. R  . .	ETAINING WALL DESIGN: STRUCTURAL DESIGN HEREIN REPRESENTS A FINISHED STR PROVIDE ALL INTERIM BRACING, SHORING, INTERIM DRAINA PROTECTION REQUIRED UNTIL FINAL CAPPING, PAVING, CUR IS COMPLETE.	UCTURE. THE GENERAL CONTRACTOR/OWNER SHALI AGE PROVISIONS, DRAINAGE DIVERSION AND EROSIC RBING AND COMPLETION OF FINAL STORM DRAIN SY	- 6. DN 6. STEM	BASE LEVELING PAD INSTALLATION: I. LEVELING PAD SHALL BE PLACED AS SH LEAN CONCRETE (2,000 PSI) - 6" N WELL GRADED I" GRAVEL WITH EINE
. .  . .	<ol> <li>IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTORS</li> <li>IS DIRECTED AWAY FROM THE RETAINING WALL SYSTEM</li> <li>IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR</li> <li>FROM ADJACENT CONSTRUCTION AREAS IS NOT ALLOW</li> </ol>	OR/OWNER TO ENSURE THAT THE FINISHED SITE DRA A. OR/OWNER TO ENSURE THAT THE SURFACE WATER F WED TO ENTER THE RETAINING WALL ARFA OF THF	AINAGE 6. RUNOFF	2. SAND OR GRAVEL BASE SHALL BE CON LEVEL BEARING PAD.
	CONSTRUCTION SITE.		6.	3. LEAN CONCRETE SHALL CURE A MINIMU
1.2.	THE DESIGN OF THE SEGMENTAL RETAINING WALLS IS IN A SEGMENTAL RETAINING WALLS 3RD EDITION AND INCLUDES REINFORCED MASS, AND INTERNAL STABILITY; PULLOUT, C GEOGRID AS WELL AS FACIAL STABILITY OF THE FACE UNIT INDIVIDUAL ELEVATIONS.	CCORDANCE WITH NCMA DESIGN MANUAL FOR 5 EXTERNAL STABILITY; SLIDING AND OVERTURNING ONNECTION STRENGTH AND TENSILE STRENGTH OF 'S. THE APPLIED BEARING PRESSURES ARE LISTED (	6. OF THE THE 7. DN THE 7.	<ol> <li>LEVELING PAD SHALL BE CONSTRUCTED UNIT INSTALLATION:</li> <li>THE FIRST COURSE OF SEGMENTAL CO FOR LEVEL, ALIGNMENT, AND FULL CON</li> </ol>
1.3.	THE DESIGN OF THE SEGMENTAL RETAINING WALLS IS BAS	ED ON THE FOLLOWING DOCUMENTS:	7.	2. UNITS SHALL BE PLACED SIDE BY SIDE I
	DRAWINGS CO4 DATED 5/30/2018 PREPARED BY CEDC: C	IVIL ENGINEERING DESIGN CONSULTANTS.	7.	3. PLACE DRAINAGE AGGREGATE A MINIML
1	GEOTECHNICAL REPORT DATED 5/17/2018 PREPARED BY T	FERRACON.	ON THE	TOP OF THE UNIT. PLACE REINFORCED AGGREGATE WITH 2 PASSES OF A VIBR
.4.	ELEVATIONS AS WELL AS THE FOLLOWING CRITERIA:	D ON THE INDIVIDUAL SOLET KOLEKILLS AS LISTED (		COURSE.
	SEISMIC ACCELERATION = N/A	VEL PAD (WHERE H = HEIGHT OF WALL)	7.	4. LAY UP EACH COURSE INSURING POSIT
	HYDROSTATIC LOADING = NONE		8.	I. GEOGRID SHALL BE LAID AT THE PROPE
	SURCHARGE LOADING = SEE WALL ELEVATION(S)		8.	2. THE GEOGRID REINFORCEMENT SHALL E THE BLOCK.
ع . ا .	ETTLEMENT: SEGMENTAL RETAINING WALLS ARE FLEXIBLE MASSES THAT	T CAN TOLERATE MINOR SETTLEMENT. ROSCH	8.	3. PLACE GEOGRID ON CONCRETE WALL U GEOGRID TIGHT PRIOR TO BACKFILLING
	ENGINEERING SHALL BE NOTIFIED OF ANY SETTLEMENT SENSITIVE RIGID MASSES FOUNDED ON OR ABOVE THE SEGMENTAL RETAINING WALL.		8.	4. GEOGRID SHALL BE LAID AT THE PROPE
5. N 3.1.	1ATERIAL PROPERTIES: SEGMENTAL CONCRETE WALL UNITS SHALL BE MANUFACTURED IN ACCORDANCE WITH ASTM C I 372 HAVING A		8. A	5. CORRECT ORIENTATION OF THE GEOGR
	MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 3,000 PSI AND A MAXIMUM MOISTURE ABSORPTION OF 8%. ALL UNITS SHALL BE SOUND AND FREE OF CRACKS OR OTHER DEFECTS THAT WOULD INTERFERE WITH THE PROPER PLACING OF THE UNIT OR SIGNIFICANTLY IMPAIR THE STRENGTH OP PERFORMANCE OF THE CONSTRUCTION		ALL 8. R	G. 3" OF REINFORCED BACKFILL SHALL BE
.2	PLACING OF THE UNIT OR SIGNIFICANTLY IMPAIR THE STREE	NGITION PERFORMANCE OF THE CONSTRUCTION.	9. 9.	DAGNFILL FLACEMENT: 1. REINFORCED BACKFILL MATERIAL SHALL STANDARD PROCTOR DENSITY (ASTM D
·	FOLLOWING GRADATION AS DETERMINED IN ACCORDANCE SIEVE SIZE PERCENT PASSING	WITH ASTM D 422:	9.	2. DRAINAGE ROCK SHALL BE COMPACTED
	I INCH         I 00 <sup>3</sup> / <sub>4</sub> INCH         75-100           NO         4		q	DENSITY TESTING WILL NOT BE REQUIRE
	NO. 40         0-60           NO. 200         0-50		9.	UNLESS NOTED OTHERWISE.
.3.	COMPACTED ROCK SHALL BE FREE OF ORGANIC MATERIAL	THE ROCK SHALL BE A WELL GRADED GRAVEL OR	9.	4. REINFORCED BACKFILL SHALL BE PLACE DEVELOPMENT OF WRINKLES AND/OR M
	LIMESTONE WITH A MAXIMUM PARTICLE SIZE OF 2" AND A LIMESTONE SCREENINGS MEETING THESE REQUIREMENTS /	MAXIMUM OF 20% PASSING A NO. 200 SIEVE. ARE ACCEPTABLE.	9.	5. REINFORCED BACKFILL SHALL BE PLACE FMBANKMENT TO INSURE THAT THE GE
.4.	LOW PERMEABLE SOIL SHALL CONSIST OF MATERIAL HAVIN I 0% SHALL BE RETAINED ON A NO. 4 SIEVE AND NO LESS	NG A MINIMUM PLASTICITY INDEX OF 10. NO MORE THAN 35% SHALL PASS A NO. 200 SIEVE. MATERI	THAN AL 9.	6. TRACKED CONSTRUCTION EQUIPMENT
F	WITH A USC DESIGNATION OF ML, CL, OR OL ARE ACCEPTA	ABLE FOR USE AS LOW PERMEABLE SOIL.		THICKNESS OF 6" SHALL BE MAINTAINE TRACKED CONSTRUCTION EQUIPMENT S
.5.	THE GEOGRID SHALL BE A HIGH DENSITY POLYETHYLENE EX SPECIFICALLY FABRICATED FOR USE AS SOIL REINFORCEM AS FOLLOWS:	IENT. ACCEPTABLE GEOGRID TYPES AND MANUFACT	RIAL, TURER 9.	<ol> <li>AT THE END OF EACH DAYS OPERATION</li> </ol>
	TYPE I: SF35 BY SYNTEEN TECHNICAL FABRICS, INC.			INTERIOR (CONCEALED) FACE OF THE W
	STRATAGRID 200 BY STRATA SYSTEMS, INC.		10. 1C	DRAIN PIPE INSTALLATION: D. I . DRAINAGE COLLECTION PIPES SHALL BE REINFORCED SOIL ZONE THE DRAINAG
	SF55 BY SYNTEEN TECHNICAL FABRICS, INC. STRATAGRID 350 BY STRATA SYSTEMS, INC.			DAYLIGHT THROUGH THE FACE OF THE
			. 	CAP INSTALLATION: .I. CAP UNITS SHALL BE ADHERED TO THE
	STRATAGRID 550 BY STRATA SYSTEMS, INC.			THE ADHESIVE AND ALLOW TO CURE.
.6.	GEOTEXTILE FILTER FABRIC SHALL BE A NONWOVEN GEOTE. MINIMUM FLOW RATE OF 140 GPM/FT2 WHEN TESTED ACC	XTILE COMPOSED OF POLYPROPYLENE FIBERS WITH ORDING TO ASTM D 4491.	A 12. 12	FIELD QUALITY CONTROL:
.7.	DRAINAGE PIPE SHALL BE A 4"Ø PERFORATED OR SLOTTED PVC OR CORRUGATED HDPE PIPE.			OF ALL SYSTEM COMPONENTS TO MEE
.8.	DRAINAGE PIPE SHALL BE MANUFACTURED IN ACCORDANC	E WITH ASTM F 405 OR ASTM F 758.	12	2.2. TESTING METHODS, FREQUENCY AND V THE INDEPENDENT THIRD PARTY INSPEC
.9.	CONSTRUCTION ADHESIVE SHALL BE EXTERIOR GRADE ADE CONCRETE WALL UNIT MANUFACTURER.	HESIVE AS RECOMMENDED BY THE SEGMENTAL	13. TF	ABBREVIATIONS: TOP OF FOOTING FLEVATION
E F. F.	CAVATION: THE CONTRACTOR SHALL EXCAVATE TO THE LINES AND GR	ADES SHOWN ON THE PLANS. THE CONTRACTOR S	TW SHALL STA	TOP OF WALL ELEVATION STATION
2	TAKE PRECAUTIONS TO MINIMIZE OVER-EXCAVATION.			
·.∠.	IS THE RESPONSIBILITY OF THE CONTRACTOR.	EXCAVATION AND ITS INFLUENCE ON ADJACENT FRO	Л LNIT	RC BA
F .I.	DUNDATION SOIL PREPARATION: FOLLOWING EXCAVATION FOR THE LEVELING PAD AND THE	REINFORCED SOIL ZONE, FOUNDATION SOIL SHALL	BE	
	EXAMINED BY THE OWNER'S GEOTECHNICAL ENGINEER TO A OR EXCEEDS THE ASSUMED DESIGN BEARING STRENGTH. REMOVED AND REPLACED WITH SOIL MEETING THE DESIGN	SOIL NOT MEETING THE REQUIRED STRENGTH SOIL NOT MEETING THE REQUIRED STRENGTH SHAL CRITERIA. AS DIRECTED BY THE OWNER'S GEOTECT	MEETS L BE INICAL	AN VA
	ENGINEER.			
5.2.	FOUNDATION SOIL IS DEFINED AS THE SOIL UNDER THE SE TOE OF THE LEVELING PAD TO THE BACK OF THE REINFORC	EGMENTAL RETAINING WALL VOLUME, EXTENDING FRO CED MASS.	OM THE	
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		の ー ・ · · · · · · · · · · · · · · · · · ·	-	GEOGRID LENGTH
	REQ'D BEARING CAPACITY	I ,000 PSF REINFORCED BACKFILL IS		REQ'D BEARING CAPACITY
		DRAINAGE ROCK - SEE DETAIL 2/3		
	6 WALL ELEVATION		-	
	$ \begin{array}{c} 1 \\ \hline \end{array} \begin{array}{c} \text{SCALE: } 1 = 20^{\circ} - 0^{\circ} \\ \hline \end{array} \begin{array}{c} \text{HOKIZONIAL} \\ \hline \\ \text{SCALE: } 1 = 5^{\circ} - 0^{\circ} \\ \hline \end{array} \begin{array}{c} \text{VERTICAL} \\ \hline \end{array} $			

GENERAL NOTES:

S SHOWN ON THE DRAWINGS AND CONSIST OF EITHER: 6" MINIMUM THICK FINES - 6" MINIMUM THICK

COMPACTED WITH 3 PASSES OF A VIBRATORY COMPACTOR TO PROVIDE A FIRM,

INIMUM OF 12 HOURS PRIOR TO UNIT PLACEMENT.

CONCRETE WALL UNITS SHALL BE PLACED ON THE LEVELING PAD AND CHECKED CONTACT WITH BASE.

BIDE FOR FULL LENGTH OF WALL. ALIGNMENT SHALL BE DONE BY MEANS OF A MENT FROM BASE LINE.

INIMUM OF 12" DIRECTLY BEHIND AND BETWEEN THE UNITS AND LEVEL WITH THE CED BACKFILL DIRECTLY AGAINST DRAINAGE FILL. COMPACT DRAINAGE VIBRATORY COMPACTOR. COMPACTION TESTING OF DRAINAGE AGGREGATE IS . SHALL BE REMOVED FROM TOP OF UNITS PRIOR TO INSTALLATION OF NEXT

OSITIVE CONTACT BETWEEN PREVIOUS COURSE IS ACHIEVED.

ROPER ELEVATION AND ORIENTATION AS SHOWN ON THE DRAWINGS.

ROPER ELEVATION AND ORIENTATION AS SHOWN ON THE DRAWINGS.

EOGRID SHALL BE VERIFIED.

L BE PLACED BETWEEN ALL LOCATIONS OF OVERLAPPING GEOGRID.

SHALL BE PLACED IN 8" MAXIMUM LIFTS AND COMPACTED TO A MINIMUM 95% OF STM D 698).

QUIRED FOR DRAINAGE ROCK.

OR MOVEMENT OF THE GEOGRID.

E GEOGRID REMAINS TIGHT.

AINED TO OPERATE TRACKED VEHICLES OVER THE GEOGRID. TURNING OF IENT SHALL BE KEPT TO A MINIMUM TO PREVENT TRACKS FROM DISPLACING THE

ATION, SLOPE THE LAST LEVEL OF COMPACTED BACKFILL AWAY FROM THE THE WALL TO DIRECT SURFACE WATER RUNOFF FROM THE WALL FACE.

LL BE INSTALLED TO MAINTAIN GRAVITY FLOW OF WATER OUTSIDE OF THE NAGE COLLECTION PIPE SHOULD CONNECT INTO A STORM SEWER MANHOLE OR THE WALL.

THE TOP UNITS USING MANUFACTURER SUPPLIED ADHESIVE BY PLACING TWO 1/4" IT ALONG THE ENTIRE LENGTH OF THE WALL. PRESS THE CAP UNITS FIRMLY INTO

ENTATIVE IS RESPONSIBLE FOR ENGAGING THE SERVICES OF AN INDEPENDENT ERVE AND VERIFY ALL SOIL PROPERTIES AS WELL AS VERIFY CORRECT INSTALLATION ) MEET THE REQUIREMENTS OF THESE GENERAL NOTES AND DRAWINGS.

ND VERIFICATION OF MATERIAL SPECIFICATIONS SHALL BE THE RESPONSIBILITY OF ISPECTOR.











BASED ON THE DESIGN CRITERIA, ASSUMED SOIL PARAMETERS, AND KNOWN LOADING CONDITIONS AS LISTED IN THESE DRAWINGS. THE OWNERS REPRESENTATIVE, INDEPENDENT THIRD PARTY SPECIAL INSPECTOR AND INSTALLER SHALL NOTIFY ROSCH ENGINEERING OF ANY CHANGES OR DIFFERENCES IN ACTUAL SITE CONDITIONS WHICH VARY FROM THOSE LISTED, PRIOR TO CONSTRUCTING THE WALL.