engineer shall

less than 6 inches and then compacted in accordance with the

a lift of fill or portion thereof. The Contractor shall rework the rejected portion of fill and obtain notification

density tests will be determined on each lift of fill. Interim

allowed to remain bare without being seeded and mulched.

Care should be exercised to prevent soil from damaging adjacent property and silting up existing downstream

6. Debris and foundation material from any existing on-site

must be removed and properly disposed of off-site.

development must be disposed of off-site.

notes, or reasonably implied there from, all in accordance

with the plans and notes as interpreted by the Geotechnical

compaction operation as shown on the plans, stated in these

grading and backfilling operations.

provided with temporary ditches.

storm drainage system.

regular intervals.

placement of additional fill.

Contractor's expense.

the optimum moisture control.

under placement to freeze.

specified in the following table:

Fill other than building areas

Natural subgrade

Pavement subgrade

Pavement base course

CATEGORY

Fill in building areas below footings

Fill under slabs, walks, and pavement

by modified Proctor Test (ASTM-D-1557).

- 1. Underground utilities have been plotted from available information and therefore their locations shall be considered approximate only. The verification of the location of all underground utilities, either shown or not shown on these plans shall be the responsibility of the contractor, and shall be located prior to any grading or construction of the improvements.
- 2. All manhole tops built without elevations furnished by the Engineer will be the responsibility of the sewer contractor.
- 3. 8" P.V.C. sanitary sewer pipe shall meet the following standards. A.S.T.M.-D-3034 SDR-35, with wall thickness compression joint A.S.T.M.-D-3212. An appropriate rubber seal waterstop as approved by the sewer district shall be installed between P.V.C. pipe and masonry structures.
- 4. All filled places, including trench backfills, under buildings, proposed storm and sanitary sewer lines and/or paved, areas, shall be compacted to 90% maximum density as determined by the "Modified AASHTO T-180 Compaction Test," (A.S.T.M.-D-1557). All filled places within public roadways shall be compacted to 95% of maximum density as determined by the "Standard Proctor Test AASHTO T-99, Method C" (A.S.T.M.D.-698).
- 5. All trench backfills under paved areas shall be granular backfill, and shall be compacted to 90% of the maximum density as determined by the "Modified AASHTO T-180 Compaction Test," (A.S.T.M.-D.-1557). All other trench backfills may be earth material (free of large clads or stones). All trench backfills shall be water jetted.
- 6. All sanitary house connections have been designed so that the minimum vertical distance from the low point of the basement to the flow line of a sanitary sewer at the corresponding house connection is not less than the diameter of the pipe plus the vertical distance of 2 1/2 feet.
- 7. No area shall be cleared without the permission of the Project
- 8. All P.V.C. sanitary sewer is to be SDR-35 or equal with clean 1/2" to 1" granular stone bedding uniformly graded. This bedding shall extend from 4" below the pipe to the springline of the pipe. Immediate backfill over pipe shall consist of same size "clean" or minus stone from springline of pipe to 12" above the top of pipe.
- 9. All soils test shall be verified by a Soils Engineer concurrent with the grading and backfilling operations.
- 10. Easements shall be provided for sanitary sewers, and all utilities on the Record Plat. See Record Plat for location and size of easements.
- 11. Maintenance and upkeep of the common ground area shall be the responsibility of the developer and/or successors.
- 12. A 25' building line shall be established along all Public Right-Of-Way.
- 13. All water lines shall be laid at least 10 feet horizontally, from any sanitary sewer, storm sewer, or manhole, 18" vertical clearance from outside of pipe to outside of pipe shall be maintained wherever water lines must cross sanitary sewers, laterals, or storm drains the water line shall be laid at such an elevation that the bottom of the water line is above the top of the drain or sewer. A full length of water pipe shall be centered over the sewer line to be crossed so that the joints will be equally distant from the sewer and as remote therefrom as possible. This vertical separation shall be maintained for that portion of the water line located within 10 feet horizontally, of any sewer or drain it crosses.
- 18. All PVC water pipe shall conform to ASTM D2241, SDR 21 Standard Specification for P.V.C. Pressure Pipe, 200 P.S.I. working pressure for water, with approved joint.
- 19. Water lines, valves, sleeves, meters, and fittings shall meet all specifications and installation requirements of City of O'fallon.
- 20. All water hydrants and valves shall be ductile iron and installed in accordance with plans and details. All ductile iron pipe for water mains shall conform to A.W.W.A. Specifications C-106 and/or C-108. The ductile iron fittings shall conform to A.W.W.A. Specification CC-110. All rubber gasket joints for water ductile iron pressure pipe and fittings shall conform to A.W.W.A. Specification C-111.
- 21. All sanitary manholes shall be waterproofed on the exterior in accordance with Missouri Department of Natural Resources specifications 10 CSR-8.120 (7)E.
- 22. Brick will not be used in the construction of sanitary sewer manholes.
- 23. All pipes shall have positive drainage through manholes. No flat base structures are allowed.
- 24. The City of O'Fallon and shall be notified 48 hours prior to construction for coordination and inspection.
- 25. Gas, water and other underground utilities shall not conflict with the depth or horizontal location of existing or proposed sanitary or storm sewers, including

- 26. All existing site improvements disturbed, damaged or destroyed shall be repaired or replaced to closely match preconstruction conditions.
- 27. The contractor shall prevent all storm, surface water, mud and construction debris from entering the existing sanitary sewer system.
- 28. All construction and materials shall conform to the current construction standards of the City of O'Fallon.
- 29. All sanitary and storm sewer trench backfills shall be water jetted. Granular backfill will be used under pavement areas.
- 30. All existing areas disturbed during construction of the offsite sanitary sewer line shall be seeded and mulched to prevent erosion.
- 31. All sanitary sewer laterals shall be a minimum of 4" in diameter per
- 32. No flushing hydrants or water meters shall be located in driveways and or walkways.
- 33. Concrete pipe for storm sewers shall be Class III, A.S.T.M. C-76 with a minimum diameter of 12" except in the R.O.W. it shall be 15".
- 34. The ADS N-12 pipe shall have a smooth interior wall.
- 35. Concrete pipe joints shall be MSD type "A" approved compression-type joints and shall comform to the requirements of the specifications for joints for circular concrete sewer and culvert pipe, using flexible, watertight, rubber-type gaskets ASTM C443. Band-type gaskets depending entirely on cement for adhesion and resistance to displacement during jointing shall not be used.
- 36. When HDPE pipe is used, City of O'Fallon specifications or manufacturers specifications, which ever are more stringent, shall be followed.
- 37. The use of High Density Polyethylene Corrugated pipe, ADS N-12 or equal will be permitted as an acceptable alternative to rein-forced concrete pipe. Pipe shall meet A.S.T.M. D-2321 and A.A.S.H.T.O. M-294-291.
- 38. All flared end sections and inlet structures will be concrete.
- 39. All storm sewer pipe installed in the Public Right-of-Way shall be Rein-forced concrete Class III pipe.
- 40. All concrete pipe or ADS N-12 pipe shall be installed with "O-Ring" Rubber type gaskets per M.S.D. standard construction specifications or manufacturer.

KEY MAP

R-1 (P.U.D.)

TOM GINNEVER

WABASH RO.

COOL SPRINGS PEARL ORIVE

NORTH SERVICE ROAD

LOCATION MAP

78.598 Acres

INTERSTATE HWY 70

2. Existing Zoning: Single Family Homes 3. Proposed Use: 170 Lots 4. Number of Lots Proposed:

DEVELOPMENT NOTES

7,920 Square Feet 5. Minimum Lot Area Proposed:

6. The proposed height and lot setbacks are as follows: Minimum Front Yard: 25 feet Minimum Side Yard: 6 feet Minimum Rear Yard: 25 feet Maximum Height of Building: 2 1/2 stories or 35 fee

7. Site is served by: City of O'Folion Sanitary Sewer AmerenUE St. Charles Gas Company City of O'Fallon Water

1. Area of Tract:

Verizon Telephone Company Fort Zumwalt School District O'Fallon Fire Protection District

- 8. Part of this tract is located within the 100 year flood plain limits per F.I.R.M. #29183C0235 E. August 2, 1996
- 9. All streets will be constructed to City of O'Fallon standards. Steets will consist of 26 foot wide concrete pavement with integral rolled curb centered in a 50 foot right-of-way. A minimum centerline radius shall be 150 feet.
- 10. Minimum street grades shall be 1%.
- 11. All homes shall have a minimum of 2 off-street parking places with 2-car
- 12. All utilities must be located underground
- 13. A 4' foot wide concrete sidewalk shall be constructed on one side of streets as indicated on plan.
- 14. The developer realizes that they will comply with the current Tree Preservation Ordinance Number 1689 and provide landscaping as set forth in Article 23 of the City of O'Fallon Zoning Ordinance.
- 15. Lots adjacent to the existing creek shall have low sill elevations set high enough to prevent high water from the creek reaching the building structure. A L.O.M.R. shall be applied for these lots.
- 16. All lots shall access interior streets only. No driveways shall access Cool Springs Rd.

BENCHMARK

U.S.G.S.

Chiseled L on top of wingwall in northeast corner of Old Highway 79 bridge over Belleau Creek

Cross at the centerline intersection of Vantage Pass and Kingston Crossing in The Crossings Village B. Elev.= 497.69

LEGEND

- - STORM SEWER

SANITARY SEWER

	CURB INLET	•	STREET LIGHT	TREE PRESERVATION CALCULATIONS:
1	DOUBLE CURB INLET AREA INLET	582-	EXISTING CONTOUR	TOTAL AREA OF EXISTING TREE MASSES: 44.33 AC. 44.33 AC. × 20% = 8.87 AC.
H E	MANHOLE FLARED END SECTION		PROPOSED CONTOUR	TOTAL AREA OF PROPOSED CLEARING: 19.11 AC. TOTAL AREA OF REMAINING TREES: 25.22 AC. 25.22 AC. > 8.87 AC. = (NO ADDITIONAL TREES N
P.	END PIPE CONCRETE PIPE	Sxs	STREET SIGN	
R.	REINFORCED CONCRETE PIPE	100	NO PARKING SIGN	
LP. P.	CORRUGATED METAL PIPE. CAST IRON PIPE	×	WATER VALVE	LANDSCAPE REQUIRMENTS: LENGTH OF CENTERLINE OF STREETS = 8,644 LF
C .	POLY VIVYL CHLORIDE (PLASTIC) CLEAN OUT	8.0.	BLOW OFF ASSEMBLY	8,644 LF, X 2 = 17,288 LF. 17,288 LF / 50 LF. = 346 TPEES
×	FIRE HYDRANT	1-	FLOWLINE ELEVATION OF HOUSE CONNECTION	TOTAL PROPOSED - 346 TREES NOTE: PROPOSED REPLACEMENTS TREES WILL BE HARDWOOD

- FLOWINE ELEVATION OF SEWER MAIN

CONSTRUCTION AND YARD FINISH GRÂDING BY THE HOMEOWNER AS REQUIRED IN THE COVENENTS AND RESTRICTIONS.

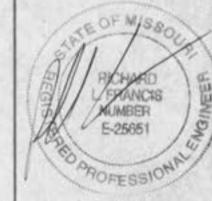
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