Project ID:

Re	tain	ing Wall			F	ile = N:\2	017\0	ONETIM~1\ZZMR-0~1\STRUCT ENERCALC, INC. 1983-201		
4										Case Engineering
aining	y Wall - I	5'-8" Tall								
				Soil Data				Calculations per ACI 31	8-08.	ACI 530-08, IBC 2009
	=	5.67 ft		Allow Soil Bearing =	2,000.0 p	sf			,	CBC 2010, ASCE 7-0
il	=	4.00 ft		Equivalent Fluid Pressure Method		51				
	=	0.00 : 1		Heel Active Pressure =	35.0 ps	sf/ft				
be	=	20.00 in		Toe Active Pressure =	30.0 ps					
el	=	0.0 ft		Passive Pressure =	250.0 ps					
f acti		0.0 11		Soil Density, Heel =	•					
options:				Soil Density, Toe =	110.00 pcf 110.00 pcf					
Soil Pressure.				Friction Coeff btwn Ftg & Soil =	0.400	,				
	0	stance.		Soil height to ignore						
Jvert	urning	Resistance.		for passive pressure =	12.00 in					
				Lateral Load Applied to Stem	1		A	djacent Footing Load		
4	=	100.0 psf		Lateral Load =	0.0 plf]		Adjacent Footing Load	=	0.0 lbs
ding a		turning		Height to Top =	0.00 ft			Footing Width	=	0.00 ft
	=	0.0 psf		Height to Bottom =	0.00 ft			Eccentricity	=	0.00 in
	urning			-				Nall to Ftg CL Dist	=	0.00 ft
to S	Stem							Footing Type		Line Load
	=	0.0 lbs					t	Base Above/Below Soil at Back of Wall	=	0.0 ft
	=	0.0 lbs		Wind on Exposed Stem =	5.0 psf		F	Poisson's Ratio	=	0.300
ity	=	0.0 in								0.000
				Stem Construction		op Sten	n	2nd Stem OK		
				Design Height Above Ftg	ft =	5.	.67	0.00		
	=	2.63 0		Wall Material Above "Ht"	=	Fen	ice	Concrete		
Slidin	=	2.12 0	<	Thickness	in =		.00	8.00		
muni	5	2,981 lbs		Rebar Size Rebar Spacing	= in =	# 12.	5	# 5 12.00		
	=	7.54 in		Rebar Placed at	= 111		lge	Edge		
			0.1	Design Data	_	Lu	90	-		
I	=	1,538 psf		fb/FB + fa/Fa	=			0.328		
I	=	15 psf 2,000 psf		Total Force @ Section	lbs =		0.0	1,153.8		
Less	Than A	Allowable		MomentActual	ft-l =		0.0	2,726.8		
	=	1,846 psf		MomentAllowable	ft-l =		0.0	8,312.6		
	=	18 psf		ShearActual ShearAllowable	psi = psi =		0.0 0.0	15.5 94.9		
<u>ب</u>	=	2.3 psi		Wall Weight	psi = psf =		0.0 0.0	100.0		
el	=	29.4 psi		Rebar Depth 'd'	in =		.00	6.19		
= 94.9 psi				Lap splice if above	in =		.00	18.50		
Resists All Sliding ! = 873.0 lbs				Lap splice if below	in =	0.	.00	3.60		
orce	= = -	656.3 lbs		Hook embed into footing	in =	0.	.00	3.60		
rce	= -	1,19 0.0 lbs		Concrete Data ———		1.00		1 000 0		
	=	0.0 lbs	OK	f'c Fv	psi =	4,000		4,000.0		
/	=	0.0 lbs		Fy	psi =	20,000	J.U	20,000.0		

Cantilevered F

Lic. # : KW-06010584 Description : Retain

Criteria

CITIEITA				
Retained Height	=	5.67 ft		
Wall height above soil	=	4.00 ft		
Slope Behind Wall	=	0.00 :	1	
Height of Soil over Toe	=	20.00 in	l I	
Water height over heel	=	0.0 ft		
Vertical component of act				
Lateral soil pressure optio				
NOT USED for Soil F NOT USED for Slidin				
NOT USED for Overt	urnina l	Resistanc	e.	
	j			
Surcharge Loads				
Surcharge Over Heel		100.0 p	sf	
Used To Resist Sliding Surcharge Over Toe	a Overi	urning 0.0 p	sf	
Used for Sliding & Over	turning	0.0 P	51	
Axial Load Applied to S	Stem			
Axial Dead Load	=	0.0 lb	S	
Axial Live Load	=	0.0 lb		
Axial Load Eccentricity	=	0.0 in		
Design Summary				
Wall Stability Ratios				
Overturning	=	2.63	0k 2 Ok	Ķ
Sliding Slab Resists All Slidin	= a I	Z.12	Ur	`
Total Bearing Load	g. =	2,981	lbs	
resultant ecc.	=	7.54	in	
Soil Pressure @ Toe	_	1,538	ncf	
Soil Pressure @ Heel	=		psf	
Allowable	=	2,000		
Soil Pressure Less	s Than A	llowable	•	
ACI Factored @ Toe ACI Factored @ Heel	=	1,846		
	=		psf	
Footing Shear @ Toe Footing Shear @ Heel	=	2.3 29.4	psi	
Allowable	=	94.9		Ur
Sliding Calcs Slab Resis	sts All S		P0.	
Lateral Sliding Force	=	873.0	lbs	
less 100% Passive Force	= -	656.3		
less 100% Friction Force	= -	1,190.0	lbs	
Added Force Req'd	=		lbs	
for 1.5 : 1 Stability	=	0.0	lbs	0
Load Factors				
Dead Load		1.20		
Live Load		1.60		
Earth, H		1.60		
Wind, W Seismic, E		1.60 1.00		
		1.00	.0	