GENERAL NOTES:

- 1. IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR CONDITIONS OF THE JOB SITE. INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS. ANY CONSTRUCTION OBSERVATION BY THE ENGINEER OF THE CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES, IN, ON OR NEAR THE CONSTRUCTION SITE.
- 2. THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT ALL NECESSARY PERMITS HAVE BEEN OBTAINED FROM THE GOVERNING AGENCIES AND COORDINATING ALL GOVERNING AGENCY INSPECTIONS REQUIRED THROUGHOUT THE DURATION OF THE PROJECT.
- 3. CONTRACTOR SHALL BE RESPONSIBLE FOR RAZING AND REMOVAL OF THE EXISTING STRUCTURES, RELATED UTILITIES, PAVING, AND ANY OTHER EXISTING IMPROVEMENTS AS NOTED REFERENCE SITE WORK SPECIFICATIONS
- 4. CONTRACTOR IS TO REMOVE AND DISPOSE OF ALL DEBRIS, RUBBISH AND OTHER MATERIALS RESULTING FROM PREVIOUS AND CURRENT DEMOLITION OPERATIONS. DISPOSAL WILL BE IN ACCORDANCE WITH ALL LOCAL, STATE AND/OR FEDERAL REGULATIONS GOVERNING SUCH
- 5. THE CONTRACTOR WILL BE HELD SOLELY RESPONSIBLE FOR DAMAGE TO ADJACENT PROPERTIES AND NEW CONSTRUCTION IN PLACE DURING THE CONSTRUCTION PHASES OF THIS PROJECT. ANY DISTURBED IMPROVEMENTS SHALL BE REPLACED IN KIND AT THE CONTRACTORS EXPENSE.
- 6. ANY QUANTITIES PROVIDED ON THESE PLANS ARE FOR GENERAL REFERENCE PURPOSES ONLY. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE QUANTITIES REQUIRED FOR
- 7. THE EXISTING FEATURES SHOWN ON THESE PLANS ARE THOSE NOTED IN THE FIELD AND THOSE TAKEN FROM RECORD DRAWINGS. THERE IS NO GUARANTEE THAT ALL FEATURES (ABOVE OR BELOW GROUND) ARE SHOWN ON THE PLANS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL EXISTING FEATURES PRIOR TO BIDDING THE PROJECT.
- 8. THE CONTRACTOR SHALL LOCATE ALL UTILITIES PRIOR TO BEGINNING CONSTRUCTION BY CONTACTING THE LOCAL UTILITY COMPANIES AND/OR UTILIZING THE LOCAL ONE-CALL SYSTEM ANY DAMAGE DONE TO EXISTING UTILITIES (THAT ARE TO REMAIN IN PLACE) DURING CONSTRUCTION OPERATIONS WILL BE THE CONTRACTOR'S RESPONSIBILITY AND REPAIRED AT THE CONTRACTOR'S EXPENSE.
- 9. ALL SITE WORK FOR THIS PROJECT SHALL MEET OR EXCEED THE OWNERS CONTRACT DOCUMENTS AND SPECIFICATIONS. ALL WORK SHALL MEET OR EXCEED THE RELEVANT UTILITY COMPANIES AND REGULATORY AGENCIES, CONTRACT DOCUMENTS AND SPECIFICATIONS. ALL WORK WITHIN PUBLIC AND STATE RIGHT OF WAY SHALL BE IN ACCORDANCE WITH THE GOVERNING AGENCIES STANDARDS AND SPECIFICATIONS
- 10. TRAFFIC CONTROL SHALL CONFORM TO THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD), CURRENT EDITION. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE PROPER TRAFFIC CONTROL IS IN PLACE FOR EACH PHASE OF CONSTRUCTION. THE CONTRACTOR IS ALSO RESPONSIBLE FOR PROPERLY MAINTAINING TRAFFIC CONTROL DEVICES THROUGHOUT THE DURATION OF THE WORK. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING TRAFFIC CONTROL PLANS TO THE CITY AND DEPARTMENT OF TRANSPORTATION AS REQUIRED.

WETLANDS NOTICE:

ANY DEVELOPMENT, EXCAVATION, CONSTRUCTION, OR FILLING IN A U.S. CORPS OF ENGINEERS DESIGNATED WETLAND IS SUBJECT TO LOCAL, STATE AND FEDERAL APPROVALS. THE CONTRACTOR SHALL COMPLY WITH ALL PERMIT REQUIREMENTS AND/OR RESTRICTIONS AND ANY VIOLATION WILL BE SUBJECT TO FEDERAL PENALTY. THE CONTRACTOR SHALL HOLD THE OWNER/ DEVELOPER, THE ENGINEER AND THE LOCAL GOVERNING AGENCIES HARMLESS AGAINST SUCH VIOLATION.

WARRANTY/DISCLAIMER:

THE DESIGNS REPRESENTED IN THESE PLANS ARE IN ACCORDANCE WITH ESTABLISHED PRACTICES OF CIVIL ENGINEERING FOR THE DESIGN FUNCTIONS AND USES INTENDED BY THE OWNER AT THIS TIME. HOWEVER, NEITHER THE ENGINEER NOR ITS PERSONNEL CAN OR DO WARRANT THESE DESIGNS OR PLANS AS CONSTRUCTED EXCEPT IN THE SPECIFIC CASES WHERE THE ENGINEER INSPECTS AND CONTROLS THE PHYSICAL CONSTRUCTION ON A CONTEMPORARY BASIS AT THE SITE.

NOTICE TO BIDDERS:

ALL QUESTIONS REGARDING THE PREPARATION OF THE GENERAL CONTRACTOR'S BID SHALL BE DIRECTED TO THE OWNER'S CONSTRUCTION REPRESENTATIVE. SUBCONTRACTORS MUST DIRECT THEIR QUESTIONS THROUGH THE GENERAL CONTRACTOR. THE CONSULTING ARCHITECT AND/OR THE CONSULTING ENGINEER SHALL NOT BE CONTACTED DIRECTLY WITHOUT PRIOR AUTHORIZATION FROM THE OWNER/DEVELOPER.

FLOOD CERTIFICATION:

THIS PROPERTY LIES IN ZONE "X" AND DOES NOT LOCATE WITHIN ANY PRESENTLY ESTABLISHED 100-YEAR FLOOD PLAIN, AS SHOWN BY THE FEDERAL EMERGENCY MANAGEMENT AGENCY, FLOOD INSURANCE RATE MAP FOR THE CITY OF O'FALLON, MISSOURI, COMMUNITY PANEL NUMBER 29183C0237G EFFECTIVE DATE JANUARY 20, 2016.

BENCHMARKS:

VERTICAL RELIEF, AS SHOWN, DERIVED FROM GPS OBSERVATIONS USING THE MODOT NETWORK WITH GEIOD MODEL G18US.

BM-1 IRON PIPE AT THE SW CORNER OF LOT 1, MAIN STREET PLAZA PB 32, PG 286 ELEVATION = 537.75

SITE DEVELOPMENT PLANS FOR QUIKTRIP STORE #0683

418 S. MAIN STREET O'FALLON, MISSOURI





Vicinity Map

PROJECT CONTACT LIST:

UTILITY CONTACT LIST:

ENGINEER OF RECORD CIVIL DESIGN, INC. CONTACT: NATHAN COWAN, P.E. 5220 OAKLAND AVE. ST. LOUIS, MISSOURI, 63110 TEL: 314-881-5475

SURVEYOR OF RECORD VOLZ, INC. CONTACT: MARK WILEY, L.S. 10849 INDIAN HEAD IND'L BLVD. ST. LOUIS, MISSOURI 63132

TEL: 314-426-6212

CITY OF O'FALLON

CONTACT:

CONTACT:

100 N. MAIN STREET

O'FALLON, MO, 63366

TEL: 636-379-5596

CITY OF O'FALLON

100 N. MAIN STREET

O'FALLON, MO, 63366

TEL: 636-379-7600

CONTACT: RANDY CLARK

100 N. MAIN STREET

O'FALLON, MO. 63366

TEL: 636-379-7632

STORM SEWERS

CITY OF O'FALLON

QT REAL ESTATE PROJECT MANAGER QUIKTRIP CORPORATION CONTACT: GWEN KEEN 13500 RIVERPORT DRIVE, SUITE 175 MARYLAND HEIGHTS, MISSOURI, 63043

QT CIVIL PROJECT MANAGER QUIKTRIP CORPORATION CONTACT: TRAVIS WUNSCH, P.E. 4705 SOUTH 129TH EAST AVENUE TULSA. OK 74134

TEL: 636-627-0003

TEL: 918-615-7376

AMEREN MISSOURI

CONTACT: NEIL MANNING

WENTZVILLE, MO, 63385

CONTRACT: NICHOLAS EGGERT

CONTACT: ANTHONY WILLIAMS

1120 S. TYRON STREET,

CHARLOTTE, NC 28203

TEL: 883-692-7773

200 CALLAHAN ROAD

TEL: 636-466-6903

700 MARKET STREET

TEL: 314-330-5720

ST. LOUIS, MO 63101TEL:

SPIRE GAS

MUNICIPAL CONTACT LIST:

O'FALLON PLANNING DEPARTMENT CITY OF O'FALLON 100 N. MAIN STREET CONTACT: ETHAN HENDRICKS TEL: 636-379-5467

O'FALLON ENGINEERING DEPARTMENT CITY OF O'FALLON 100 N. MAIN STREET CONTACT: RYAN ROCKWELL TEL: 636-379-7630

111 LAURA K DRIVE O'FALLON, MO 63366 TEL: 636-272-3493

O'FALLON FIRE PROTECTION DISTRICT

* C531 COMPREHENSIVE TRENCHING DETAILS II

* C570 JURISDICTIONAL DETAILS I

* C571 JURISDICTIONAL DETAILS II

* L100 LANDSCAPE PLAN

* L110 IRRIGATION PLAN

IRRIGATION DETAILS SHEET 2

* COVER RETAINING WALL COVER SHEET

RETAINING WALL PLANS

RETAINING WALL DETAILS

CITY OF O'FALLON CONDITIONS OF APPROVAL

BRIGHTSPEED

SUITE 700

STAFF RECOMMENDATIONS:

- 1. THIS APPROVAL IS CONDITIONAL UPON ALL APPLICABLE REQUIREMENTS PROVIDED WITHIN TITLE IV OF THE CITY'S MUNICIPAL CODE BEING ADDRESSED ON THE CONSTRUCTION SITE PLAN.
- 2. A FULL PARAPET WALL IS REQUIRED FOR HVAC AND MECHANICAL SCREENING. PLEASE REVISE THE ELEVATIONS. 3. THE MINIMUM PARKING SPACE SIZE IS 9' BY 19'. PLEASE ENSURE
- ALL PARKING SPACES MEET THE MINIMUM DIMENSIONS. 4. BE AWARE THAT CITY STAFF IS WORKING WITH AMEREN FOR AN ESTIMATE TO RELOCATE THE OVERHEAD POWER LINES UNDERGROUND AS PART OF THE MAIN STREET PHASE 2 PROJECT. WORK WITH STAFF TO COORDINATE THESE PLANS. COORDINATE WITH THE CITY ON MAIN STREET IMPROVEMENTS TO DETERMINE TIMING TO AVOID DUPLICATION

OF CONSTRUCTION. BUILD YOUR ENTRANCE AND SIDEWALKS AND

- MUNICIPAL CODE REQUIREMENTS:
- 1. SHOW 10' WIDE UTILITY EASEMENT BEHIND ALL RIGHT-OF-WAY.

CHARTER COMMUNICATIONS

CONTACT: CHRISTIE DOLE

CONTACT: RENOY THOMAS

ST. CHARLES, MO 63301

TEL: 636-229-1609

COLUMBIA, MO 65202

TEL: 573-239-8085

501 FOUNTAIN LAKE BLVD.

CONTACT: ADIAN HAMMOND

TEL: 888-438-2427

GATEWAY FIBER

SUITE 105

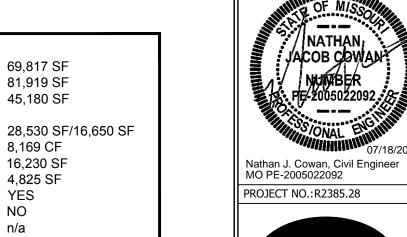
SOCKET

2703 CLARK LN

- 2. PROVIDE FIRE DISTRICT, MODNR, SANITARY AND WATER DISTRICT APPROVAL
- 3. PROVIDE A SANITARY EFFLUENT SURVEY FOR BUSINESSES IN THE O'FALLON SEWER DISTRICT.
- 4. PROVIDE THE ESTIMATED SANITARY FLOW IN GDP ON THE PLAN.
- 5. PROVIDE A STRIPED ADA ACCESS PATH FROM THE SIDEWALK TO THE

6. ADA PARKING SPACES NEED TO BE 9FT WIDE.

CITY OF O'FALLON ENGINEERING DEPARTMENT ACCEPTED FOR CONSTRUCTION BY: Ryan Rockwell DATE: 09/11/2025 PROFESSIONAL ENGINEER'S SEAL INDICATES RESPONSIBILITY FOR DESIGN



69,817 SF

81,919 SF

45,180 SF

8,169 CF

16,230 SF

4.825 SF

THE QUANTITIES ABOVE ARE INTENDED FOR INTERNAL TRACKING PURPOSES ONLY IHEY ARE NOT REPRESENTATIVE OF THE QUANTITIES FOR BIDDING PURPOSES.
THE CONTRACTOR IS RESPONSIBLE FOR CALCULATING THEIR OWN QUANTITIES.

SOD OR DECOMPOSED GRANITE AREA

ALL OTHER NON-QT SPEC PAVING

SHEET INDEX

NO. TITLE

ODS QUANTITIES:

DEVELOPED SITE (LOD)

QT SPEC ASPHALT/CONCRETE

QT SPEC CONCRETE

MASONRY CANOPY

WATER QUALITY UNIT

-UNIT TYPE:

ALTERNATE:

UG DETENTION

* C001 COVER SHEET

* C002 SITE DATA AND JURISDICTIONAL NOTES

* C030 DEMOLITION PLAN

* C100 SITE PLAN * C110 GRADING PLAN

* C111 DETAILED GRADING PLANS I

* C112 DETAILED GRADING PLANS II

* C113 DETAILED GRADING PLANS III * C114 ADA COMPLIANCE REFERENCE PLAN

* C120 STORM SEWER PLAN

* C121 PRE AND POST-DEVELOPED DRAINAGE MAP

* C122 PROPOSED SUB-AREA DRAINAGE MAP

* C130A ASPHALT PAVING PLAN

* C130B CONCRETE PAVING PLAN

* C131 BUILDING PAVING PLAN

* C140 SWPP PLAN PHASE 1 AND NOTES

* C141 SWPP PLAN PHASE 2 AND NOTES * C142 SWPP DETAILS I

* C150 UTILITY PLAN

* C151 UTILITY VERIFICATION PLAN

* C160 PHOTOMETRIC SITE PLAN

* C300 STORM SEWER PROFILES I * C301 STORM SEWER PROFILES II

* C310 STORM SEWER BMP DETAILS

* C311 STORM SEWER BMP DETAILS II

* C500 MISC. SITE DETAILS I

* C501 MISC. SITE DETAILS II

* C510 ADA DETAILS

* C520 PAVING DETAILS I * C521 PAVING DETAILS II

* C522 PAVING DETAILS III

* C523 PAVING DETAILS IV

* C524 PAVING DETAILS V

* C530 COMPREHENSIVE TRENCHING DETAILS I

* C540 DRAINAGE DETAILS I

* C572 JURISDICTIONAL DETAILS III

* L500 LANDSCAPE DETAILS

IRRIGATION DETAILS SHEET 1

RETAINING WALL ELEVATIONS

RETAINING WALL NOTES & DETAILS



PLANNING AND DEVELOPMENT DIVISION FILE NO. 25-000086



PROJECT NO.:R2385.28 CIVIL DESIGN,INC 5220 Oakland Ave. St. Louis, MO 63110 (314) 863-5570 Missouri State Certificate of

Authority #2002006804

9

0 2 uikTrip Q



ANY UNAUTHORIZED USE, REPRODUCTION. WHOLE OR IN PART, IS STRICTLY FORBIDDEN PROTOTYPE: P-120 (11/24/2024) DIVISION: VERSION: 001 DESIGNED BY: DRAWN BY: **REVIEWED BY:**

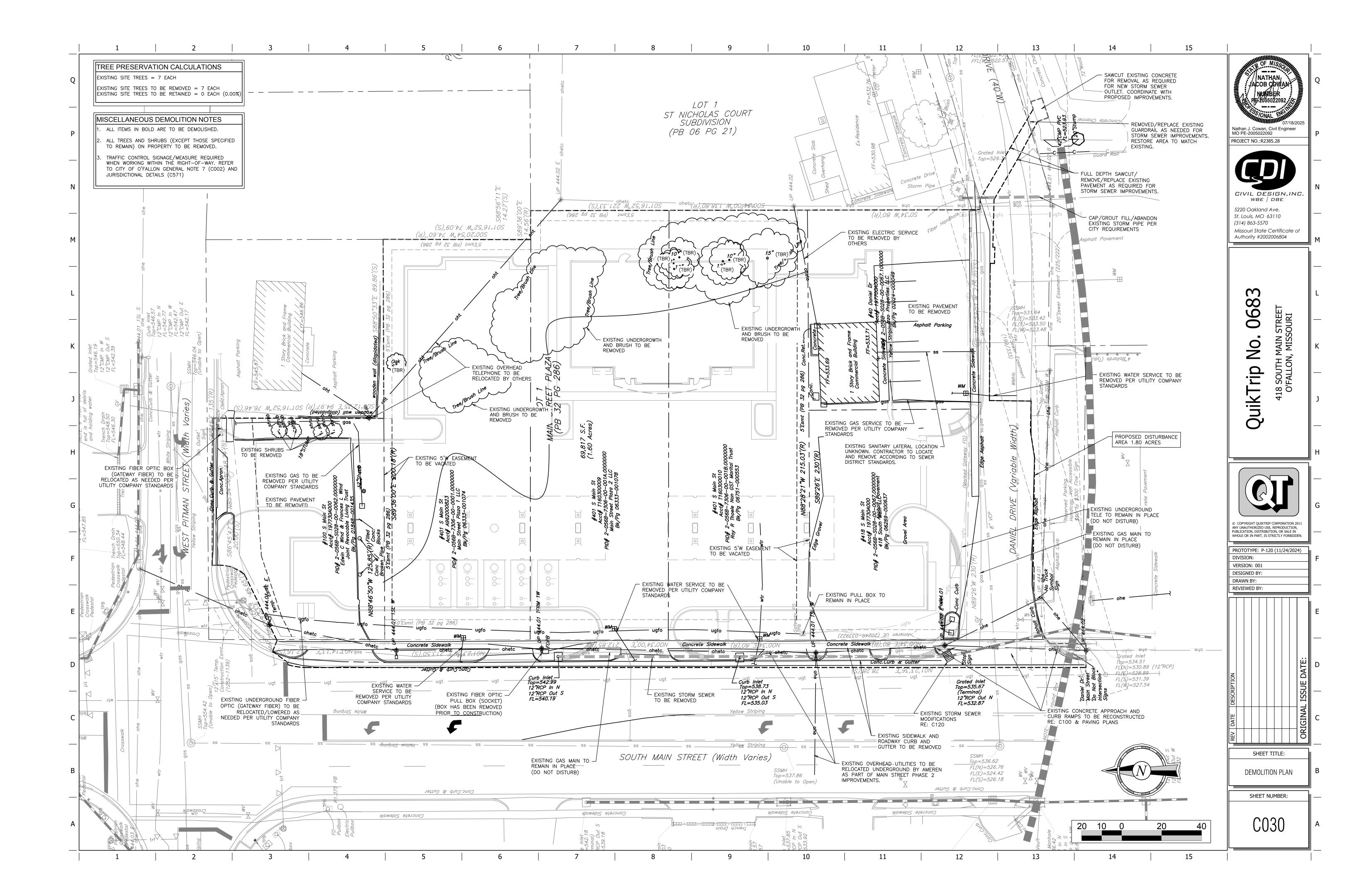
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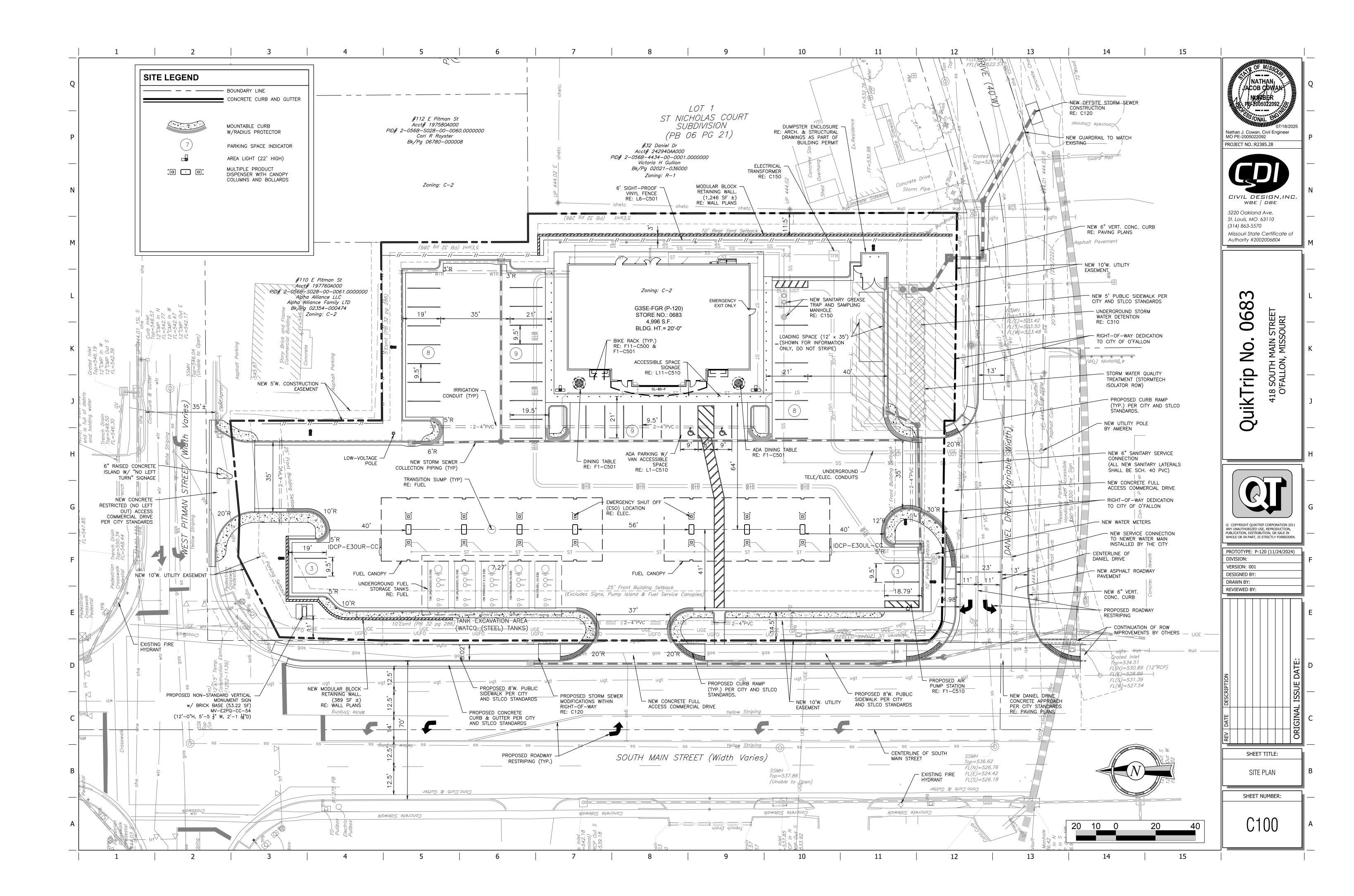
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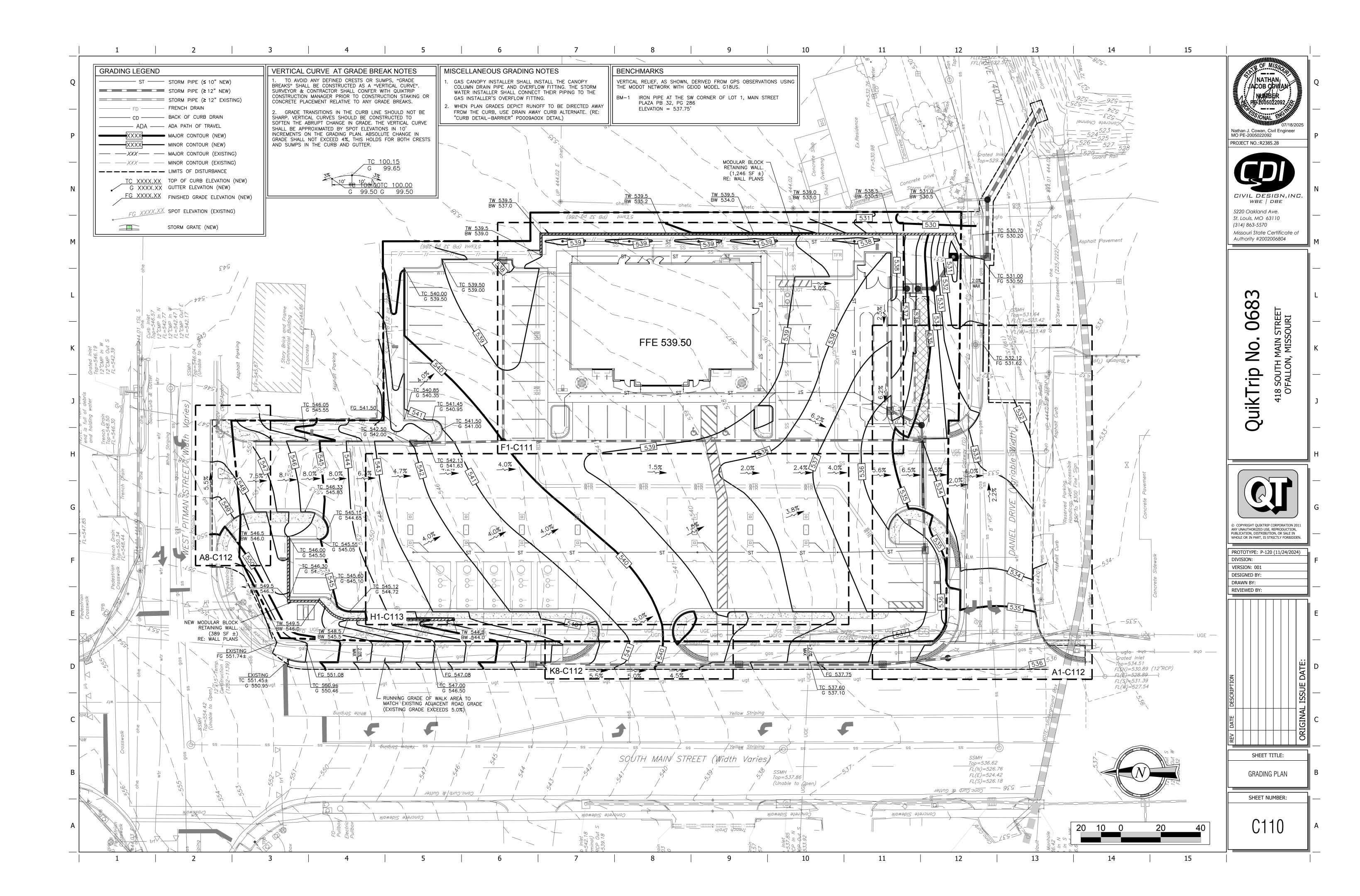
COVER SHEET

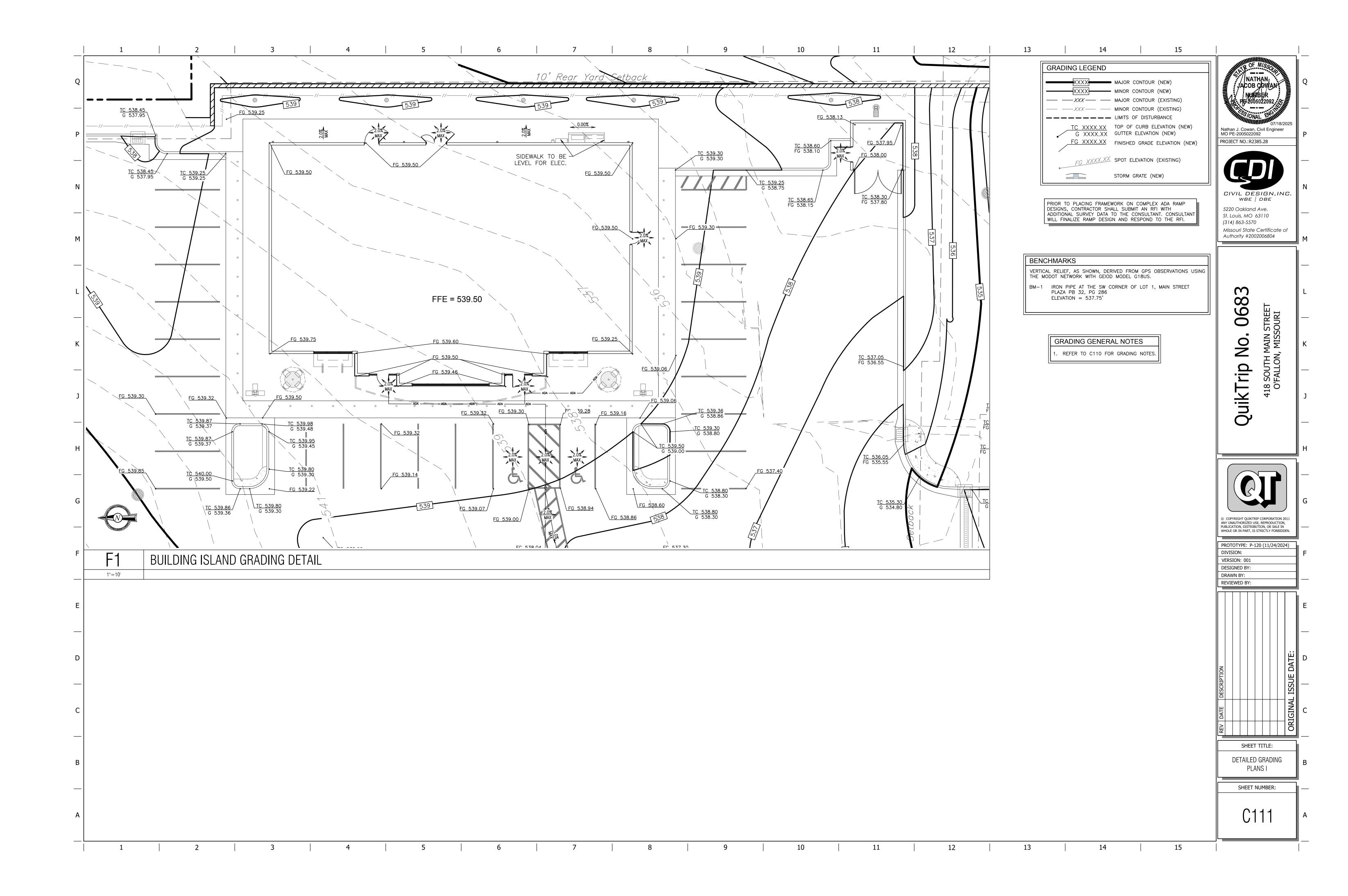
CURB AND GUTTER.

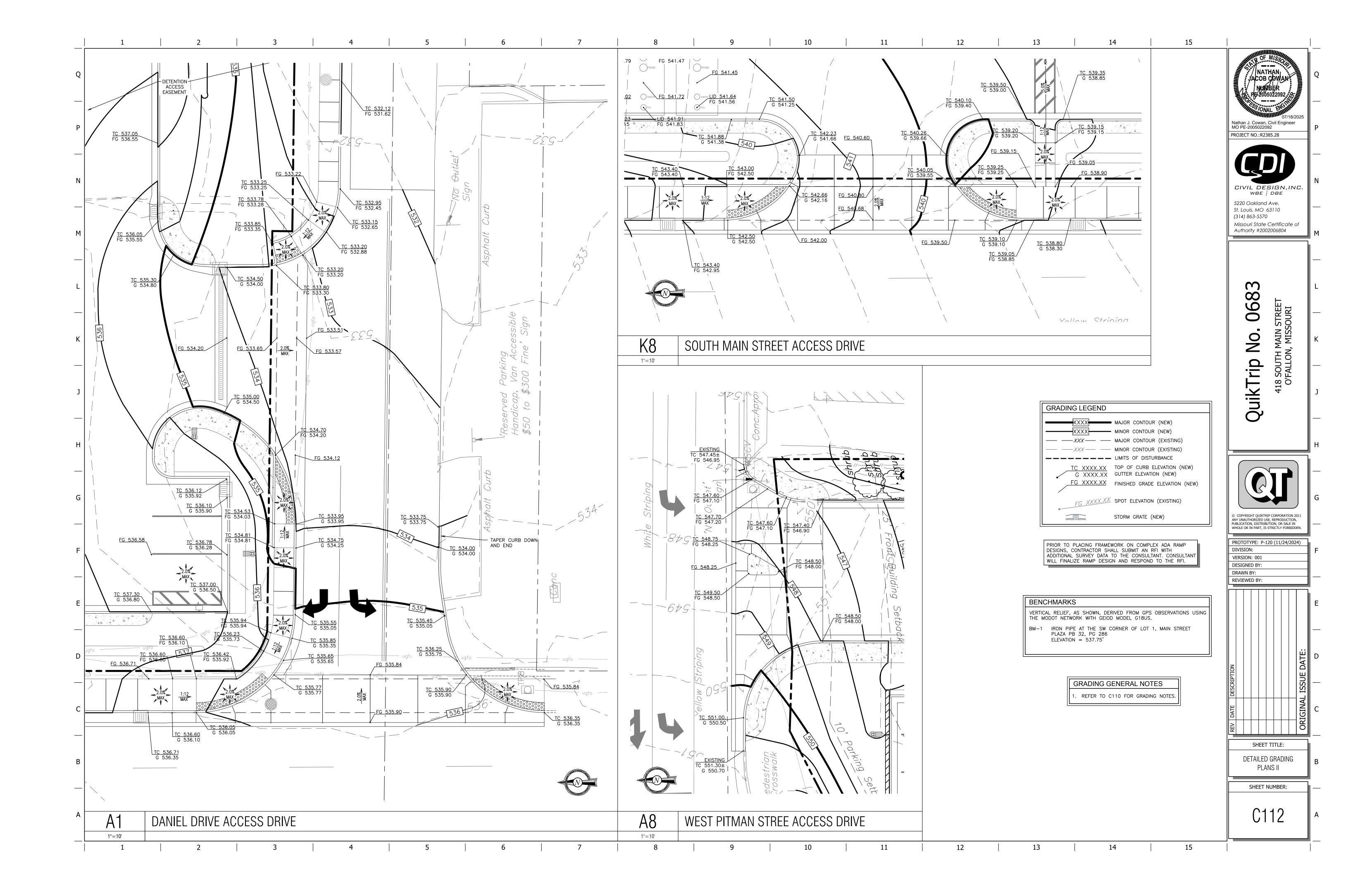
TY OF O'FALLON GENERAL NOTES: DRIVEWAY LOCATIONS SHALL NOT INTERFERE WITH THE SIDEWALK HANDICAP RAMPS, OR CURB INLET SUMPS	CITY OF O'FALLON SANITARY SEWER NOTES: 1. ALL SANITARY SEWER INSTALLATION IS TO BE IN ACCORDANCE WITH M.S.D. STANDARDS AND SPECIFICATIONS EXCEPT AS MODIFIED BY THE CITY OF O'FALLON	SITE DATA AND NOTES:	MINIMA OF M
SIDEWALKS, CURB RAMPS, RAMPS AND ACCESSIBLE PARKING SPACES SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE CURRENT APPROVED "AMERICAN WITH DISABILITIES ACT ACCESSIBILITY GUIDELINES" (ADAAG) ALONG WITH THE REQUIRED GRADES, CONSTRUCTION MATERIALS, SPECIFICATIONS AND SIGNAGE. IF ANY CONFLICT OCCURS BETWEEN THE ABOVE INFORMATION AND THE PLANS. THE ADAAG GUIDELINES SHALL TAKE PRECEDENCE AND THE CONTRACTOR PRIOR TO ANY CONSTRUCTION	ORDINANCES. 2. BRICK SHALL NOT BE USED IN THE CONSTRUCTION OF SANITARY SEWER STRUCTURES. PRE CAST CONCRETE STRUCTURES ARE TO BE USED UNLESS OTHERWISE	EXISTING/PROPOSED ZONING: C-2 DOWNTOWN OVERLAY DISTRICT	// NATH
SHALL NOTIFY THE PROJECT ENGINEER. .1. TRUNCATED DOMES FOR CURB RAMPS LOCATED IN PUBLIC RIGHT OF WAY SHALL MEET PROWAG REQUIREMENTS AND SHALL BE CONSTRUCTED USING RED PRE—CAST	APPROVED BY THE CITY OF O'FALLON. 3. CONNECTIONS AT ALL SANITARY STRUCTURES ARE TO BE MADE WITH A-LOCK JOINT OR EQUAL 4. ALL SANITARY LATERALS SHALL BE A MINIMUM OF 4" RESIDENTIAL, 6" COMMERCIAL DIAMETER PIPE.	PROPOSED USE: CONVENIENCE/FOOD STORE	NUMAB 12. PE-20050
TRUNCATED DOMES PER PAVEMENT DETAILS. ANY PROPOSED PAVILIONS OR PLAYGROUND AREAS WILL NEED A SEPARATE PERMIT FROM THE BUILDING DIVISION. THE CONTRACTOR IS DESCRIBED TO CALL MISSOURI ONE CALL AND THE CITY OF C'EALLON FOR THE LOCATION OF HEILITIES. CONTACT THE CITY OF C'EALLON (CZC)	5. ALL SANITARY MAINS SHALL BE A MINIMUM OF 4" RESIDENTIAL, O" COMMERCIAL DIAMETER PIPE. 6. ALL SANITARY SEWER LINE WITH A SLOPE GREATER THAN 20% WILL REQUIRE CONCRETE CRADLE OR CONCRETE COLLAR AT EACH PIPE JOINT. SANITARY LINE WITH A	W/ PETROLEUM SERVICES	
THE CONTRACTOR IS RESPONSIBLE TO CALL MISSOURI ONE CALL AND THE CITY OF O'FALLON FOR THE LOCATION OF UTILITIES. CONTACT THE CITY OF O'FALLON (636) 379-3814 FOR THE LOCATION OF CITY MAINTAINED CABLE FOR STREET LIGHTS AND TRAFFIC SIGNALS, ALL OTHER UTILITIES CALL MISSOURI ONE CALL 1-800-DIG-RITE. 1-800-344-7483	SLOPE GREATER THAN 50% WILL REQUIRE A SPECIAL APPROVED DESIGN AS SHOWN ON DETAIL SHEET. 7. ALL MANHOLES BUILT WITHIN THE 100 YEAR FLOOD PLAIN MUST HAVE LOCK TYPE WATERTIGHT MANHOLE COVERS.	EXISTING TOTAL SITE AREA: 69,817 SF (1.60 ACRES)	Nathan J. Cowan, Ci
ALL PROPOSED UTILITIES AND/OR UTILITY RELOCATIONS SHALL BE LOCATED UNDERGROUND. ALL PROPOSED FENCING REQUIRES A SEPARATE PERMIT THROUGH THE BUILDING SAFETY DIVISION. ALL CONSTRUCTION OPERATIONS AND WORK ZONE TRAFFIC CONTROL WITHIN THE RIGHT OF WAY WILL FOLLOW MODOT OR M.U.T.C.D. STANDARDS WHICHEVER IS MORE STRINGENT.	8. ALL SANITARY SEWER MAINS MUST HAVE A MINIMUM OF 42" COVER. 9. WHEN SANITARY MAINS CROSS OVER STORM LINE THE SANITARY MAIN MUST BE DUCTILE IRON PIPE FOR 10 FEET ON EACH SIDE OF THE CROSSING. 10. ENCASE WITH CONCRETE BOTH SANITARY AND STORM SEWER AT CROSSING WHEN STORM SEWER IS WITHIN 18 INCHES ABOVE SANITARY SEWER. ADD CONCRETE CRADLE TO ONLY RCP STORM SEWER AND ENCASE FLEXIBLE STORM SEWER WHEN IT IS MORE THAN 18 INCHES ABOVE SANITARY LINE. SHOW ON PROFILE SHEET. 11. THE SANITARY SEWERS SHOULD RUN DIAGONALLY THROUGH THE SIDE YARDS TO MINIMIZE ANY ADDITIONAL UTILITY EASEMENTS REQUIRED.	PROPOSED TOTAL SITE AREA: 68,163 SF (1.56 ACRES) (AFTER DANIEL DR. ROW DEDICATION)	PROJECT NO.:R2385
(INTENTIONALLY OMITTED) ALL SUBDIVISION IDENTIFICATION OR DIRECTIONAL SIGN(S) MUST HAVE THE LOCATIONS AND SIZES APPROVED AND PERMITTED SEPARATELY THROUGH THE PLANNING AND DEVELOPMENT DIVISION. MATERIALS SUCH AS TREES, ORGANIC DEBRIS, RUBBLE, FOUNDATIONS, AND OTHER DELETERIOUS MATERIAL SHALL BE REMOVED FROM THE SITE AND DISPOSED OF IN	12. ALL SANITARY SEWER STRUCTURES SHALL BE WATERPROOFED ON THE EXTERIOR IN ACCORDANCE TO MISSOURI DNR SPECIFICATIONS 10CSR-8.120 (7)(E). 13. ALL SANITARY SEWER PIPE SHALL BE SDR35 OR EQUAL. ALL SANITARY SEWER LATERALS SHALL BE SCHEDULE 40. 14. ALL SANITARY SEWER MANHOLES AND PIPES WILL BE TESTED TO THE FOLLOWING SPECIFICATIONS. ASTM C1244, STANDARD TESTING METHOD FOR CONCRETE SEWER	BUILDING INFORMATION: NEW 1-STORY BUILDING 6,262 SF	
COMPLIANCE WITH ALL APPLICABLE LAWS AND REGULATIONS. IF THE MATERIAL LISTED PREVIOUSLY ARE REUSED, A LETTER FROM A SOIL ENGINEER MUST CLARIFY AMOUNT, LOCATION, DEPTH. ETC. AND BE APPROVED WITH THE CONSTRUCTION PLANS. LANDFILL TICKETS FOR SUCH DISPOSAL SHALL BE MAINTAINED ON FILE BY THE DEVELOPER. BURNING ON SITE SHALL BE ALLOWED ONLY BY PERMIT FROM THE LOCAL FIRE DISTRICT. IF A BURN PIT IS PROPOSED THE LOCATION AND MITIGATION SHALL BE SHOWN ON THE GRADING PLAN AND DOCUMENTED BY THE SOILS ENGINEER.	MANHOLE BY NEGATIVE AIR PRESSURE (VACUUM), LATEST REVISION ASTM F1417, STANDARD TESTING METHOD FOR INSTALLATION ACCEPTANCE OF PLASTIC GRAVITY SEWER LINES USING LOW PRESSURE AIR, LATEST REVISION. 15. ADD 1" MINUS ROCK BACK FILL TO ALL SANITARY SEWER AND ALL OTHER UTILITIES THAT LIE WITHIN THE 1:1 SHEAR PLANE OF THE ROAD. CITY OF O'FALLON STORM SEWER NOTES:	BUILDING SETBACKS: FRONT 25' (EXCLUDES SIGNS, PUMP ISLANDS AND CANOPIES OF GASOLINE SERVICE	CIVIL DES
1. TWENTY-FOUR (24) HOURS PRIOR TO STARTING ANY OF THE WORK COVERED BY THE ABOVE PLANS AND AFTER APPROVAL THEREOF, THE DEVELOPER SHALL MAKE ARRANGEMENTS WITH THE CONSTRUCTION INSPECTION OFFICE TO PROVIDE FOR INSPECTION OF THE WORK, SUFFICIENT IN THE OPINION OF THE CITY ENGINEER, TO ASSURE COMPLIANCE WITH THE PLANS AND SPECIFICATIONS AS APPROVED. 2. THE CITY ENGINEER OR THEIR DULY AUTHORIZED REPRESENTATIVE SHALL MAKE ALL NECESSARY INSPECTIONS OF CITY INFRASTRUCTURE, ESCROW ITEMS OR	1. ALL STORM SEWER INSTALLATION IS TO BE IN ACCORDANCE WITH M.S.D. STANDARDS AND SPECIFICATIONS EXCEPT AS MODIFIED BY THE CITY OF O'FALLON ORDINANCES. 2. BRICK SHALL NOT BE USED IN THE CONSTRUCTION OF STORM SEWER STRUCTURES. PRE CAST CONCRETE STRUCTURES ARE TO BE USED UNLESS OTHERWISE APPROVED	STATIONS) SIDE O' REAR O'	WBE 1 5220 Oakland A St. Louis, MO 63
INFRASTRUCTURE LOCATED ON THE APPROVED PLANS. 3. ALL INSTALLATIONS AND CONSTRUCTION SHALL CONFORM TO THE APPROVED ENGINEERING DRAWINGS. HOWEVER, IF THE DEVELOPER CHOOSES TO MAKE MINOR	BY THE CITY OF O'FALLON. 3. A 5/8" TRASH BAR SHALL BE INSTALLED HORIZONTALLY IN THE CENTER OF THE OPENING(S) IN ALL CURB INLETS AND AREA INLETS. 4. (INTENTIONALLY OMITTED)	PARKING SETBACKS: FRONT 10'	(314) 863-5570
MODIFICATIONS IN DESIGN AND/OR SPECIFICATIONS DURING CONSTRUCTION, HE/SHE SHALL MAKE SUCH CHANGES AT HIS/HER OWN RISK, WITHOUT ANY ASSURANCE THAT THE CITY ENGINEER WILL APPROVE THE COMPLETED INSTALLATION OR CONSTRUCTION. IT SHALL BE THE RESPONSIBILITY OF THE DEVELOPER TO NOTIFY THE CITY ENGINEER OF ANY CHANGES FROM THE APPROVED DRAWINGS. THE DEVELOPER MAY BE REQUIRED TO CORRECT THE INSTALLED IMPROVEMENTS SO AS TO CONFIRM TO THE APPROVED ENGINEERING DRAWINGS. THE DEVELOPER MAY REQUEST A LETTER FROM THE CONSTRUCTION DIVISION REGARDING ANY FIELD CHANGES	5. ENCASE WITH CONCRETE BOTH SANITARY AND STORM SEWER AT CROSSING WHEN STORM SEWER IS WITHIN 18 INCHES ABOVE SANITARY SEWER. ADD CONCRETE CRADLE TO ONLY RCP STORM SEWER AND ENCASE FLEXIBLE STORM SEWER WHEN IT IS MORE THAN 18 INCHES ABOVE SANITARY LINE. SHOW ON PROFILE SHEET. 6. THE STORM SEWERS SHOULD RUN DIAGONALLY THROUGH THE SIDE YARDS TO MINIMIZE ANY ADDITIONAL UTILITY EASEMENTS REQUIRED. 7. ALL CONCRETE PIPES WILL BE INSTALLED WITH O-RING RUBBER TYPE GASKETS.	SIDE 10' REAR 10'	Missouri State C Authority #20020
APPROVED BY THE CITY INSPECTORS. 4. CITY APPROVAL OF THE CONSTRUCTION SITE PLANS DOES NOT MEAN THAT ANY BUILDING CAN BE CONSTRUCTED ON THE LOTS WITHOUT MEETING THE BUILDING SETBACKS AS REQUIRED BY THE ZONING CODE.	8. CONNECTIONS AT ALL STORM STRUCTURES ARE TO BE MADE WITH A-LOCK JOINT OR EQUAL. 9. PRE CAST CONCRETE INLET COVERS ARE NOT TO BE USED.	BUFFER YARD: REAR 10' (PROPOSED)	
TY OF O'FALLON GRADING NOTES:	10. THE SWALE IN THE DETENTION BASINS SHALL HAVE A MINIMUM 2% LONGITUDINAL SLOPE AND BE LINED WITH A PERMANENT EROSION CONTROL BLANKET THAT WILL ALLOW INFILTRATION OF STORM WATER. 11. ALL STRUCTURES AND FLARED END SECTIONS MUST BE CONCRETE. H.D.P.E. PIPE WILL NOT BE ALLOWED FOR DETENTION BASIN OUTFLOWS. FINAL PIPE RUN TO	PARKING CALCULATIONS:	
DEVELOPER MUST SUPPLY CITY CONSTRUCTION INSPECTORS WITH AN ENGINEER'S SOIL REPORTS PRIOR TO AND DURING SITE GRADING. THE SOIL REPORT WILL BE REQUIRED TO CONTAIN THE FOLLOWING INFORMATION ON SOIL TEST CURVES (PROCTOR REPORTS) FOR PROJECTS WITHIN THE CITY: 1.1. MAXIMUM DRY DENSITY	DETENTION BASINS, CREEK DISCHARGE OR OTHER APPROVED MEANS. 12. (INTENTIONALLY OMITTED) 13. RIP RAP SHOWN AT FLARED END SECTIONS WILL BE EVALUATED IN THE FIELD BY THE ENGINEER, CONTRACTOR, AND CITY INSPECTORS AFTER INSTALLATION FOR	PARKING REQUIRED = 32 SPACES 1 SPACE PER 250 SF OF FLOOR AREA PLUS 1 SPACE FOR EACH 2 PUMPS (6.363 SE / 350 SE	m m
1.2. OPTIMUM MOISTURE CONTENT 1.3. MAXIMUM AND MINIMUM ALLOWABLE MOISTURE CONTENT 1.4. CURVE MUST BE PLOTTED TO SHOW DENSITY FROM A MINIMUM OF 90% COMPACTION AND ABOVE AS DETERMINED BY THE "MODIFIED AASHTO T-180 COMPACTION	EFFECTIVENESS AND FIELD MODIFIED, IF NECESSARY TO REDUCE EROSION ON AND OFF SITE. 14. ADD 1" MINUS ROCK BACK FILL TO ALL STORM SEWER THAT LIE WITHIN THE 1:1 SHEAR PLANE OF THE ROAD. 15. (INTENTIONALLY OMITTED) 16. REFER TO JURISDICTIONAL DETAILS FOR STORM SEWER INLET MARKING DETAILS.	(6,262 SF / 250 SF = 25 SPACES) (14 PUMPS / 2 = 7 SPACES) LOADING SPACE REQUIRED = 1 SPACE (12'x35')	89
TEST" (A.S.T.M.—D—1157) OR FROM A MINIMUM OF 95% AS DETERMINED BY THE "STANDARD PROCTOR TEST ASSHTO T—99, METHOD C" (A.S.T.M.—D—698). PROCTOR TYPE MUST BE DESIGNATED ON DOCUMENT. 1.5. CURVE MUST HAVE AT LEAST 5 DENSITY POINTS WITH MOISTURE CONTENT AND SAMPLE LOCATIONS LISTED ON DOCUMENT	CITY OF O'FALLON EROSION CONTROL NOTES:		0
1.6. SPECIFIC GRAVITY 1.7. NATURAL MOISTURE CONTENT	1. THE PERMITTEE SHALL ASSUME COMPLETE RESPONSIBILITY FOR CONTROLLING ALL SILTATION AND EROSION OF THE PROJECT AREA. THE PERMITTEE SHALL USE WHATEVER MEANS NECESSARY TO CONTROL EROSION AND SILTATION INCLUDING, BUT NOT LIMITED TO, STAKED STRAW BALES AND/OR SILTATION FABRIC FENCES	PARKING PROPOSED = 54 SPACES (INCLUDES 14 SPACES AT FUEL PUMPS)	o .
1.8. LIQUID LIMIT 1.9. PLASTIC LIMIT BE ADVISED THAT IF THIS INFORMATION IS NOT PROVIDED TO THE CITY'S CONSTRUCTION INSPECTOR THE CITY WILL NOT ALLOW GRADING OR CONSTRUCTION ACTIVITIES	(POSSIBLE METHODS OF CONTROL ARE DETAILED IN THE PLAN). CONTROL SHALL COMMENCE WITH THE CLEARING OPERATIONS AND BE MAINTAINED THROUGHOUT THE PROJECT UNTIL ACCEPTANCE OF THE WORK BY CITY OF O'FALLON AND AS NEEDED BY MODOT. THE PERMITTEE'S RESPONSIBILITIES INCLUDE ALL DESIGN AND	(INCLUDES 1 STANDARD & 1 VAN ACCESSIBLE SPACE AT BUILDING AND 2 ACCESSIBLE FUEL PUMPS)	Ž
TO PROCEED ON ANY PROJECT SITE ALL FILL PLACED IN AREAS OTHER THAN PROPOSED STORM SEWERS, SANITARY SEWERS, PROPOSED ROADS, AND PAVED AREAS SHALL BE COMPACTED FROM THE BOTTOM OF THE FILL UP IN 8" LIFTS AND COMPACTED TO 90% MAXIMUM DENSITY AS DETERMINED BY MODIFIED AASHTO T—180 COMPACTION TEST OR 95% OF	IMPLEMENTATION AS REQUIRED TO PREVENT EROSION AND THE DEPOSITING OF SILT. THE CITY OF O'FALLON AND AS REQUIRED BY MODOT MAY AT THEIR OPTION DIRECT THE PERMITTEE IN HIS METHODS AS DEEMED FIT TO PROTECT PROPERTY AND IMPROVEMENTS. ANY DEPOSITING OF SILT OR MUD ON NEW OR EXISTING PAVEMENT SHALL BE REMOVED IMMEDIATELY. ANY DEPOSITING OF SILTS OR MUD IN NEW OR EXISTING STORM SEWERS AND/OR SWALES SHALL BE REMOVED AFTER EACH RAIN AND	LOADING SPACE PROVIDED = 1 SPACE (12'x35')	d
MAXIMUM DENSITY AS DETERMINED BY THE STANDARD PROCTOR TEST AASHTO T-99. ENSURE THE MOISTURE CONTENT OF THE SOIL IN FILL AREAS CORRESPONDS TO THE COMPACTIVE EFFORT AS DEFINED BY THE STANDARD OR MODIFIED PROCTOR TEST. OPTIMUM MOISTURE CONTENT SHALL BE DETERMINED USING THE SAME TEST THAT	AFFECTED AREAS CLEANED TO THE SATISFACTION OF THE CITY OF O'FALLON AND AS REQUIRED BY MODOT." 2. ALL EROSION CONTROL SYSTEMS ARE TO BE INSPECTED AND CORRECTED WEEKLY, ESPECIALLY WITHIN 48 HOURS OF ANY RAIN STORM RESULTING IN ONE—QUARTER	BICYCLE PARKING CALCULATIONS:	;=
WAS USED FOR COMPACTION. SOIL COMPACTION CURVES SHALL BE SUBMITTED TO THE CITY OF O'FALLON PRIOR TO THE PLACEMENT OF FILL. THE SURFACE OF THE FILL SHALL BE FINISHED SO IT WILL NOT IMPOUND WATER. IF AT THE END OF A DAYS WORK IT WOULD APPEAR THAT THERE MAY BE RAIN PRIOR TO THE NEXT WORKING DAY. THE SURFACE SHALL BE FINISHED SMOOTH. IF THE SURFACE HAS BEEN FINISHED SMOOTH FOR ANY REASON. IT SHALL BE SCARIFIED	INCH OF RAIN OR MORE. ANY SILT OR DEBRIS LEAVING THE SITE AND AFFECTING PUBLIC RIGHT OF WAY OR STORM WATER DRAINAGE FACILITIES SHALL BE CLEANED UP WITHIN 24 HOURS AFTER THE END OF THE STORM. 3. EROSION CONTROL DEVICES (SILT FENCE, SEDIMENT BASIN, ETC.) SHALL BE IN ACCORDANCE WITH MISSOURI DEPARTMENT OF NATURAL RESOURCES PROTECTING WATER	BICYCLE PARKING REQUIRED = 4 RACK SPACES	
BEFORE PROCEEDING WITH THE PLACEMENT OF SUCCEEDING LIFTS. FILL SHALL NOT BE PLACED ON FROZEN GROUND, NOR SHALL FILLING OPERATIONS CONTINUE WHEN THE TEMPERATURE IS SUCH AS TO PERMIT THE LAYER UNDER PLACEMENT TO FREEZE. ALL SEDIMENT AND DETENTION BASINS ARE TO BE CONSTRUCTED DURING THE INITIAL PHASE OF THE GRADING OPERATION OR IN ACCORDANCE WITH THE APPROVED	QUALITY — A FIELD GUIDE TO EROSION, SEDIMENT AND STORMWATER BEST MANAGEMENT PRACTICES FOR DEVELOPMENT SITES IN MISSOURI AND KANSAS. 4. THIS DEVELOPMENT IS REQUIRED TO PROVIDE LONG TERM POST CONSTRUCTION BMP'S SUCH AS; LOW IMPACT DESIGN, SOURCE CONTROL AND TREATMENT CONTROLS THAT PROTECTS WATER QUALITY AND CONTROLS RUN OFF TO MAXIMUM EXTENT PRACTICAL IN COMPLIANCE WITH PHASE II ILLICIT STORM WATER DISCHARGE GUIDELINES.	1 RACK SPACE PER 15 REQUIRED AUTO PARKING SPACE (4 RACK MIN.) (32 AUTO SPACES / 15 = 2.1 SPACES, THEREFORE USE MINIMUM)	
SWPPP. WHEN GRADING OPERATIONS ARE COMPLETE OR SUSPENDED FOR MORE THAN 14 DAYS, PERMANENT GRASS MUST BE ESTABLISHED AT SUFFICIENT DENSITY TO PROVIDE EROSION CONTROL ON SITE. BETWEEN PERMANENT GRASS SEEDING PERIODS, TEMPORARY COVER SHALL BE PROVIDED ACCORDING TO MISSOURI DEPARTMENT OF NATURAL RESOURCES PROTECTING WATER QUALITY — A FIELD GUIDE TO EROSION, SEDIMENT AND STORMWATER BEST MANAGEMENT PRACTICES FOR DEVELOPMENT SITES IN MISSOURI AND KANSAS.ALL FINISHED GRADES (AREAS NOT TO BE DISTURBED BY IMPROVEMENTS) IN EXCESS OF 20% SLOPES (5:1) SHALL BE MULCHED AND TACKED	(ORD. 5082, SECTION 405.245) 5. GRADED AREAS SHALL BE SEEDED AND MULCHED (STRAWED) WITHIN 14 DAYS OF STOPPING LAND DISTURBANCE ACTIVITIES. UNLESS IT CAN BE SHOWN TO THE CITY ENGINEER THAT WEATHER CONDITIONS ARE NOT FAVORABLE, VEGETATIVE GROWTH IS TO BE ESTABLISHED WITHIN 6 WEEKS OF STOPPING GRADING WORK ON THE PROJECT. THE VEGETATIVE GROWTH ESTABLISHED SHALL BE SUFFICIENT TO PREVENT EROSION AND THE STANDARD SHALL BE AS REQUIRED BY EPA AND DNR. (70%	BICYCLE PARKING PROPOSED = 4 RACK SPACES (2 RACKS WITH 2 SPACES PER RACK)	0
IN MISSOURI AND KANSAS.ALL FINISHED GRADES (AREAS NOT TO BE DISTURBED BY IMPROVEMENTS) IN EXCESS OF 20% SLOPES (5:1) SHALL BE MULCHED AND TACKED AT A RATE OF 100 POUNDS PER 1000 SQUARE FEET WHEN SEEDED. . NO SLOPES SHALL EXCEED 3 (HORIZONTAL): 1 (VERTICAL) UNLESS OTHERWISE APPROVED BY THE SOILS REPORT AND SPECIFICALLY LOCATED ON THE PLANS AND	COVERAGE PER SQUARE FOOT) ORD. 6496, SECTION 405.095	SANITARY CALCULATION:	
APPROVED BY THE CITY ENGINÈER. ALL LOW PLACES WHETHER ON SITE OR OFF SHALL BE GRADED TO PROVIDE DRAINAGE WITH TEMPORARY DITCHES. ANY EXISTING WELLS AND/OR SPRINGS WHICH MAY EXIST ON THE PROPERTY MUST BE SEALED IN A MANNER ACCEPTABLE TO THE CITY OF O'FALLON CONSTRUCTION	CITY OF O'FALLON ROADWAY NOTES: 1. ALL PAVING (PUBLIC AND PRIVATE) TO BE IN ACCORDANCE WITH ST. LOUIS COUNTY STANDARDS AND SPECIFICATIONS EXCEPT AS MODIFIED BY THE CITY OF O'FALLON	ESTIMATED PROPOSED SANITARY FLOW = 5,500 GALLONS/DAY	
INSPECTION DEPARTMENT AND FOLLOWING MISSOURI DEPARTMENT OF NATURAL RESOURCES STANDARDS AND SPECIFICATIONS. (INTENTIONALLY OMITTED)	ORDINANCES.` 2. IF THE INTERSECTING ROAD DOES NOT HAVE A CURB, THEN THE CURB ON THE NEW ENTRANCE SHALL BEGIN 10' FROM THE EDGE OF THE EXISTING ROAD.	ADDITIONAL SITE NOTES:	
D. ALL TRENCH BACK FILLS UNDER PAVED AREAS SHALL BE GRANULAR BACK FILL, AND COMPACTED MECHANICALLY. ALL OTHER TRENCH BACK FILLS MAY BE EARTH MATERIAL (FREE OF LARGE CLODS, OR STONES) AND COMPACTED USING EITHER MECHANICAL TAMPING OR WATER JETTING, GRANULAR MATERIAL AND EARTH MATERIAL MATERIAL (FREE OF LARGE CLODS, OR STONES) AND COMPACTED USING EITHER MECHANICAL TAMPING OR WATER JETTING, GRANULAR MATERIAL AND EARTH MATERIAL	3. PROVIDE 6" OF CONCRETE OVER 5" OF AGGREGATE BASE ROCK OR ASPHALT EQUIVALENT FOR MINOR RESIDENTIAL STREETS PER CITY CODE 405.370. 3.1. ROCK TO MEET THE ALL THE REQUIREMENTS OF MODOT TYPE 5 ROCK WITH A TIGHTER RESTRICTION ON THE FINES BEING THAT NO MORE THAN TEN PERCENT (10%) FINES SHALL PASS A NO. 200 SIEVE. (CITY CODE 405.210.B.1) THE GRADATION OF THIS ROCK NEEDS TO BE SUBMITTED TO THE CITY FOR APPROVAL.	 AII PROPOSED SIGNAGE SHALL COMPLY WITH THE CITY OF O'FALLON SIGN REGULATIONS AND SEPARATE PERMITS WILL BE REQUIRED. 	
ASSOCIATED WITH NEW CONSTRUCTION OUTSIDE OF PAVEMENTS MAY BE JETTED, TAKING CARE TO AVOID DAMAGE TO NEWLY LAID SEWERS. THE JETTING SHALL BE PERFORMED WITH A PROBE ROUTE ON NOT GREATER THAN 7.5 FOOT CENTERS WITH THE JETTING PROBE CENTERED OVER AND PARALLEL WITH THE DIRECTION OF THE PIPE. TRENCH WIDTHS GREATER THAN 10 FEET WILL REQUIRE MULTIPLE PROBES EVERY 7.5 FOOT CENTERS.	ANY DELIVERIES MADE WITHOUT THE PROPER DELIVERY TICKET, INCLUDING SIGNATURE, WILL NOT BE ACCEPTED. THE DELIVERY TICKET MUST LIST THE PROJECT NAME OR JOBSITE LOCATION. A SEPARATE CERTIFICATION SHEET MAY BE PROVIDED ATTACHED TO THE DELIVERY TICKET WITH A SIGNATURE OF THE COMPANY'S QUALITY	2. SITE IMPROVEMENTS SHALL BE IN COMPLIANCE WITH CITY OF	
0.1. DEPTH, TRENCH BACK FILLS LESS THAN 8 FEET DEEP SHALL BE PROBED TO A DEPTH EXTENDING HALF THE DEPTH OF THE TRENCH BACK FILL, BUT NOT LESS THAN 3 FEET. TRENCH BACK FILL GREATER THAN 8 FEET IN DEPTH SHALL BE PROBED TO HALF THE DEPTH OF THE TRENCH BACK FILL BUT NOT GREATER THAN 8 FEET.	CONTROL MANAGER. THE QUALITY CONTROL CERTIFICATION MUST BE CURRENT AND DATED WITHIN 4 WEEKS OF THE DELIVERY. (CITY CODE 405.210.A.2.K) 4. MULTI-USE TRAIL (WHEN REQUIRED) SHALL HAVE A MINIMUM OF 3" TYPE "C" ASPHALT OVER 4" AGGREGATE BASE PER CITY REQUIREMENTS. 5. TYPE C (BP-1) COMPACTION REQUIREMENTS SHALL BE 98% MINIMUM DENSITY ACCORDING TO ST. LOUIS CO. STANDARD SPECIFICATIONS.	O'FALLON ARTICLE XIII, PERFORMANCE STANDARDS.	© COPYRIGHT QUIKTRIF
10.2. EQUIPMENT, THE JETTING PROBE SHALL BE A METAL PIPE WITH AN INTERIOR DIAMETER OF 1.5 TO 2 INCHES. 10.3. METHOD, JETTING SHALL BE PERFORMED FROM THE LOWEST SURFACE TOPOGRAPHIC POINT AND PROCEED TOWARD THE HIGHEST POINT, AND FROM THE BOTTOM OF THE TRENCH BACK FILL TOWARD THE SURFACE. THE FLOODING OF EACH JETTING PROBE SHALL BE STARTED SLOWLY ALLOWING SLOW SATURATION OF THE SOIL.	6. PROVIDE PAVEMENT STRIPING AT ANY POINT WHERE THE MULTI-USE TRAIL CROSSES EXISTING OR PROPOSED PAVEMENT 7. ALL STREET STUB-OUTS OVER 250' IN LENGTH WILL REQUIRE A TEMPORARY TURNAROUND.	SITE IMPROVEMENTS SHALL BE IN COMPLIANCE WITH THE CITY OF O'FALLON COMPREHENSIVE PLAN.	PUBLICATION, DISTRIBU WHOLE OR IN PART, IS S
WATER IS NOT ALLOWED TO FLOW AWAY FROM THE TRENCH WITHOUT FIRST SATURATING THE TRENCH. 10.4. SURFACE BRIDGING, THE CONTRACTOR SHALL IDENTIFY THE LOCATIONS OF THE SURFACE BRIDGING (THE TENDENCY FOR THE UPPER SURFACE TO CRUST AND ARCH	8. ALL SUB GRADE IN CUT OR FILL WILL NEED TO CONFORM TO THE CITY OF O'FALLON COMPACTION REQUIREMENTS 9. MATERIAL TESTING AND FREQUENCY. MATERIALS FOR CONSTRUCTION SHALL BE TESTED AND INSPECTED PER THE APPROPRIATE ASTM CODE OR AT THE CITY ENGINEER'S DISCRETION. THE DEVELOPER'S ENGINEER SHALL PERFORM QUALITY CONTROL GUIDELINES, IN ACCORDANCE WITH ST. LOUIS COUNTY REQUIREMENTS 501.3.1.	4. OWNER TO COORDINATE WITH CITY, AMEREN AND ANY OTHER UTILITY COMPANIES FOR THE EXISTING OVERHEAD ELECTRIC RELOCATION	PROTOTYPE: P-12
OVER THE TRENCH RATHER THAN COLLAPSE AND CONSOLIDATE DURING THE JETTING PROCESS). THE CONTRACTOR SHALL BREAK DOWN THE BRIDGED AREAS USING AN APPROPRIATE METHOD SUCH AS WHEELS OR BUCKET OF A BACKHOE. WHEN SURFACE CRUST IS COLLAPSED, THE VOID SHALL BE BACK FILLED WITH THE SAME MATERIAL USED AS TRENCH BACK FILL AND RE-JETTED. COMPACTION OF THE MATERIALS WITHIN THE SUNKEN/JETTED AREA SHALL BE COMPACTED SUCH THAT NO	10. APPROVAL OF SUB GRADE AND BASE (SUB BASE). THE CITY ENGINEER OR REPRESENTATIVE SHALL APPROVE THE SUB GRADE BEFORE ANY BASE IS PLACED THEREON AND SHALL APPROVE THE BASE BEFORE CONCRETE OR SURFACE COURSE IS PLACED. THE SUB GRADE AND BASE SHALL BE SO CONSTRUCTED THAT IT WILL BE	AND/OR POTENTIAL RELOCATION UNDERGROUND ALONG MAIN STREET.	VERSION: 001
FURTHER SURFACE SUBSIDENCE OCCURS SITE GRADING.	UNIFORM IN DENSITY THROUGHOUT. 11. IN ALL FILL AREAS IN THE ROADWAYS, SOIL TESTS SHALL BE SUBMITTED AND APPROVED BY THE CITY ENGINEER FOR EACH FOOT OF FILL AND AT LEAST ONE (1) TEST AND AN AVERAGE OF ONE (1) TEST WITHIN EVERY TWO HUNDRED FIFTY (250) FEET.	5. ALL NEW SITE SERVICE UTILITIES SHALL BE UNDERGROUND.	DESIGNED BY: DRAWN BY:
11.1. WITHIN CITY RIGHT—OF—WAY. MATERIAL IS TO BE PLACED IN EIGHT (8) INCH TO TWELVE (12) INCH LOOSE LIFTS AND COMPACTED PER THE APPROVED COMPACTION REQUIREMENTS. ONE (1) COMPACTION TEST WILL BE PERFORMED EVERY TWO HUNDRED FIFTY (250) FEET ALONG THE CENTERLINE FOR EACH LIFT. 11.2. OUTSIDE OF CITY RIGHT—OF—WAY. MATERIAL IS TO BE PLACED IN EIGHT (8) INCH TO TWELVE (12) INCH LOOSE LIFTS AND COMPACTED PER THE APPROVED	12. NO TRAFFIC WILL BE ALLOWED ON NEW CONCRETE PAVEMENT UNTIL IT HÀS CURED FOR SEVEN (7) DAYS AND IT REACHES THREE THOUSAND FIVE HUNDRED (3,500) PSI WITHIN 28 DAYS.	6. APPROVALS FROM O'FALLON FIRE DISTRICT, MODNR, SANITARY AND WATER DISTRICT SHALL BE REQUIRED PRIOR TO CONSTRUCTION.	REVIEWED BY:
COMPACTION REQUIREMENTS. ONE (1) COMPACTION TEST WILL BE PERFORMED AT TWO (2) FOOT VERTICAL INTERVALS AND APPROXIMATELY EVERY ONE THOUSAND (1,000) CUBIC YARDS.	12.1. CONCRETE PAVEMENTS SHALL NOT BE APPROVED UNLESS IT REACHES A STRENGTH OF FOUR THOUSAND (4,000) PSI. CYLINDERS/COMPRESSIVE STRENGTH. ONE (1) SET OF FOUR (5) CYLINDERS WITHIN THE FIRST FIFTY (50) CUBIC YARDS AND ONE (1) SET PER ONE HUNDRED (100) CUBIC YARDS THEREAFTER. ONE (1)	WATER DISTRICT SHALL BE REQUIRED FRIOR TO CONSTRUCTION.	
2. ACCESS TO THE SITE FROM ANY OTHER LOCATION OTHER THAN THE PROPOSED CONSTRUCTION ENTRANCE IS STRICTLY PROHIBITED!	CYLINDER MUST BE TESTED AT SEVEN (7) DAYS, THREE (3) AT TWENTY-EIGHT (28) DAYS, AND ONE (1) HELD IN RESERVE. 13. PRIOR TO PLACEMENT OF AGGREGATE BASE MATERIAL ON SUB GRADE AND PRIOR TO PLACEMENT OF PAVEMENT ON BASE MATERIAL, THE SUB GRADE AND BASE MUST BE PROOF-ROLLED WITH A FULLY LOADED (TEN (10) TON LOAD) TANDEM TRUCK OR EQUIVALENT TIRE VEHICLE WITH ONE (1) PASS DOWN EACH DRIVING LANE NO FASTER THAN THREE (3) MILES PER HOUR. IF SOFT SPOTS ARE DETECTED, OR PUMPING. RUTTING OR HEAVING OCCURS GREATER THAN ONE (1) INCH AT THE SUB		
. ALL WATER MAIN INSTALLATIONS SHALL FOLLOW THE CURRENT CITY OF O'FALLON WATER MAIN SPECIFICATIONS. . FIRE HYDRANTS SHALL BE A MAXIMUM OF 600' APART. LOCAL FIRE DISTRICT APPROVAL IS REQUIRED.	GRADE, THE ROADBED SHALL BE CONSIDERED UNSATISFACTORY AND THE SOIL IN THESE AREAS SHALL BE REMEDIATED TO THE DEPTH INDICATED BY THE CONTRACTOR'S TESTING FIRM AND APPROVED BY A REPRESENTATIVE OF THE CITY ENGINEER.		
. COORDINATE WITH THE WATER COMPANY ON THE LOCATION OF WATER METERS. FOR METERS IN THE CITY'S DISTRICT, METERS SHALL BE IN THE RIGHT-OF-WAY, OTHERWISE AN ACCESS EASEMENT FROM THE RIGHT-OF-WAY SHALL BE PROVIDED.	14. SUB GRADE AND BASE BENEATH PAVEMENTS SHALL BE COMPACTED TO ST. LOUIS COUNTY HIGHWAY DEPARTMENT SPECIFICATIONS. THE MOISTURE RANGE SHALL BE DETERMINED BY THE STANDARD OR MODIFIED PROCTOR DENSITY METHOD AASHTO T-99 AND WITHIN -2/+4 PERCENTAGE POINTS OF THE OPTIMUM MOISTURE CONTENT. 15. THE ENTIRE WIDTH AND LENGTH WILL CONFORM TO LINE, GRADE AND CROSS SECTION SHOWN ON THE PLANS OR AS ESTABLISHED BY THE ENGINEER. IF ANY SETTLING		
F. ALL WATER MAIN MUST HAVE A MINIMUM OF 42" OF COVER. (CITY WATER MAINS) 5. PROVIDE WATER VALVES TO ISOLATE THE SYSTEM. 5. ALL WATER MAINS SHALL BE C-200 DVC DRESSURE RIPE OR FOLIAL WITH LOCATOR/TRACER WIRES	OR WASHING OCCURS, OR WHERE HAULING RESULTS IN RUTS OR OTHER OBJECTIONABLE IRREGULARITIES, THE CONTRACTOR SHALL IMPROVE THE SUB GRADE OR BASE TO THE SATISFACTION OF THE CITY BEFORE THE PAVEMENT IS PLACED. ADDITIONAL ROLLING OR METHODS TO VERIFY COMPACTION SHALL BE AT THE DISCRETION OF THE		
. ALL WATER MAINS SHALL BE C-900 PVC PRESSURE PIPE OR EQUAL WITH LOCATOR/TRACER WIRES . IF THE EXCAVATIONS ARE MADE IN THE IMPROVED PORTION OF THE RIGHT-OF-WAY, TWELVE INCHES OF GRANULAR BACKFILL WILL BE PLACED OVER EXPOSED FACILITIES AND CONTROLLED LOW STRENGTH MATERIAL (CLSM) AKA FLOWABLE FILL WILL FILL THE HOLE WITH EIGHT INCHES OF THE FINISHED SURFACE FOR CONCRETE	CITY ENGINEER. TOLERANCE ALLOWED ON ALL LINES, GRADES AND CROSS SECTIONS SHALL BE PLUS OR MINUS FOUR—HUNDREDTHS (+0.04) FEET. 16. UTILITY WORK PRIOR TO BASE CONSTRUCTION. NO BASE COURSE WORK MAY PROCEED ON ANY STREET UNTIL ALL UTILITY EXCAVATIONS (STORM AND SANITARY SEWERS, WATER, GAS, ELECTRIC, ETC.) HAVE BEEN PROPERLY BACK FILLED WITH GRANULAR MATERIAL, CRUSHED STONE OR GRAVEL MECHANICALLY TAMPED IN TEN (10) INCH		MOTTION
PAVEMENT. THERE WILL BE A PLASTIC MEMBRANE PLACED BETWEEN THE ROCK BASE AND THE CLSM TO PREVENT THE MATERIAL FROM BLEEDING INTO THE ROCK BASE. THE REMAINING EIGHT INCHES WILL BE RESTORED BY PLACING A 28 DAY, 4,000 PSD CONCRETE MIX.	LIFTS. UTILITIES INSTALLED AFTER SUB GRADE PREPARATION SHALL BE BORED. COMPACTION REQUIREMENTS SHALL FOLLOW ST. LOUIS COUNTY STANDARDS. 17. EQUIPMENT CALIBRATION. THE DEVELOPER'S CONTRACTORS AND SUBCONTRACTORS MUST HAVE THEIR EQUIPMENT CALIBRATED BY THE FOLLOWING MINIMUM STANDARDS.		ESCRI
B. DISINFECTING: DISINFECTING SHALL BE ACCOMPLISHED BY PLACING SUFFICIENT HYPO CHLORITE GRANULE (HTH) IN EACH SECTION OF PIPE TO ACHIEVE A CHLORINE RESIDUAL IN THE PIPELINE, UPON INITIAL FILLING, OF 50 MG/L (PPM). HT. TABLETS WILL NOT BE ALLOWED. FOLLOWING COMPLETION OF THE PIPELINE, IT SHALL BE SLOWLY FILLED WITH WATER AND A SAMPLE WILL BE TAKEN IMMEDIATELY AND THE CHLORINE RESIDUAL MUST BE 50 MG/L OR GREATER. THE SOLUTION SHALL BE	17.1. AIR METER——WEEKLY. 17.2. CYLINDER COMPRESSION——ANNUALLY BY INDEPENDENT CALIBRATION SERVICE. 17.3. BATCH SCALES——MONTHLY.		
ALLOWED TO STAND FOR 24 HOURS AND A SAMPLE SHALL THEN BE TAKEN. THE CHLORINE RESIDUAL AFTER 24 HOURS SHALL BE 30 MG/L OR GREATER. IF THE PIPING SHOWS INSUFFICIENT CHLORINE RESIDUALS IN EITHER TEST, THE PIPING SHALL BE RE—CHLORINATED BY THE INJECTION OF HYPO CHLORITE SOLUTION UNTIL	17.4. NUCLEAR TESTING DEVICES——EVERY SIX (6) MONTHS. 17.5. PROCTOR EQUIPMENT——EVERY SIX (6) MONTHS.		DAT
SATISFACTORY RESULTS ARE ACHIEVED. ALL DISINFECTING SHALL BE DONE BY THE CONTRACTOR. ONLY THE TESTING TO DETERMINE THE CHLORINE RESIDUAL WILL BE DONE BY THE CITY.	17.6. SLUMP CONE——MONTHLY. 18. ALL PERMANENT TRAFFIC CONTROL WILL BE PER M.U.T.C.D. OR MODOT STANDARDS. S1—1 FROM THE M.U.T.C.D. MANUAL WILL BE USED AT ALL CROSSWALK LOCATIONS ACCOMPANIED WITH FTHER W16—9P OR W16—7P SIGNS.		REV
	19. ALL TRAFFIC SIGNALS, STREET SIGNS, SIGN POST, BACKS AND BRACKET ARMS SHALL BE PAINTED BLACK USING CARBOLINE RUST BOND PENETRATING SEALER SG AND CARBOLINE 133 HB PAINT (OR EQUIVALENT AS APPROVED BY CITY OF O'FALLON AND MODOT)		SHEET 1
. PRESSURE TESTING: IMMEDIATELY FOLLOWING DISINFECTION, THE PIPING SHALL BE PUMPED TO A PRESSURE (AT THE HIGHEST POINT IN THE PROJECT) OF 150 PSI OR HIGHER WHERE THE WORKING PRESSURE IS HIGHER THAN 150 PSI AS DETERMINED BY THE CITY. IN SUCH CASES, THE PRESSURE SHALL BE AS SPECIFIED BY THE CITY AND TWO PRESSURE TESTS SHALL BE CONDUCTED. THE FIRST TEST SHALL BE WITH THE FIRE HYDRANT AUXILIARY VALVE OPEN AND BE TO 50 PSI. THE SECOND TEST	20. IF THE EXCAVATIONS ARE MADE IN THE IMPROVED PORTION OF THE RIGHT-OF-WAY. TWELVE INCHES OF GRANULAR BACKFILL WILL BE PLACED OVER EXPOSED		SITE DAT
. PRESSURE TESTING: IMMEDIATELY FOLLOWING DISINFECTION, THE PIPING SHALL BE PUMPED TO A PRESSURE (AT THE HIGHEST POINT IN THE PROJECT) OF 150 PSI OR HIGHER WHERE THE WORKING PRESSURE IS HIGHER THAN 150 PSI AS DETERMINED BY THE CITY. IN SUCH CASES, THE PRESSURE SHALL BE AS SPECIFIED BY THE CITY	FACILITIES AND CONTROLLED LOW STRENGTH MATERIAL (CLSM) AKA FLOWABLE FILL WILL FILL THE HOLE WITH EIGHT INCHES OF THE FINISHED SURFACE FOR CONCRETE PAYEMENT THERE WILL BE A PLASTIC MEMBRANE PLACED BETWEEN THE ROCK BASE AND THE CLSM TO PREVENT THE MATERIAL FROM BLEEDING INTO THE ROCK BASE		JURISDICTIO
PRESSURE TESTING: IMMEDIATELY FOLLOWING DISINFECTION, THE PIPING SHALL BE PUMPED TO A PRESSURE (AT THE HIGHEST POINT IN THE PROJECT) OF 150 PSI OR HIGHER WHERE THE WORKING PRESSURE IS HIGHER THAN 150 PSI AS DETERMINED BY THE CITY. IN SUCH CASES, THE PRESSURE SHALL BE AS SPECIFIED BY THE CITY AND TWO PRESSURE TESTS SHALL BE CONDUCTED. THE FIRST TEST SHALL BE WITH THE FIRE HYDRANT AUXILIARY VALVE OPEN AND BE TO 50 PSI. THE SECOND TEST SHALL BE WITH THE FIRE HYDRANT AUXILIARY VALVE CLOSED AND BE TO THE HIGHER PRESSURE AS DIRECTED BY THE CITY. ALL PUMPING EQUIPMENT AND PRESSURE GAUGES SHALL BE PROVIDED BY THE CONTRACTOR. AFTER ACHIEVING THE TEST PRESSURE, THE PIPING SHALL BE LEFT CLOSED FOR A PERIOD OF TWO (2) HOURS. AT THE END OF THIS TIME THE PRESSURE DROP SHALL NOT EXCEED 2 PSI. IN ADDITION, IF THE PRESSURE APPEARS, IN JUDGMENT OF THE CITY'S REPRESENTATIVE, TO BE CONTINUING TO DROP, THE TEST SHALL BE CONSIDERED A FAILURE. IF THE PRESSURE TEST FAILS, THE CONTRACTOR WILL BE REQUIRED TO FIND AND CORRECT THE SOURCE OF THE LEAKAGE. IF THIS REQUIRES DRAINING OF	FACILITIES AND CONTROLLED LOW STRENGTH MATERIAL (CLSM) AKA FLOWABLE FILL WILL FILL THE HOLE WITH EIGHT INCHES OF THE FINISHED SURFACE FOR CONCRETE PAVEMENT. THERE WILL BE A PLASTIC MEMBRANE PLACED BETWEEN THE ROCK BASE AND THE CLSM TO PREVENT THE MATERIAL FROM BLEEDING INTO THE ROCK BASE. THE REMAINING EIGHT INCHES WILL BE RESTORED BY PLACING A 28 DAY, 4,000 PSI CONCRETE MIX.		
PRESSURE TESTING: IMMEDIATELY FOLLOWING DISINFECTION, THE PIPING SHALL BE PUMPED TO A PRESSURE (AT THE HIGHEST POINT IN THE PROJECT) OF 150 PSI OR HIGHER WHERE THE WORKING PRESSURE IS HIGHER THAN 150 PSI AS DETERMINED BY THE CITY. IN SUCH CASES, THE PRESSURE SHALL BE AS SPECIFIED BY THE CITY AND TWO PRESSURE TESTS SHALL BE CONDUCTED. THE FIRST TEST SHALL BE WITH THE FIRE HYDRANT AUXILIARY VALVE OPEN AND BE TO 50 PSI. THE SECOND TEST SHALL BE WITH THE FIRE HYDRANT AUXILIARY VALVE CLOSED AND BE TO THE HIGHER PRESSURE AS DIRECTED BY THE CITY. ALL PUMPING EQUIPMENT AND PRESSURE GAUGES SHALL BE PROVIDED BY THE CONTRACTOR. AFTER ACHIEVING THE TEST PRESSURE, THE PIPING SHALL BE LEFT CLOSED FOR A PERIOD OF TWO (2) HOURS. AT THE END OF THIS TIME THE PRESSURE DROP SHALL NOT EXCEED 2 PSI. IN ADDITION, IF THE PRESSURE APPEARS, IN JUDGMENT OF THE CITY'S REPRESENTATIVE, TO BE CONTINUING TO DROP, THE TEST SHALL BE CONSIDERED A FAILURE. IF THE PRESSURE TEST FAILS, THE CONTRACTOR WILL BE REQUIRED TO FIND AND CORRECT THE SOURCE OF THE LEAKAGE. IF THIS REQUIRES DRAINING OF THE PIPELINE, WHEN THE LEAKAGE IS CORRECTED, THE PIPELINE MUST BE RE—DISINFECTED AND THE PRESSURE TESTED AGAIN UNTIL SATISFACTORY RESULT ARE ACHIEVED. ANY MDNR REQUIRED DECHLORINATION WILL BE PERFORMED BY THE CONTRACTOR.	PAVEMENT. THERE WILL BE A PLASTIC MEMBRANE PLACÈD BETWEEN THE ROCK BASE AND THE CLSM TO PREVENT THE MATERIAL FROM BLEEDING INTO THE ROCK BASE. THE REMAINING EIGHT INCHES WILL BE RESTORED BY PLACING A 28 DAY, 4,000 PSI CONCRETE MIX. CITY OF O'FALLON RETAINING WALL (TERRACED AND VERTICAL) NOTES:		SHEET NU
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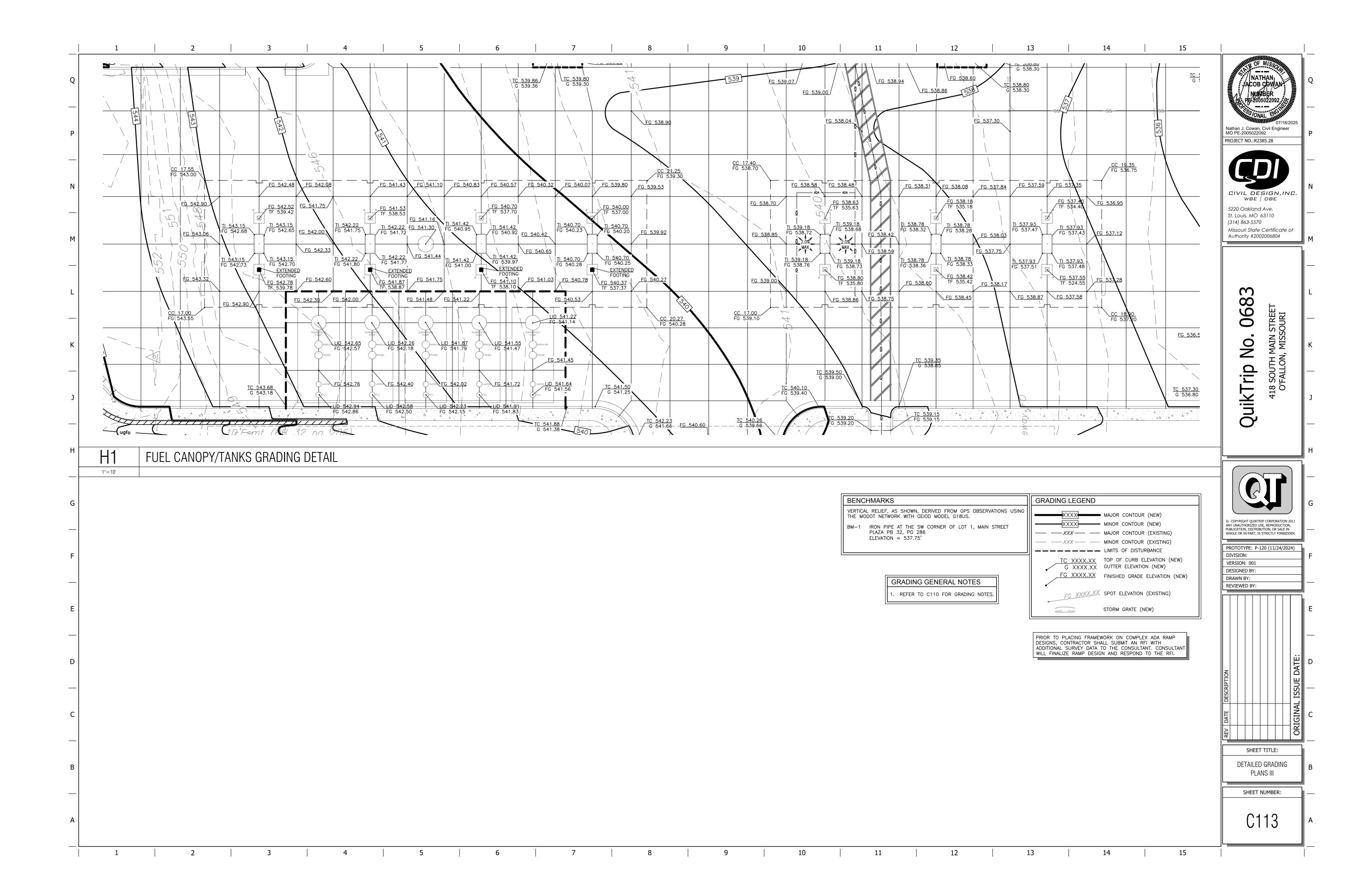


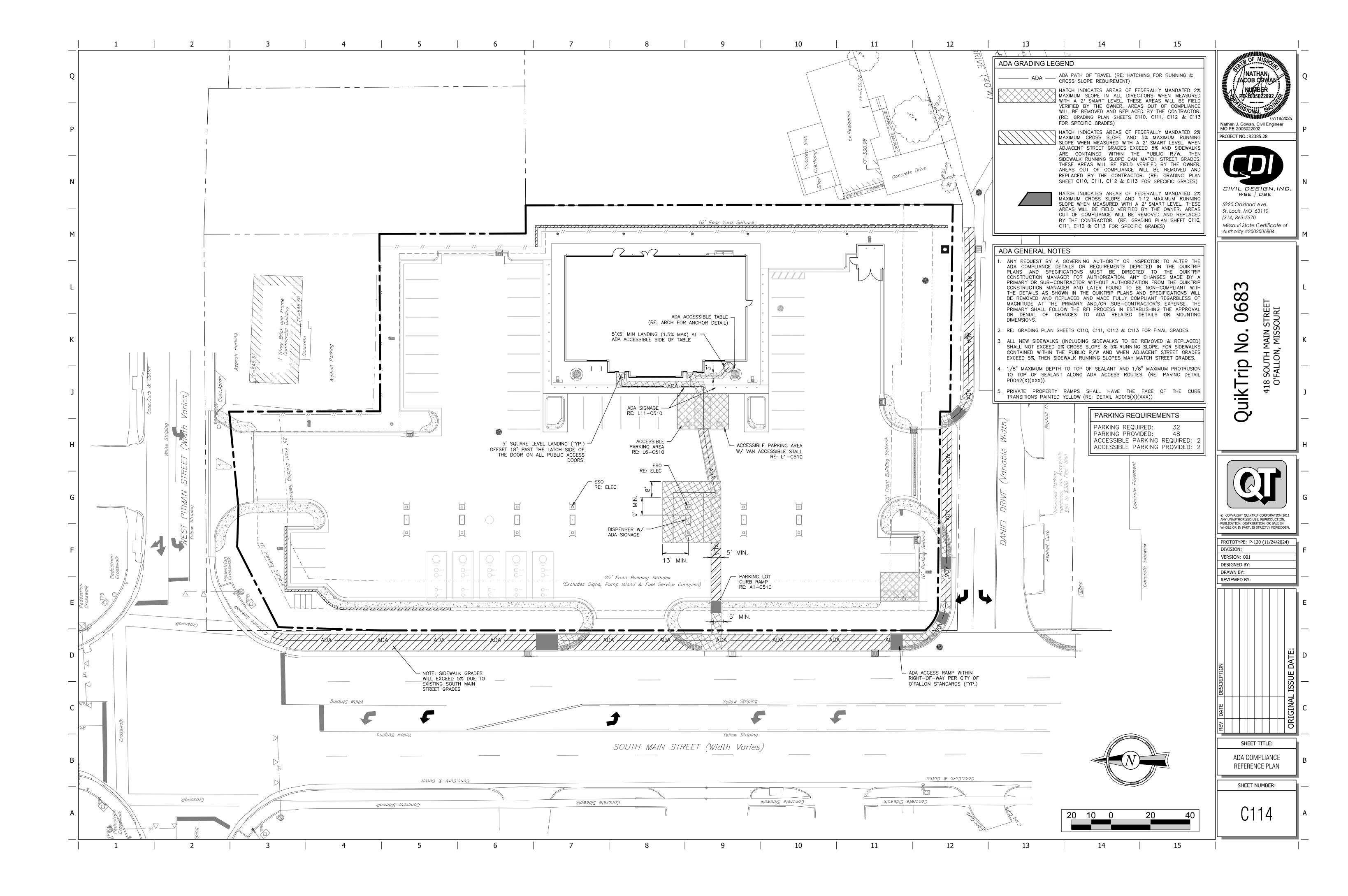


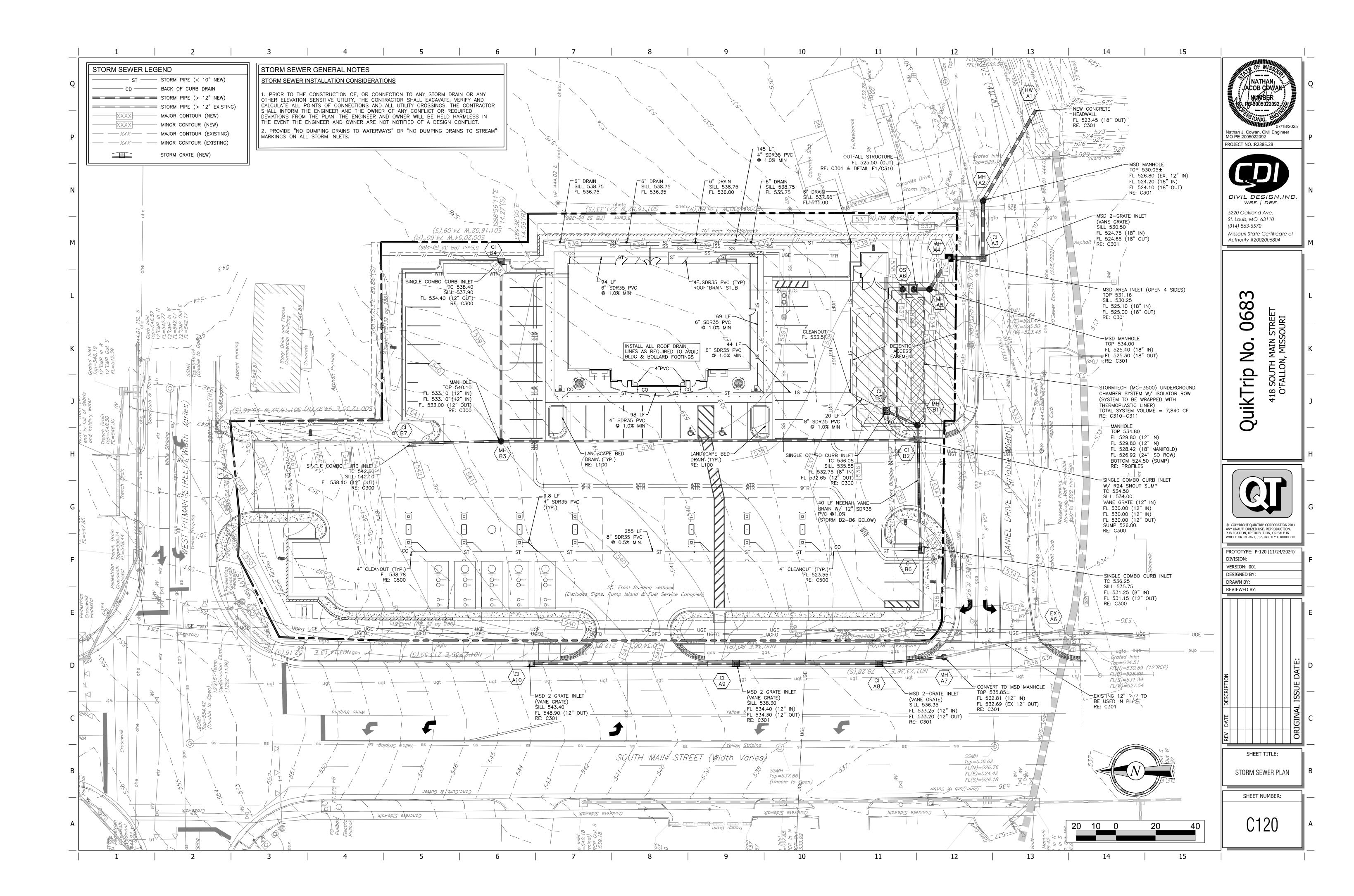


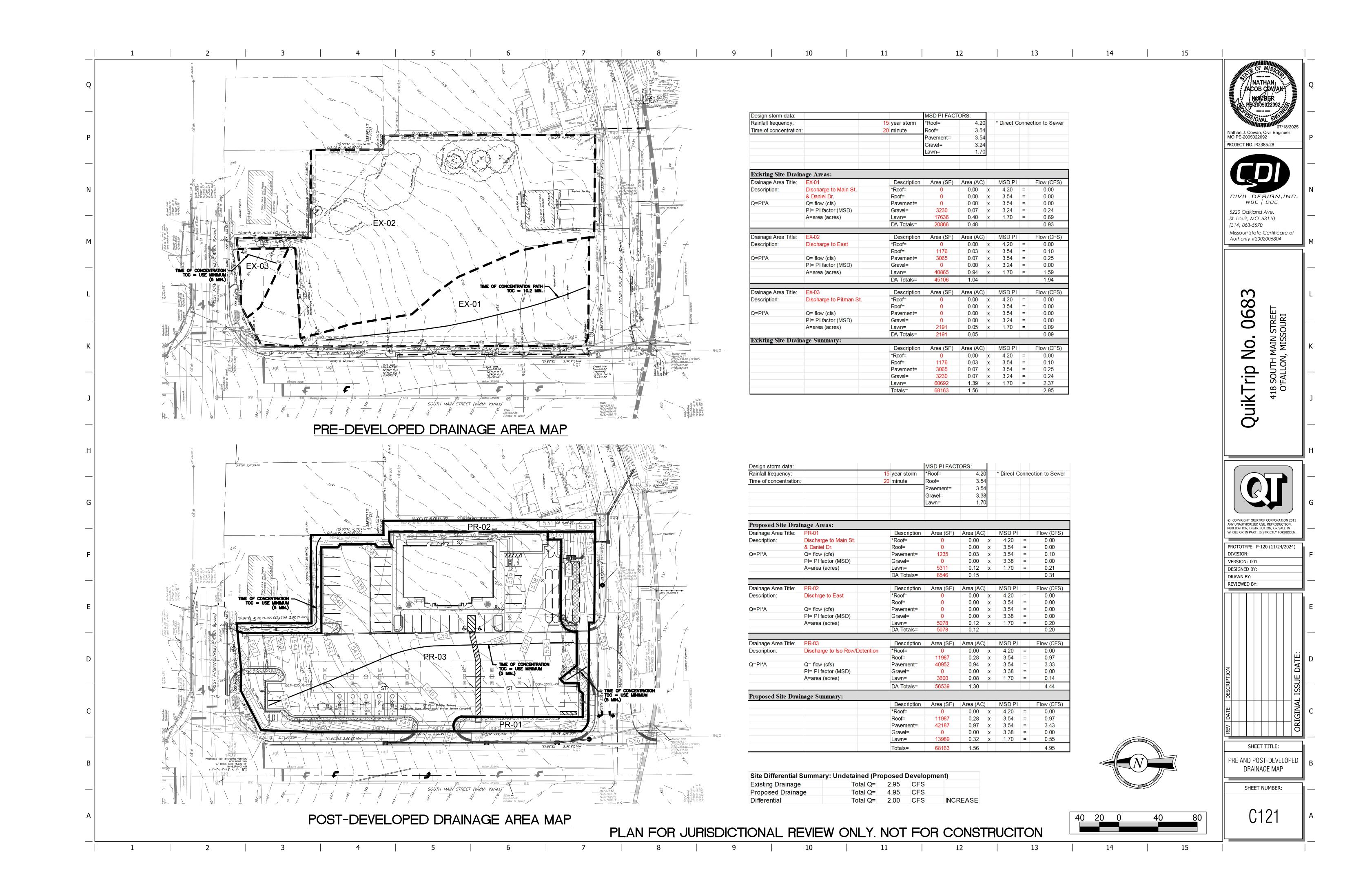


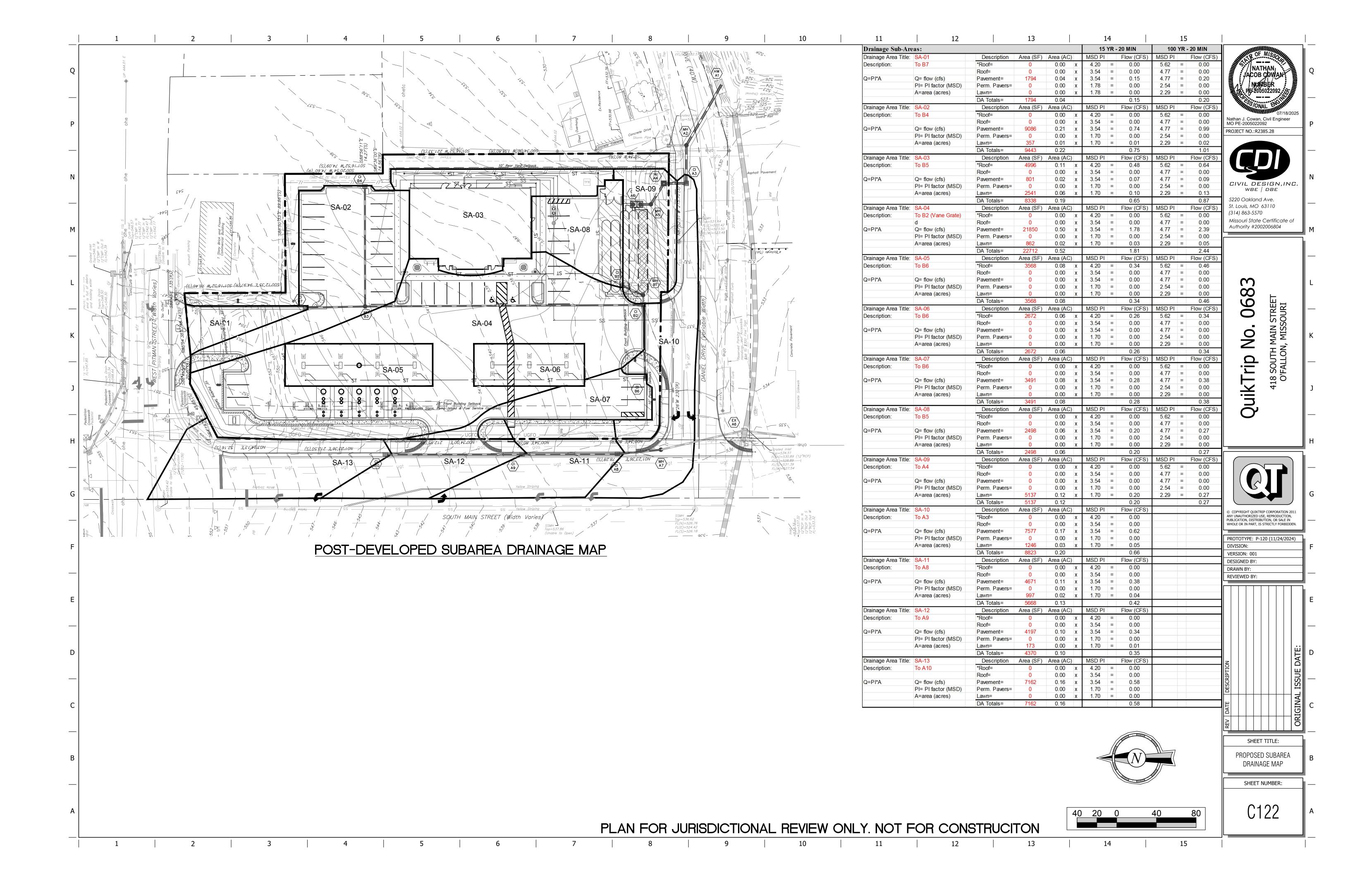


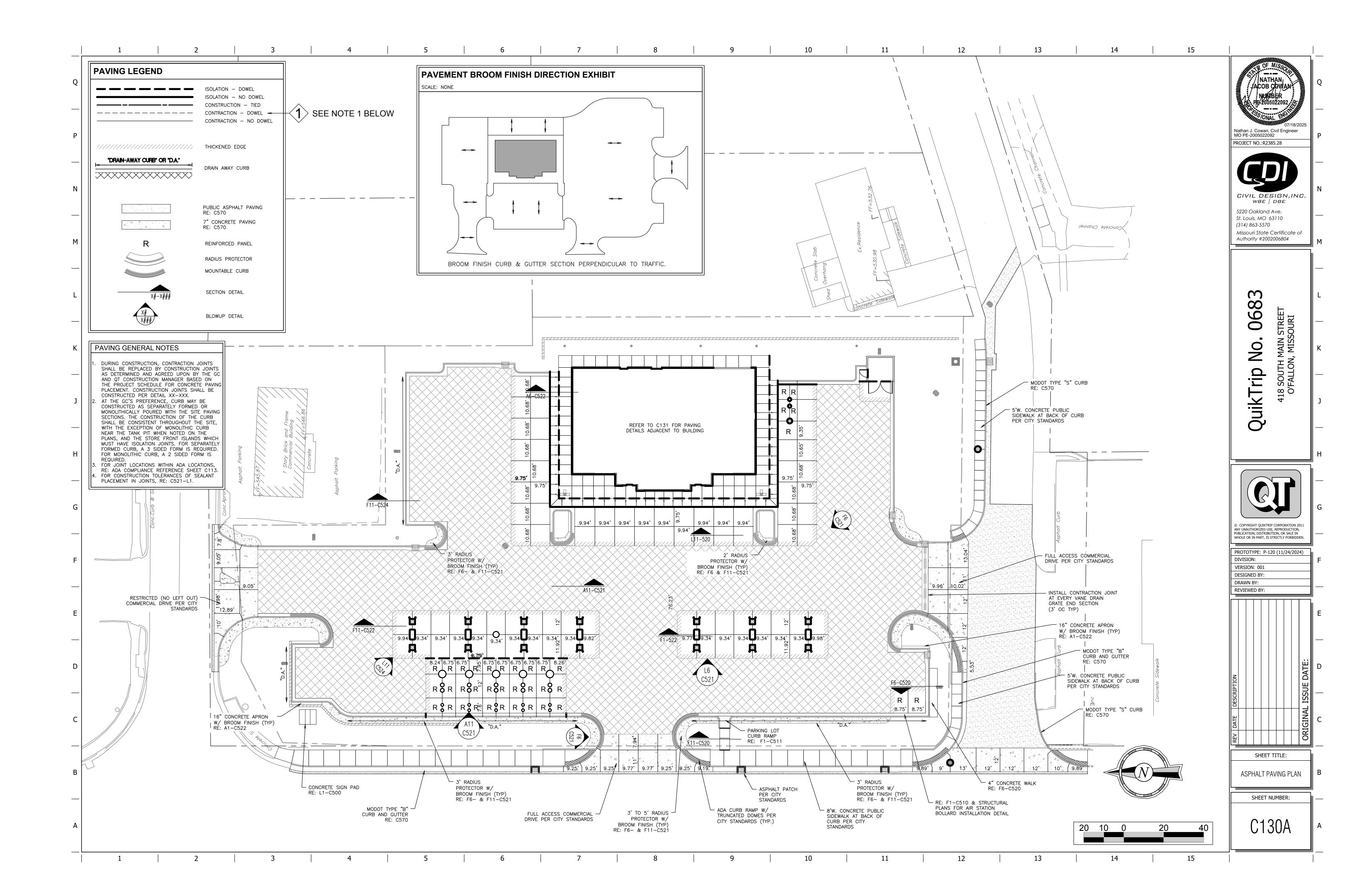


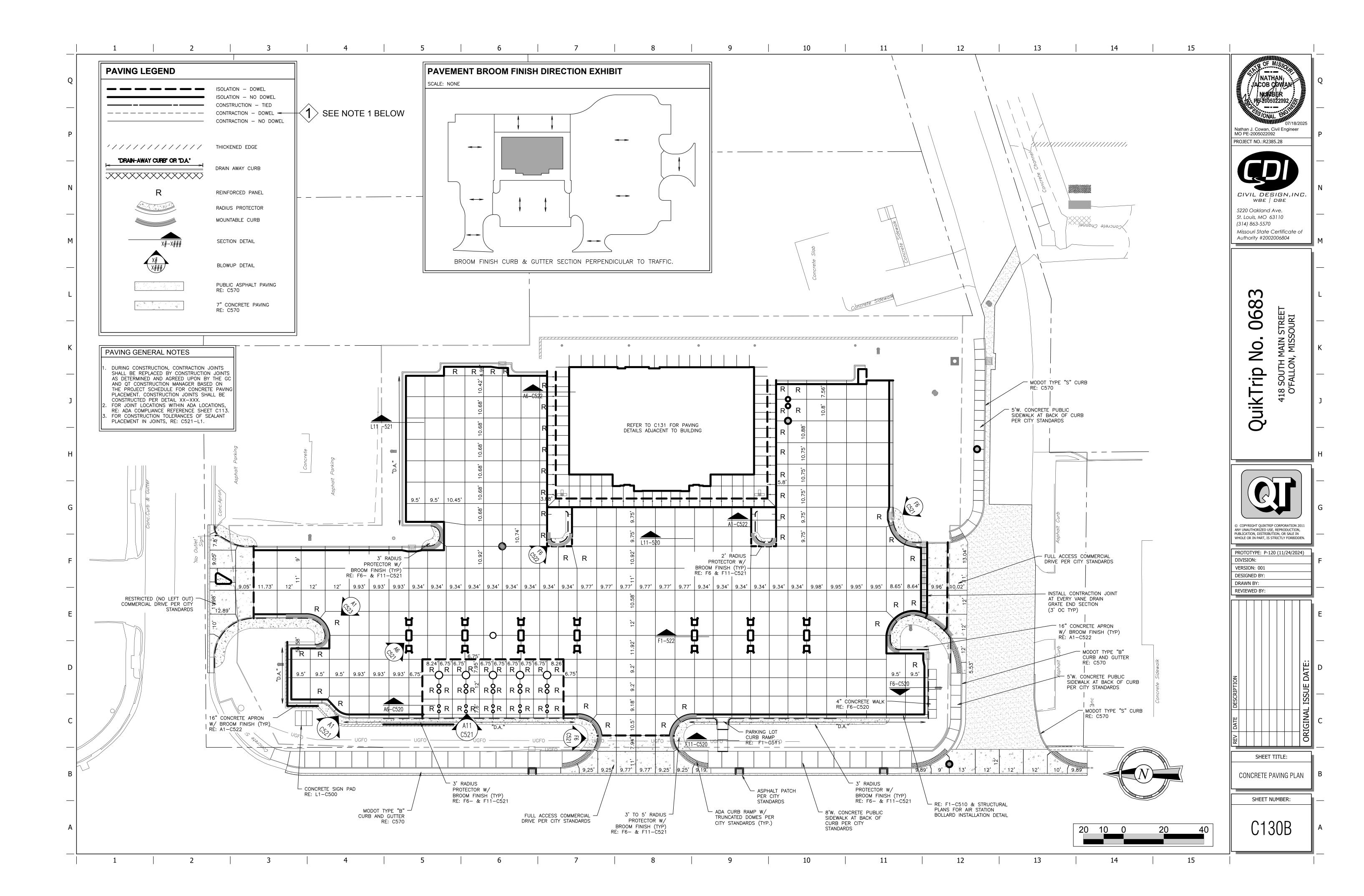


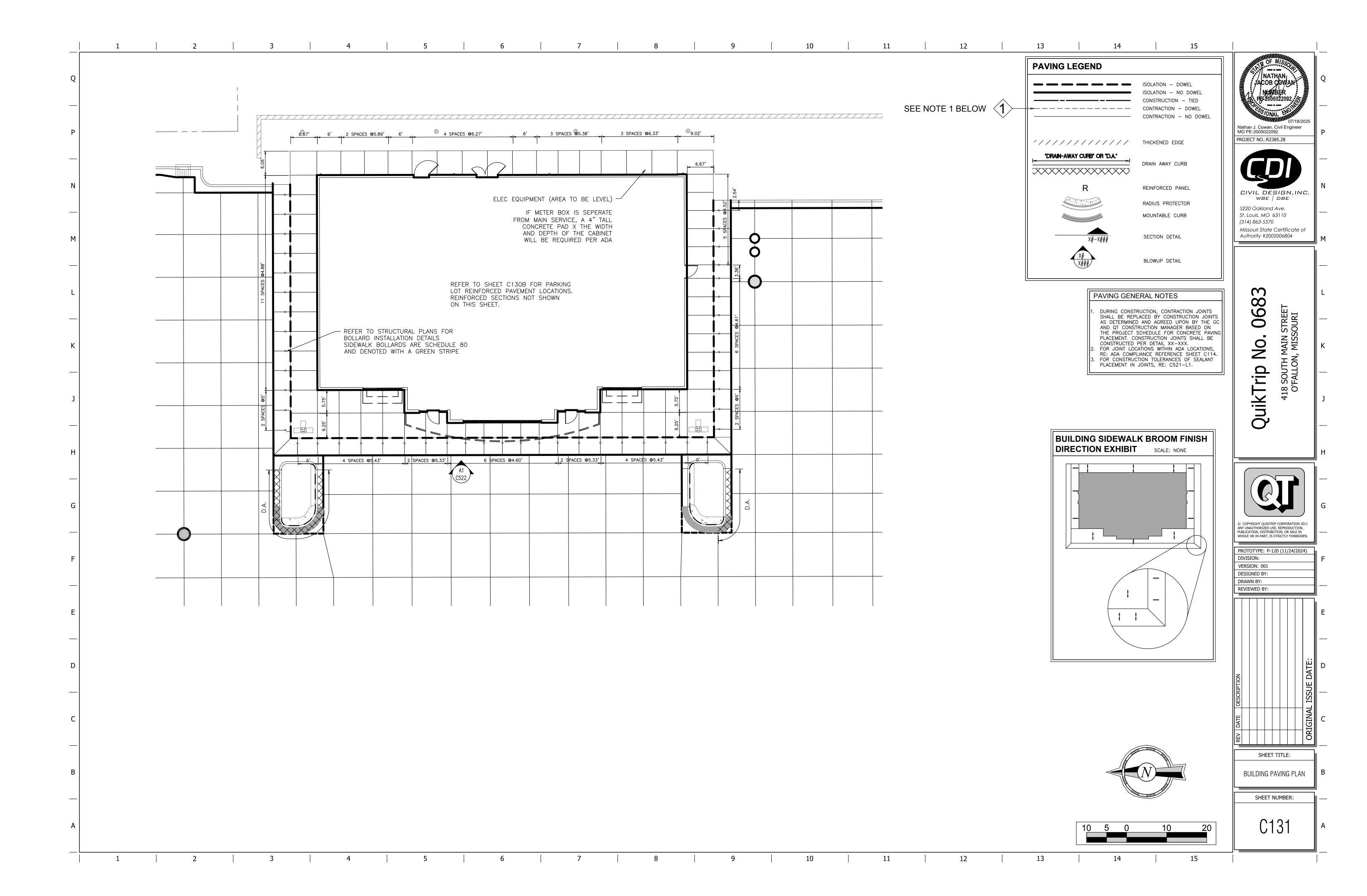


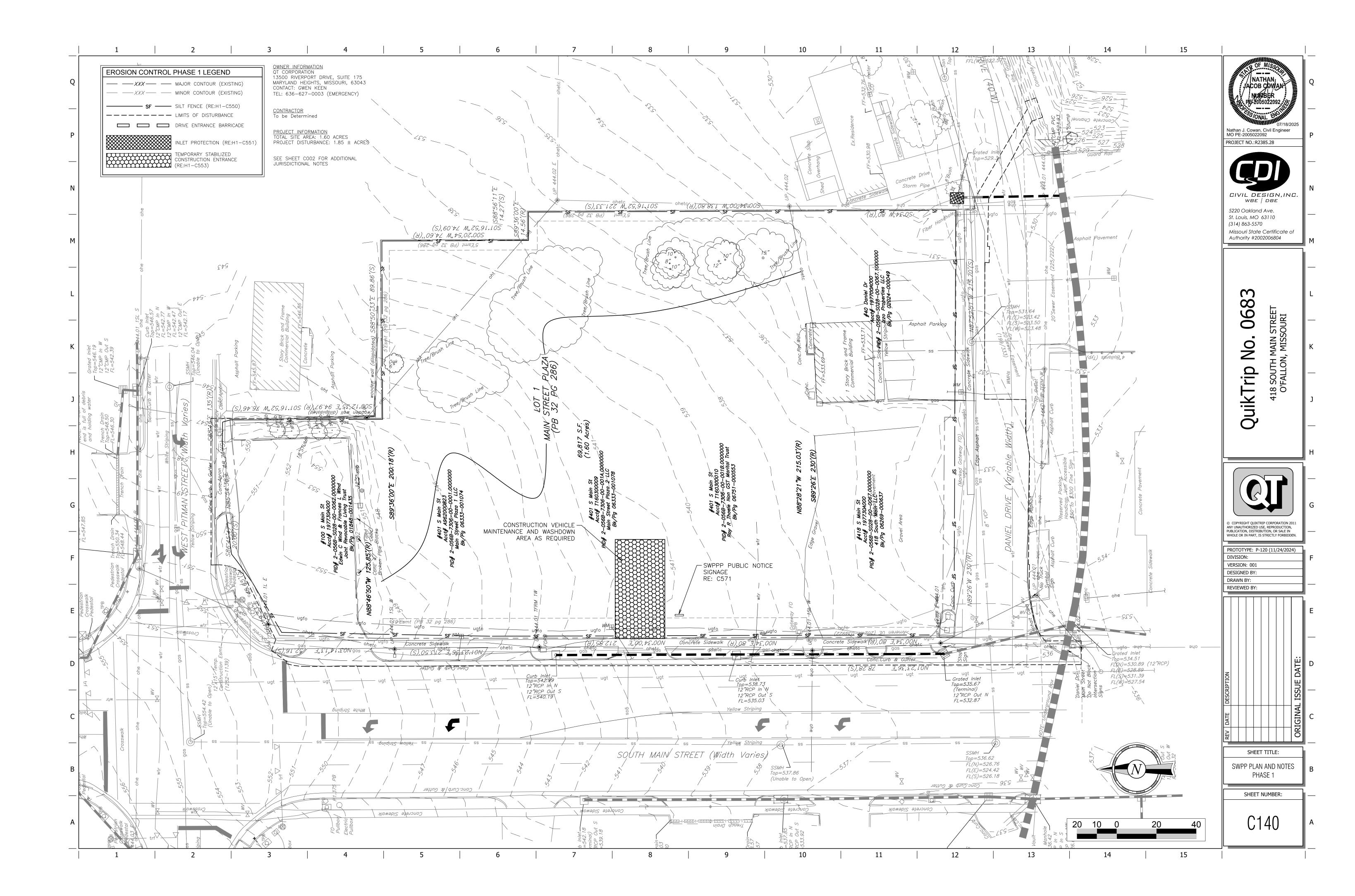


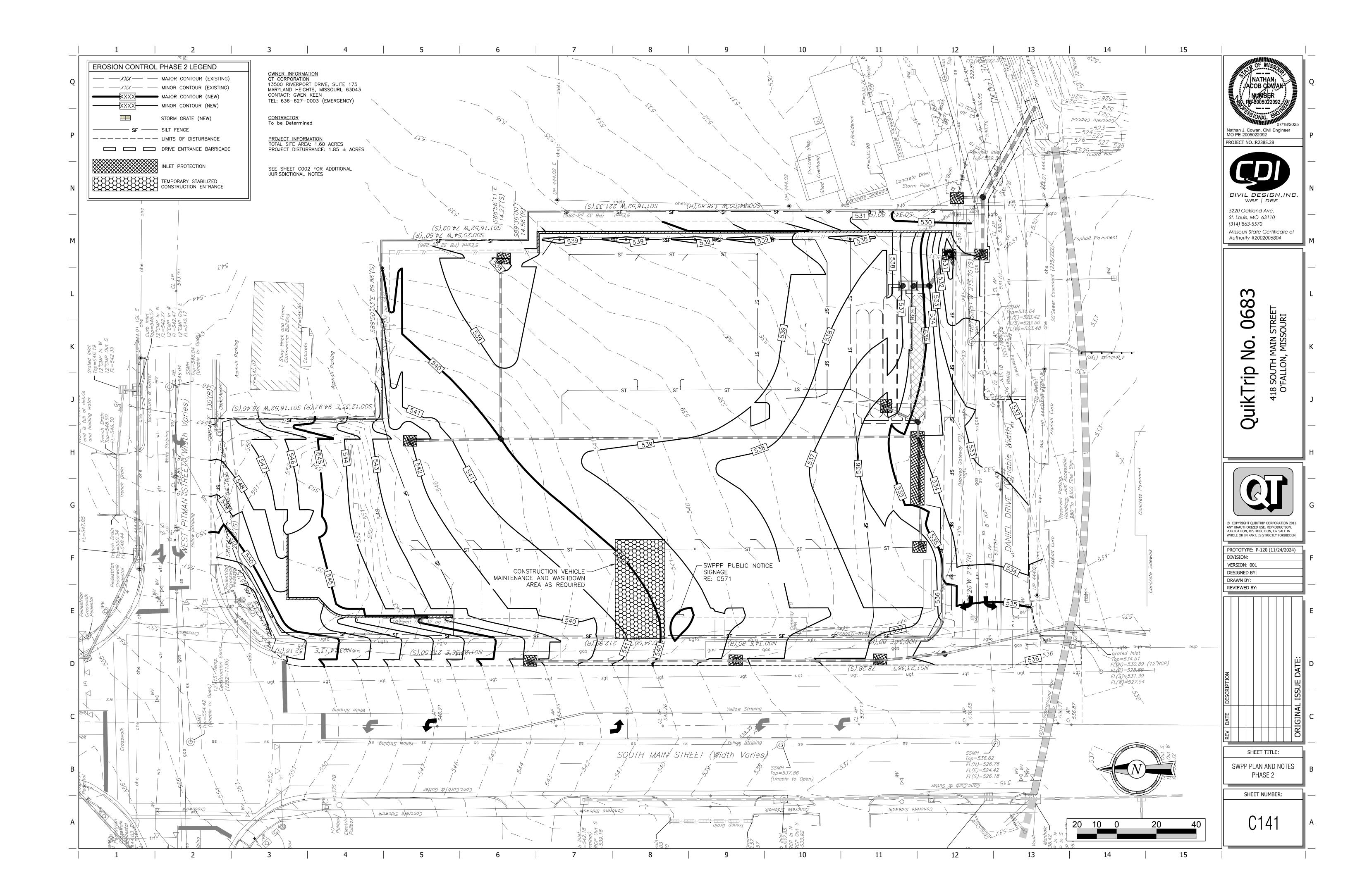


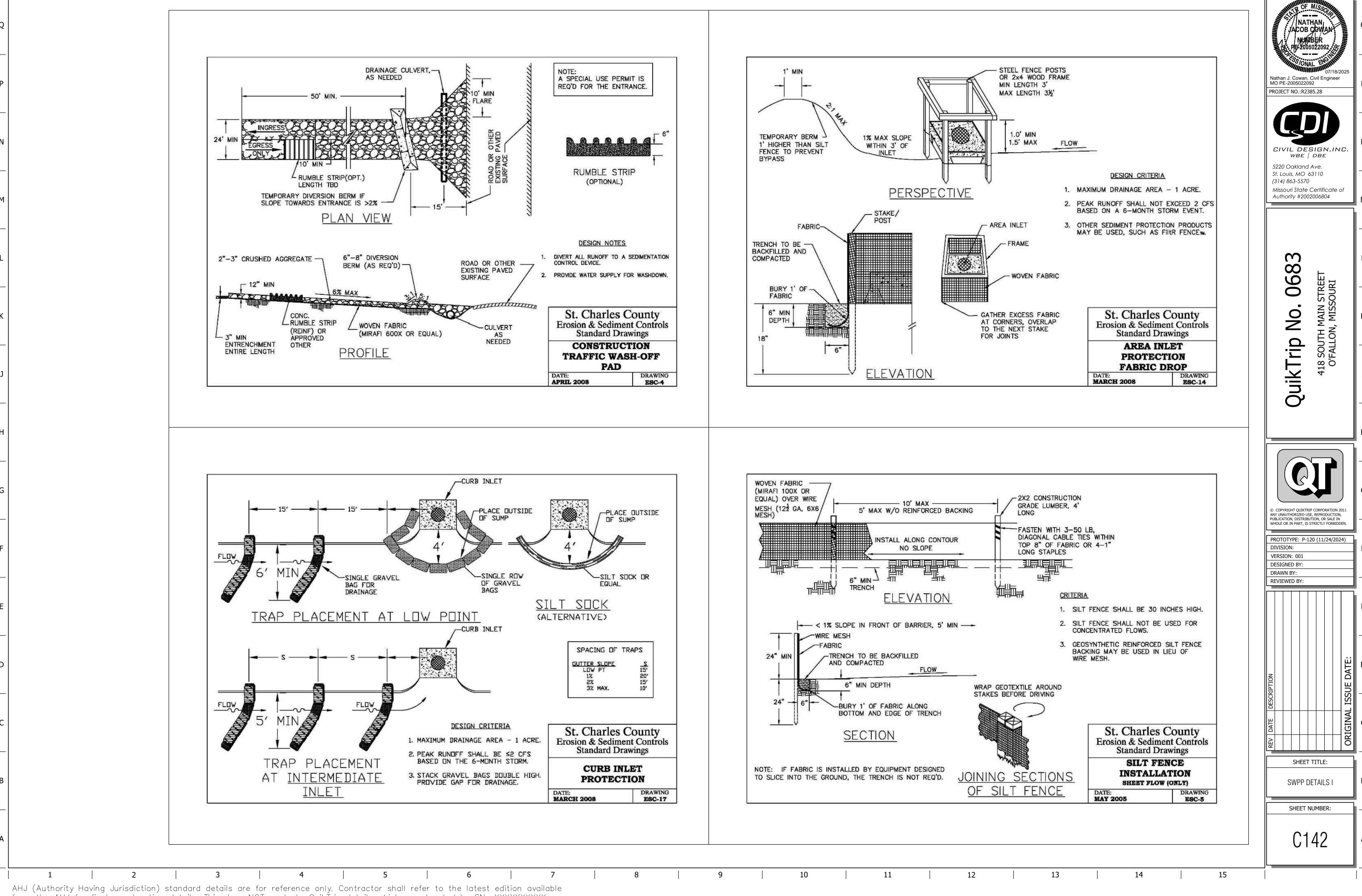


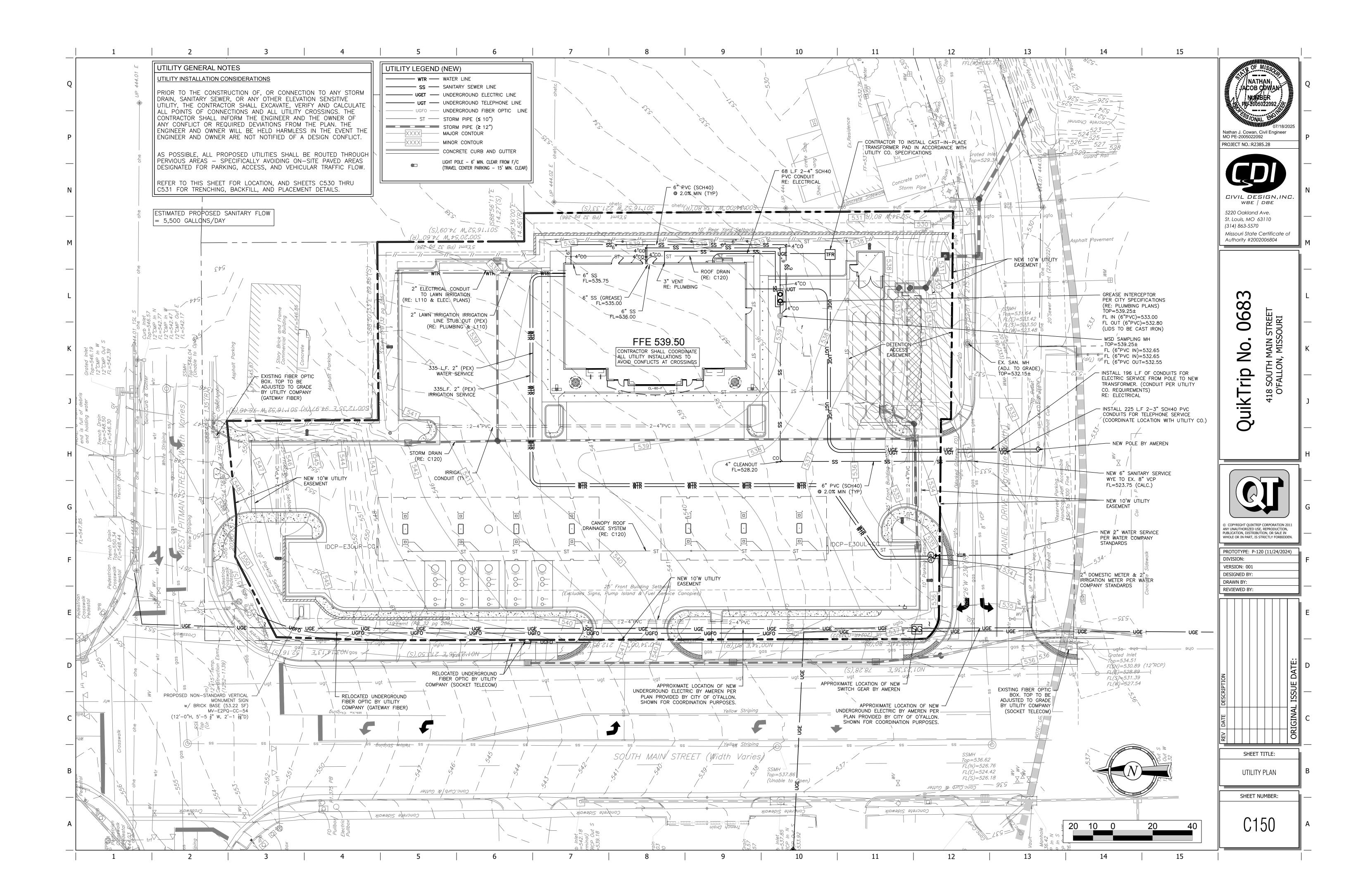


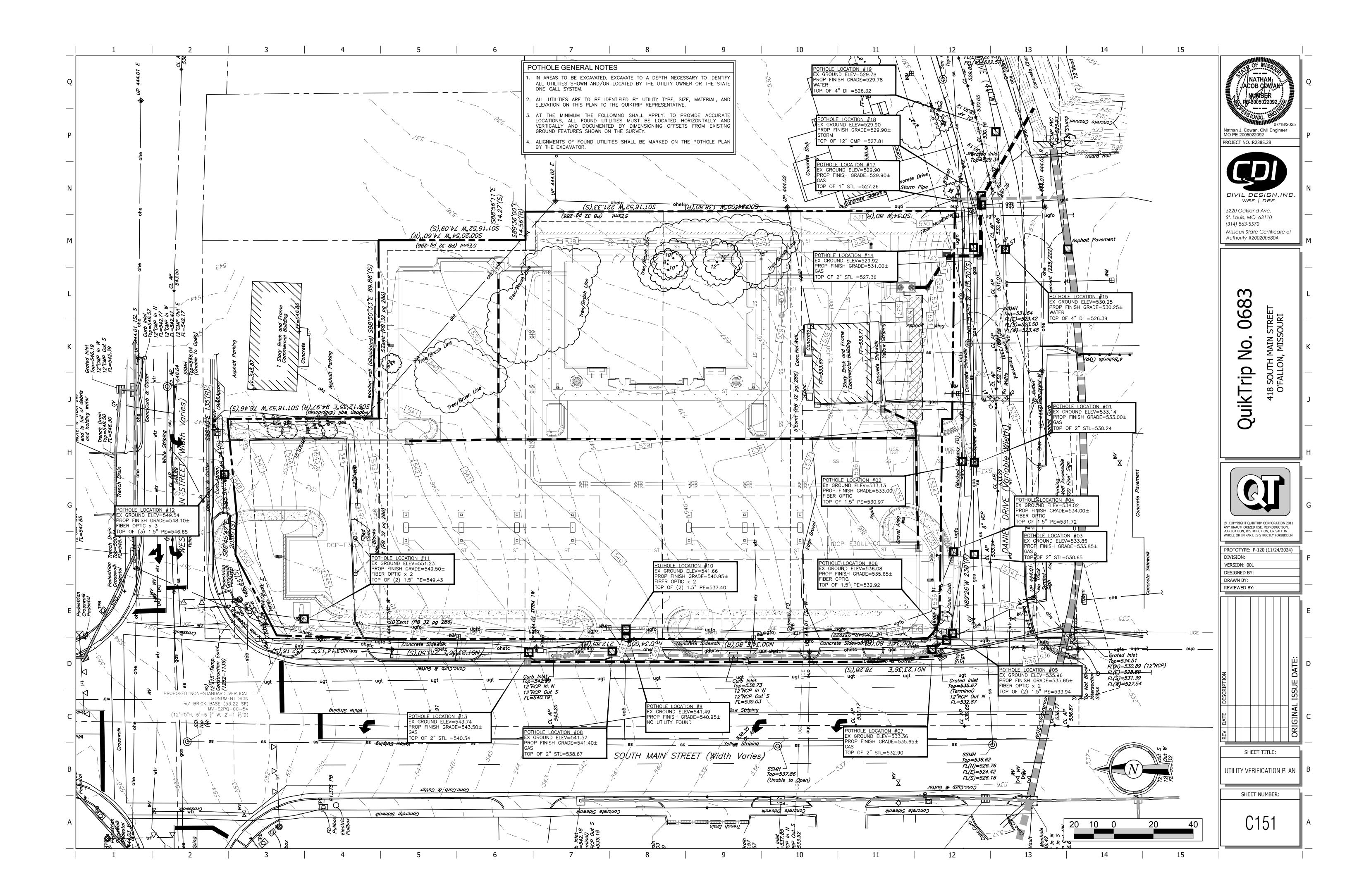


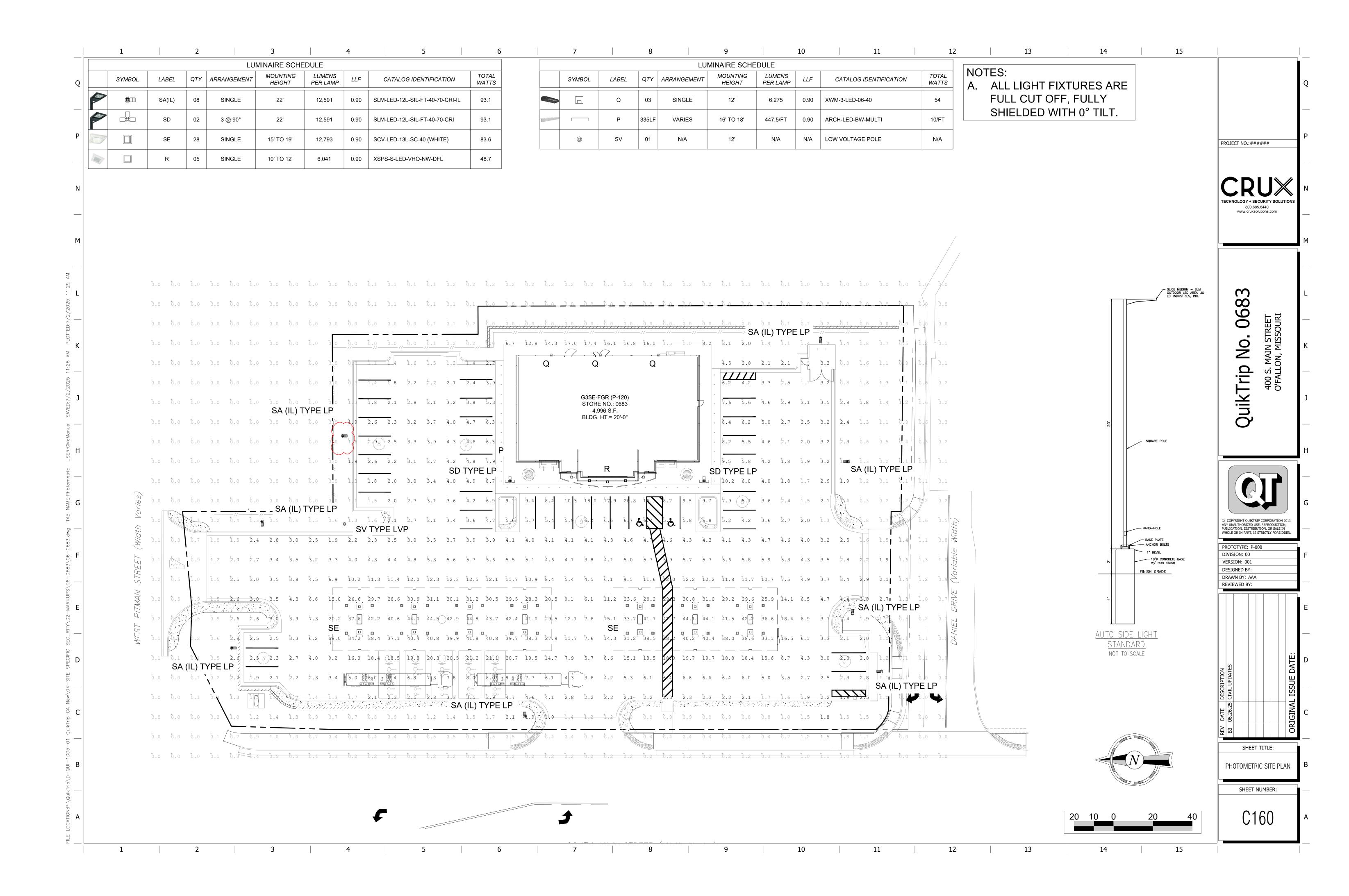


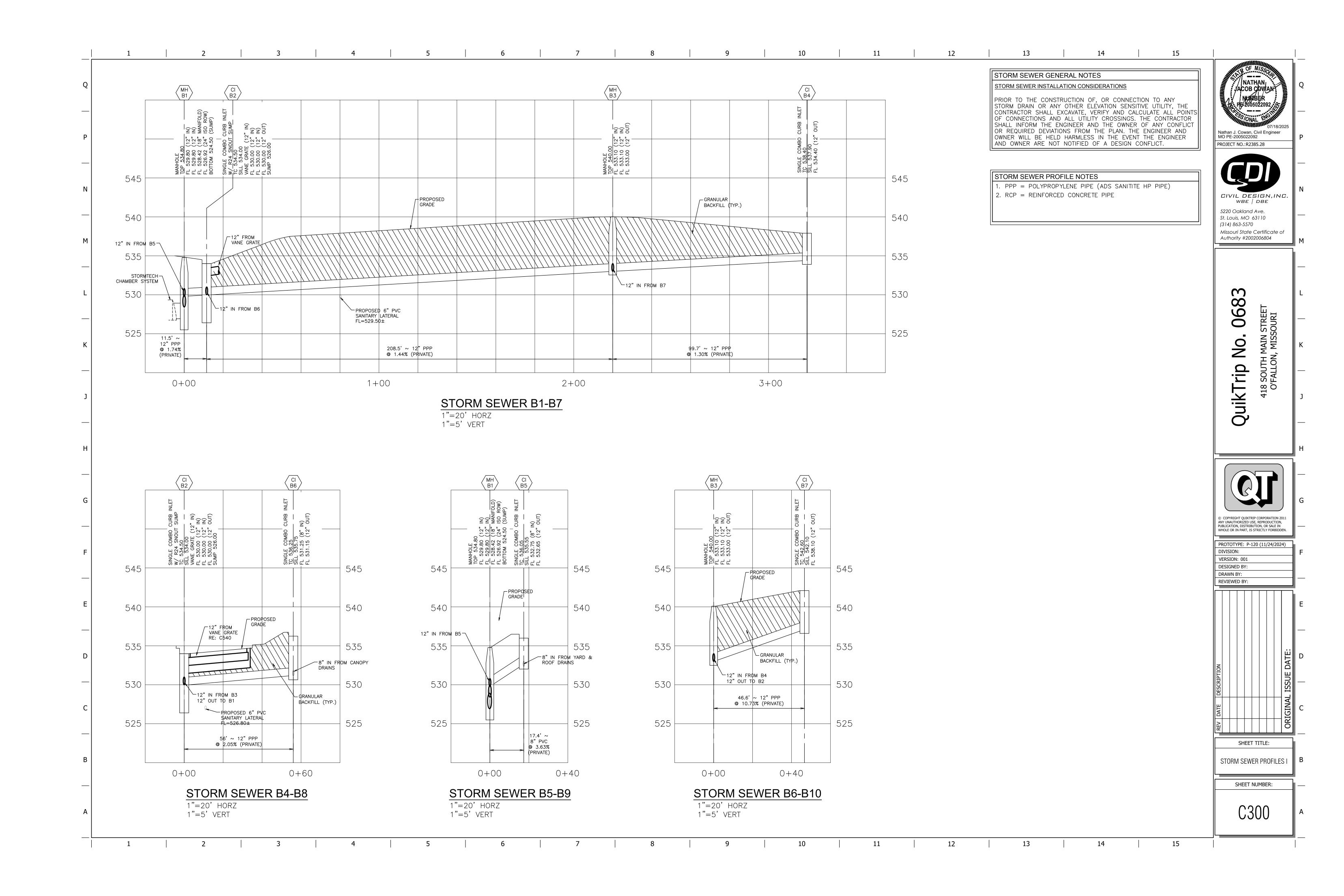


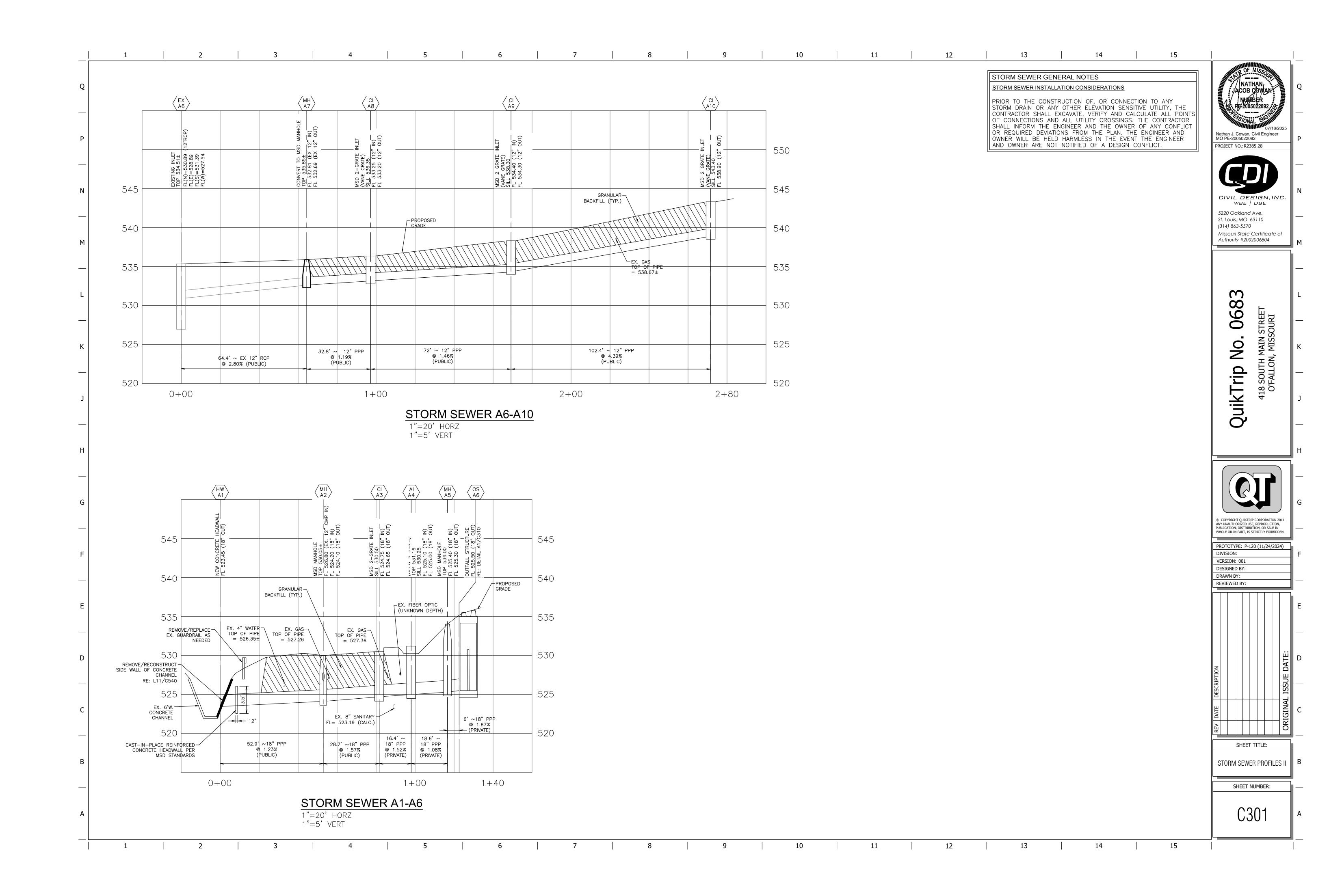


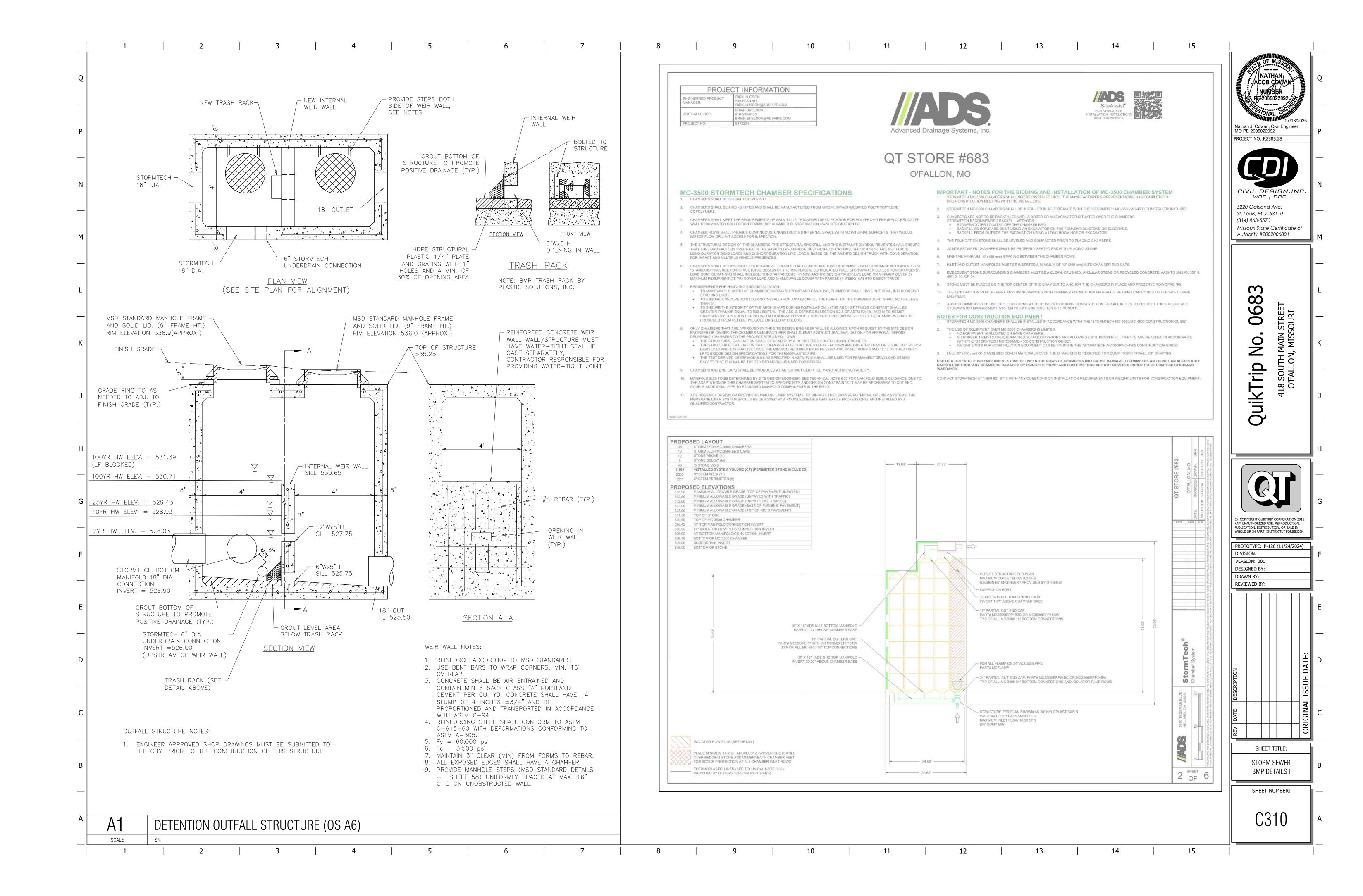


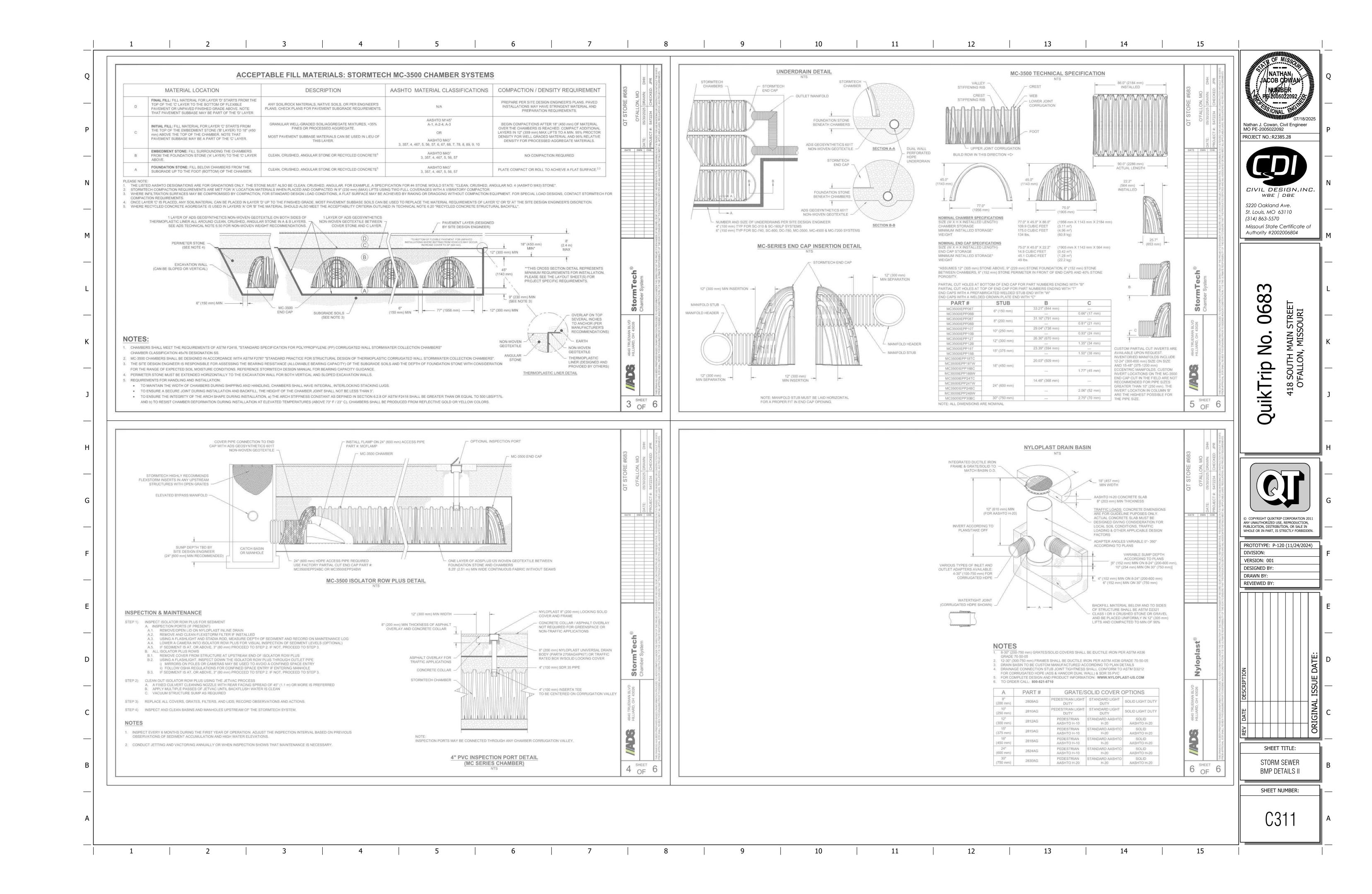


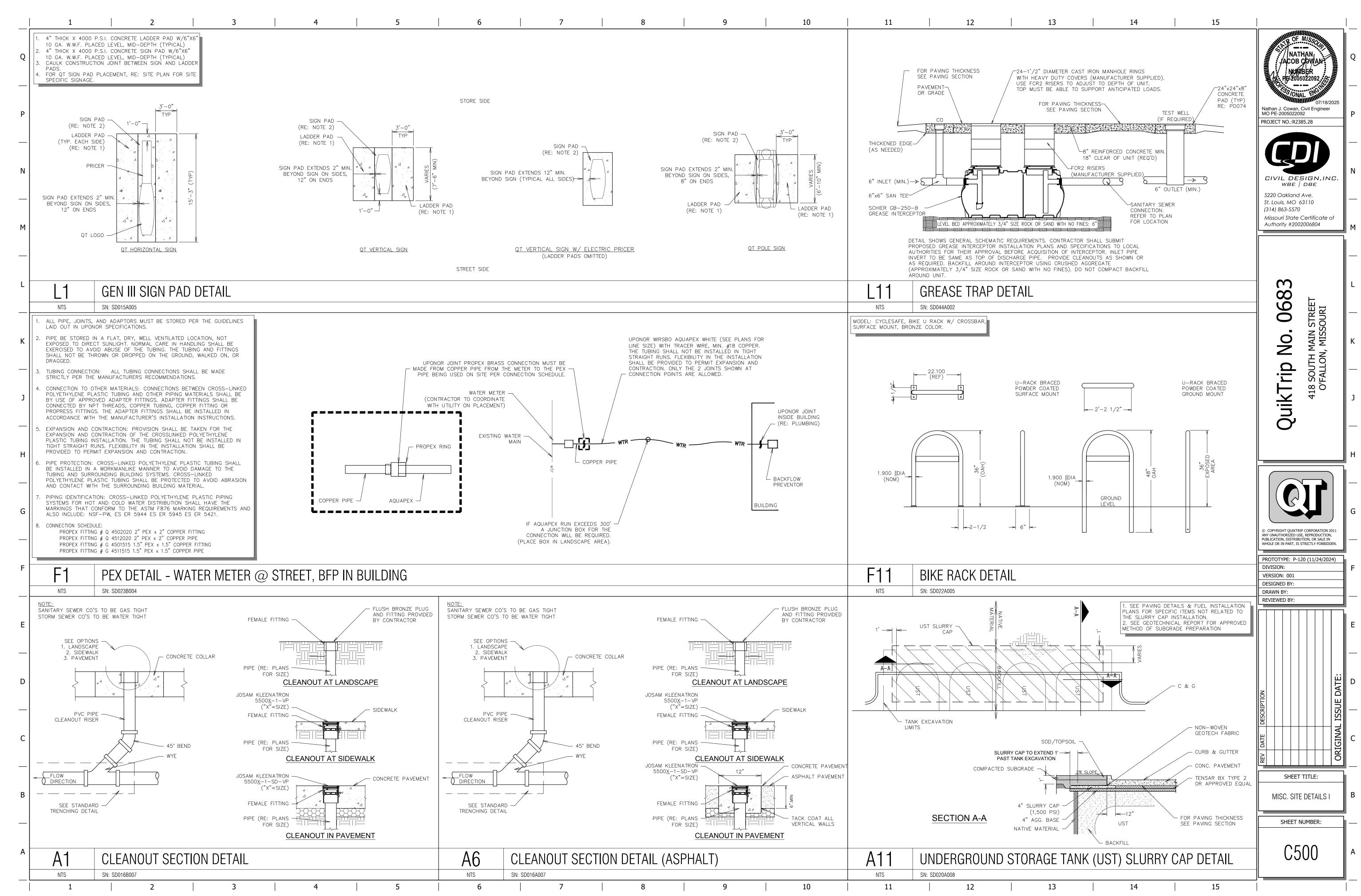


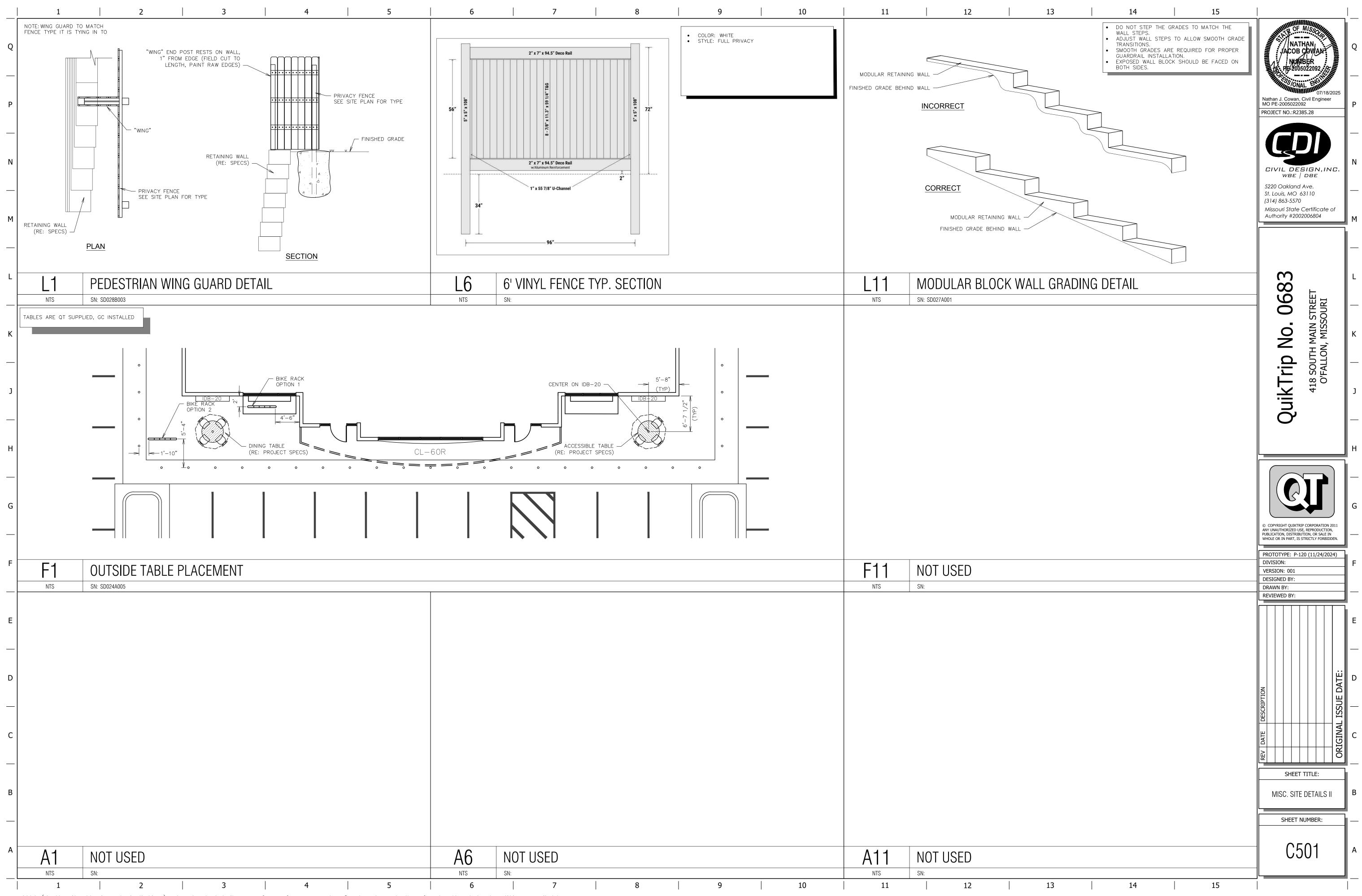


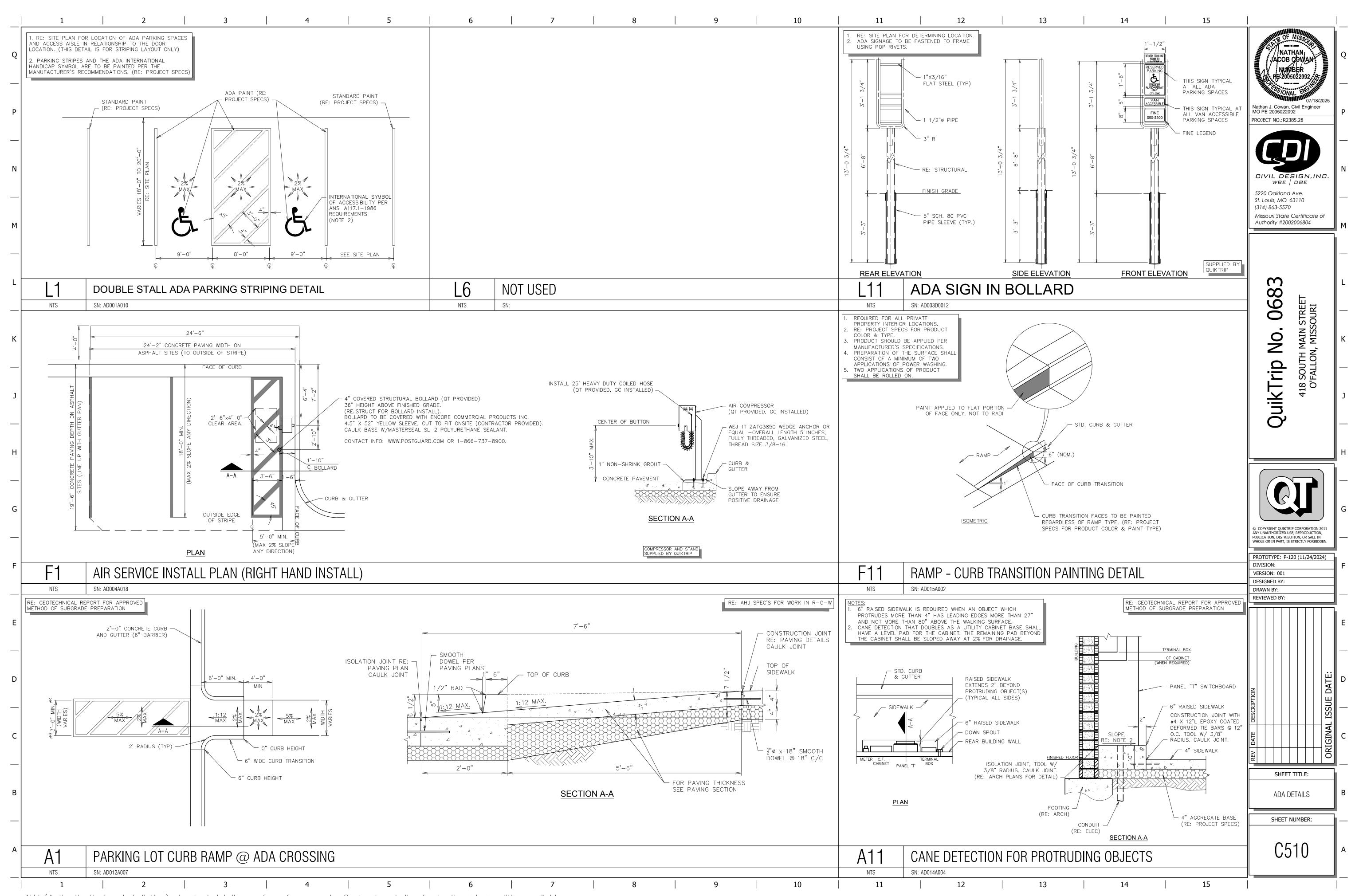




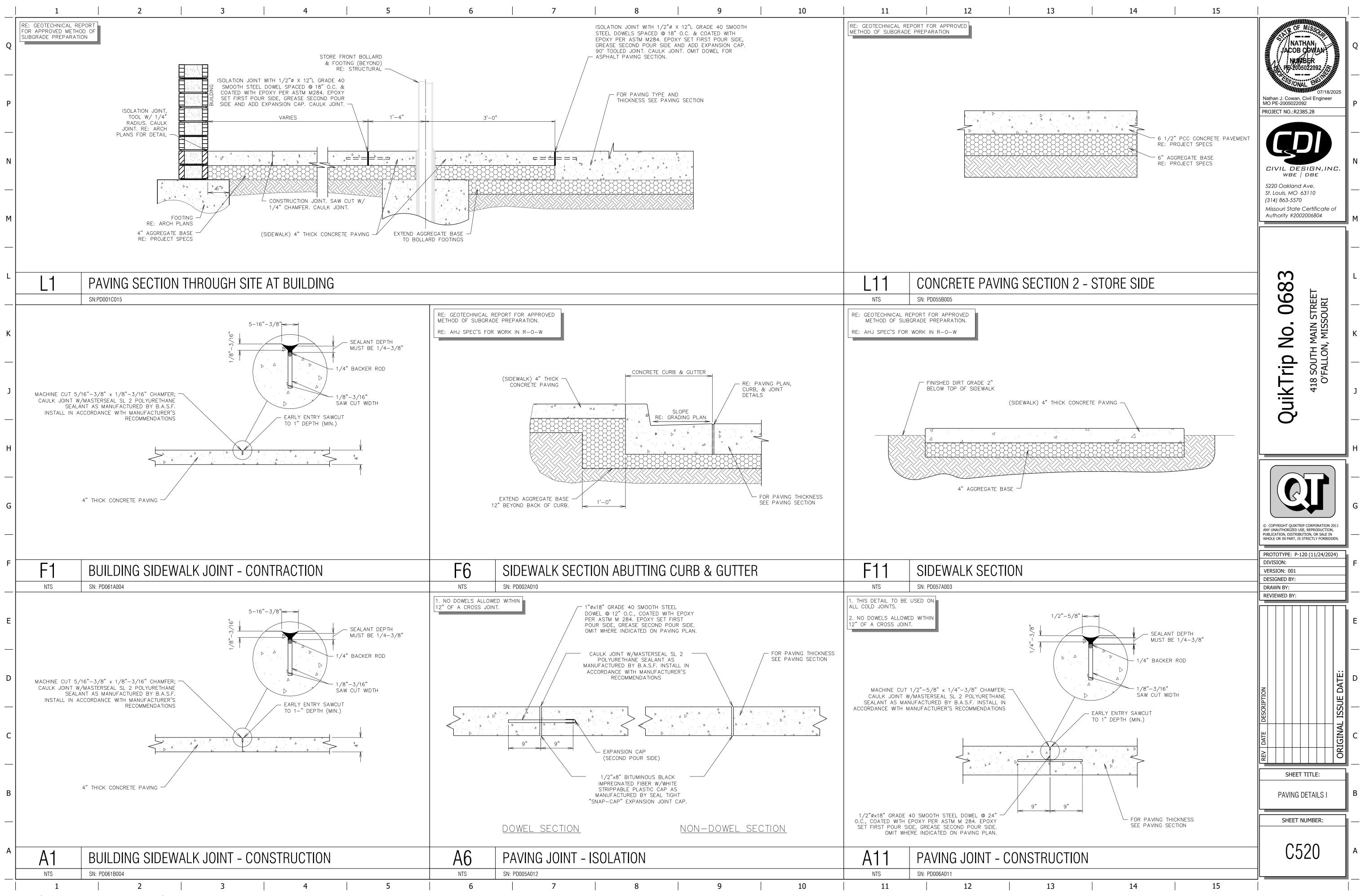




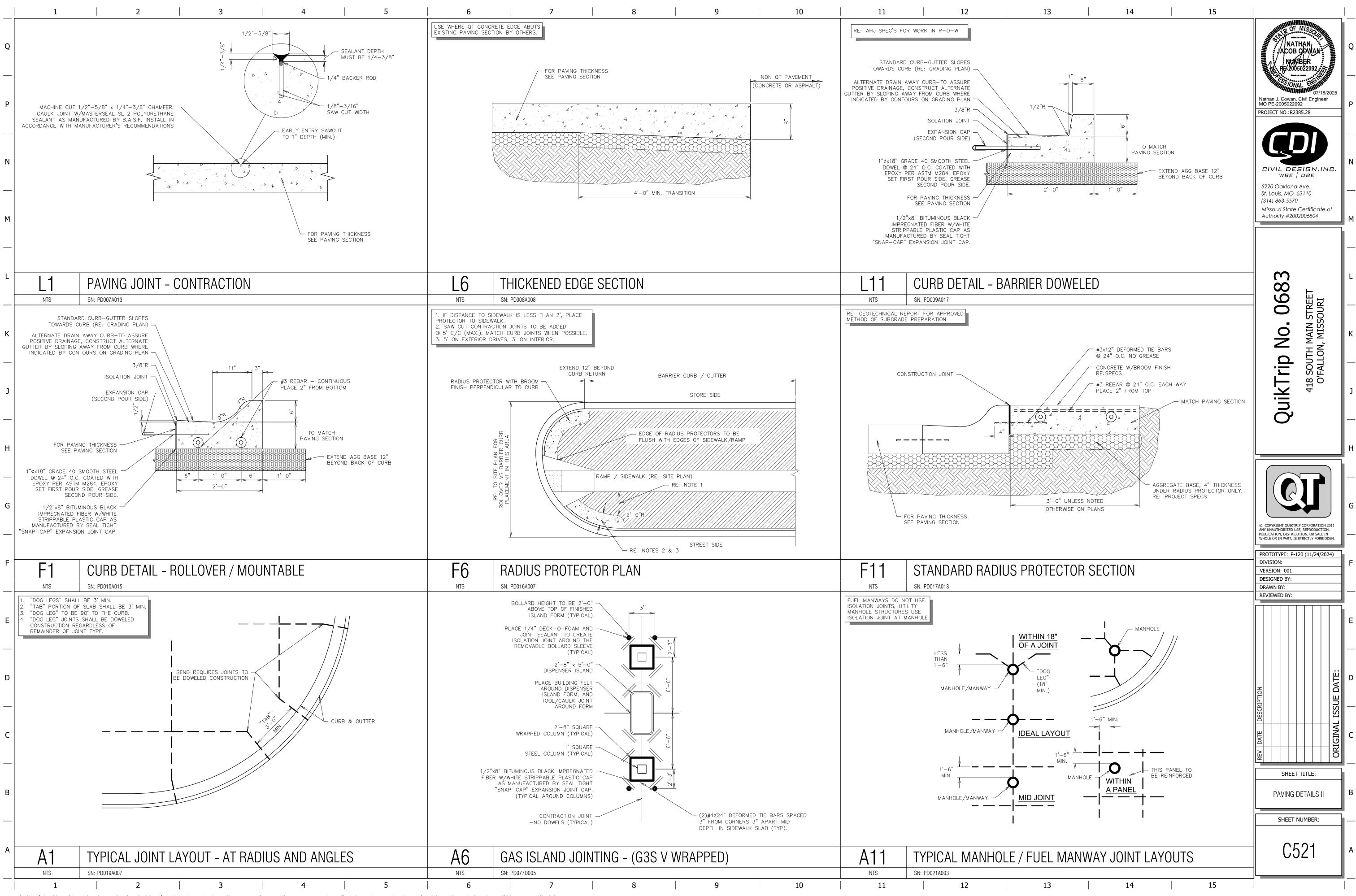




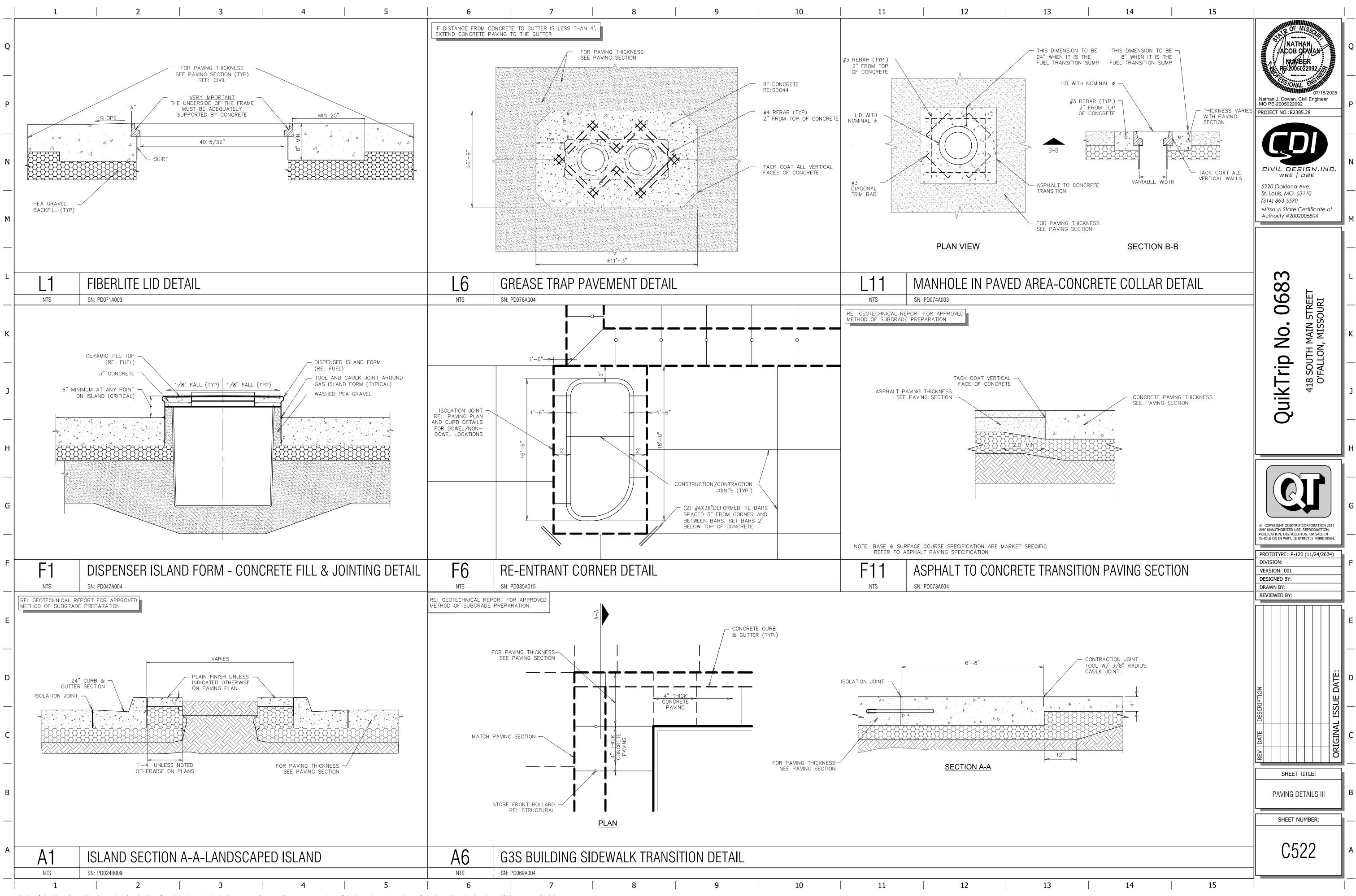
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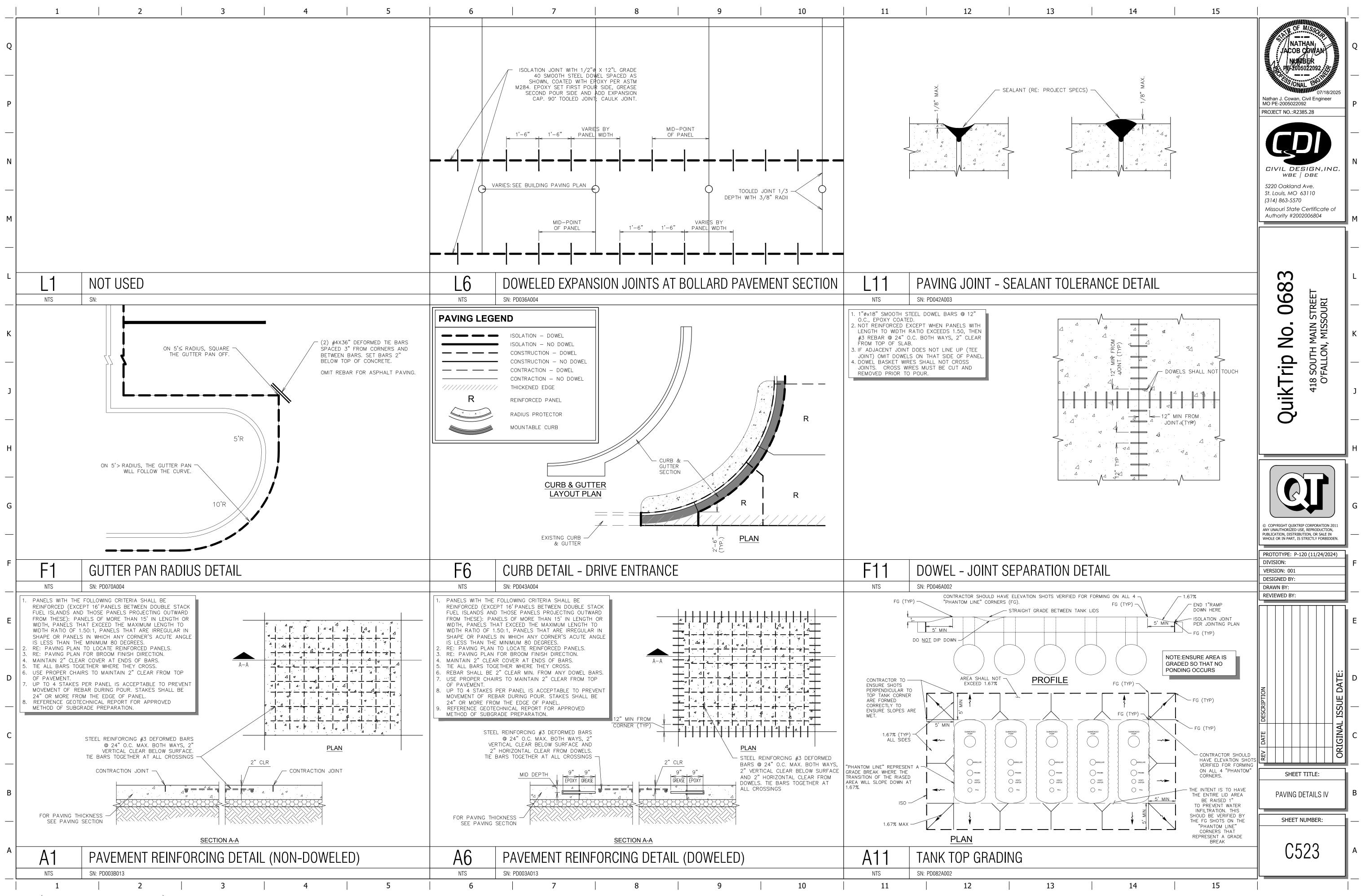
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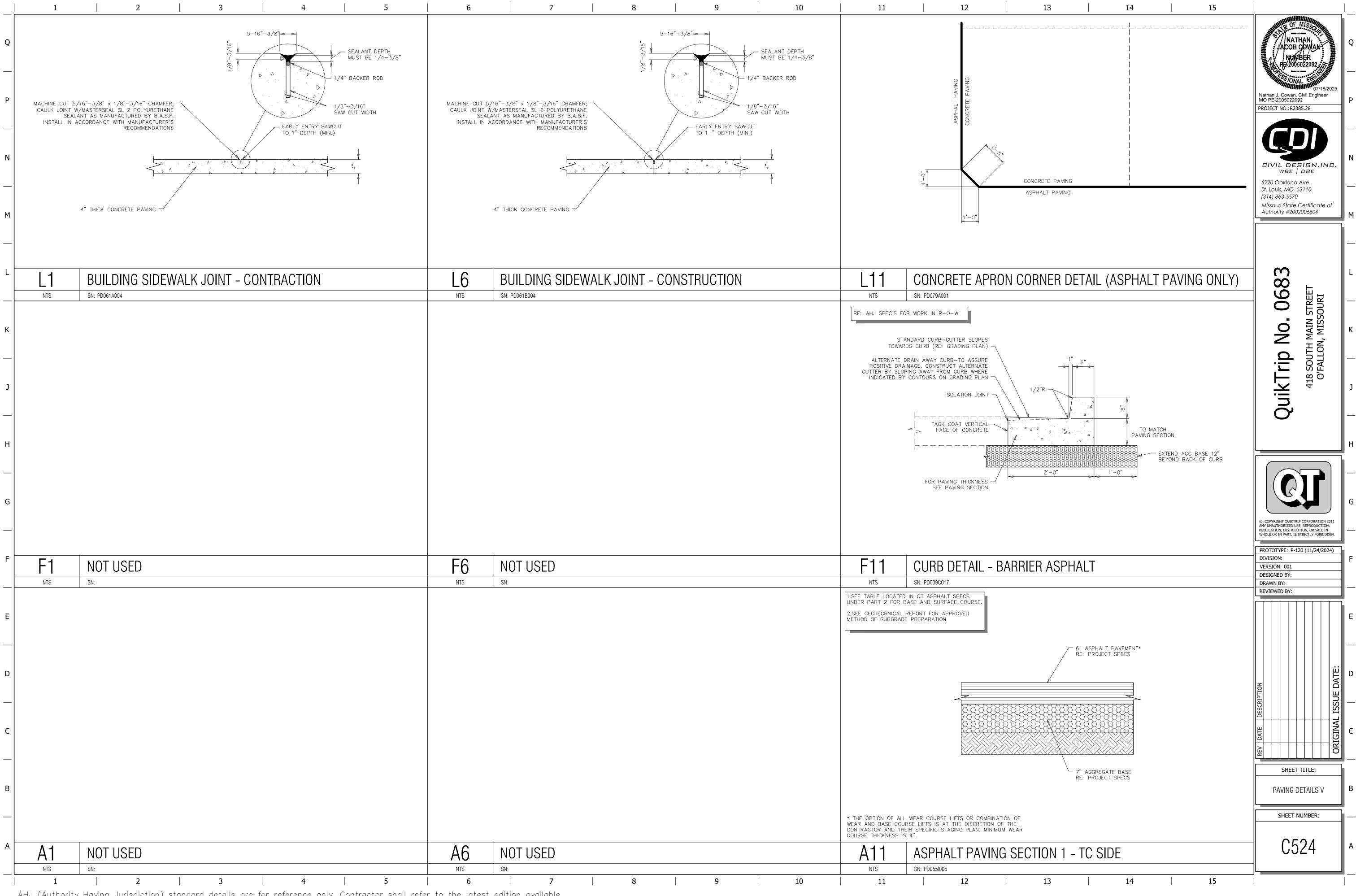
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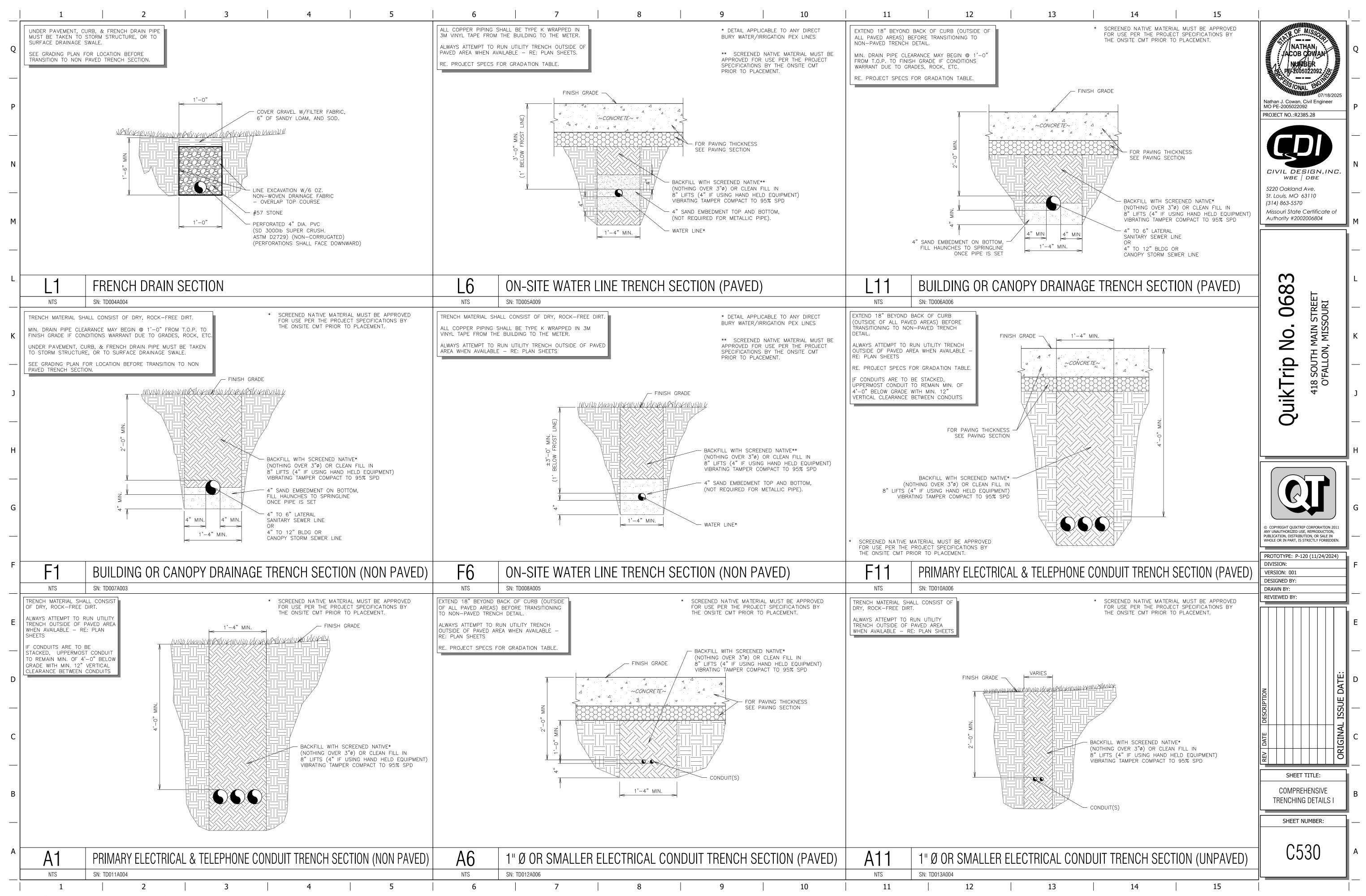
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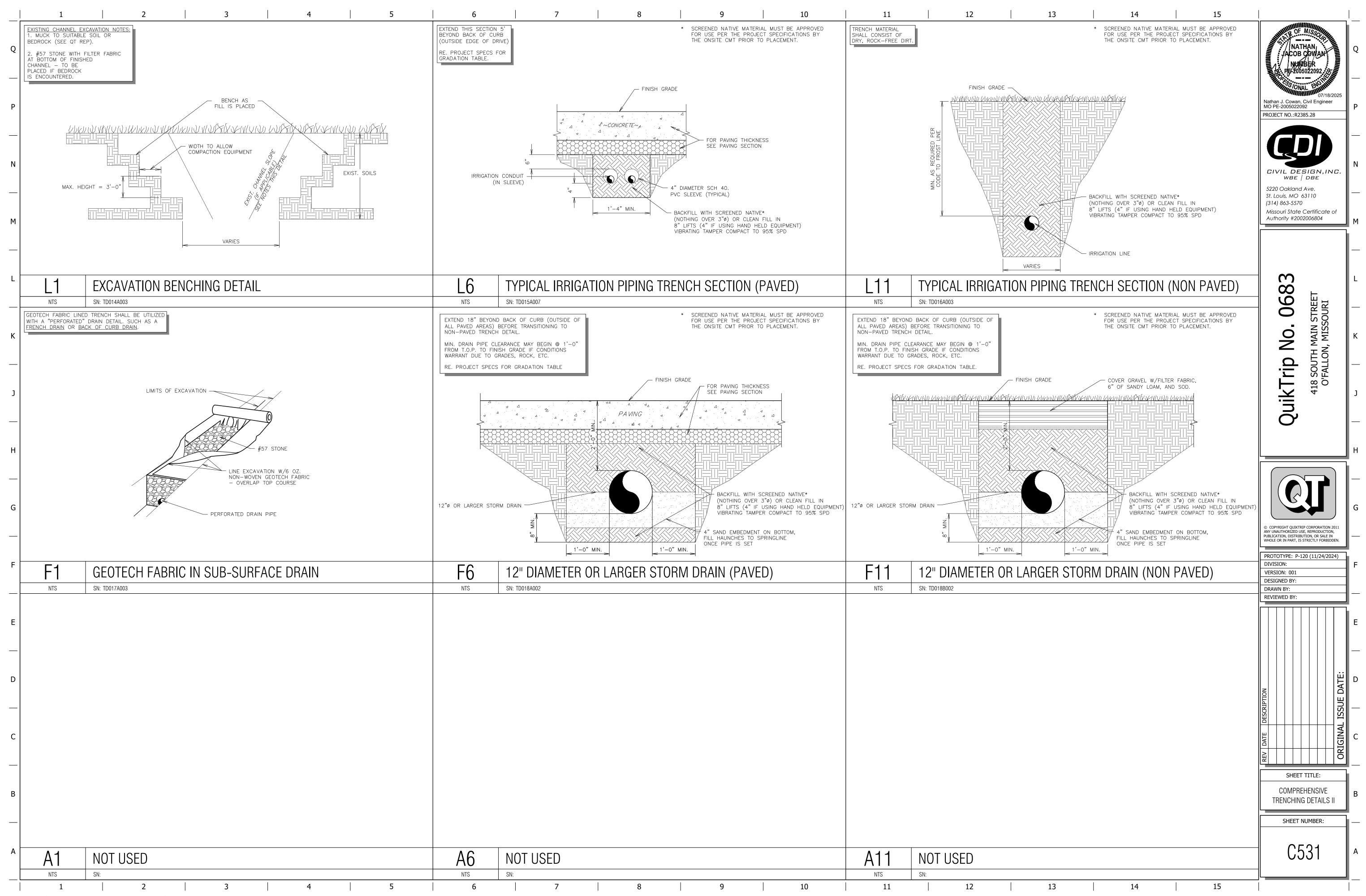


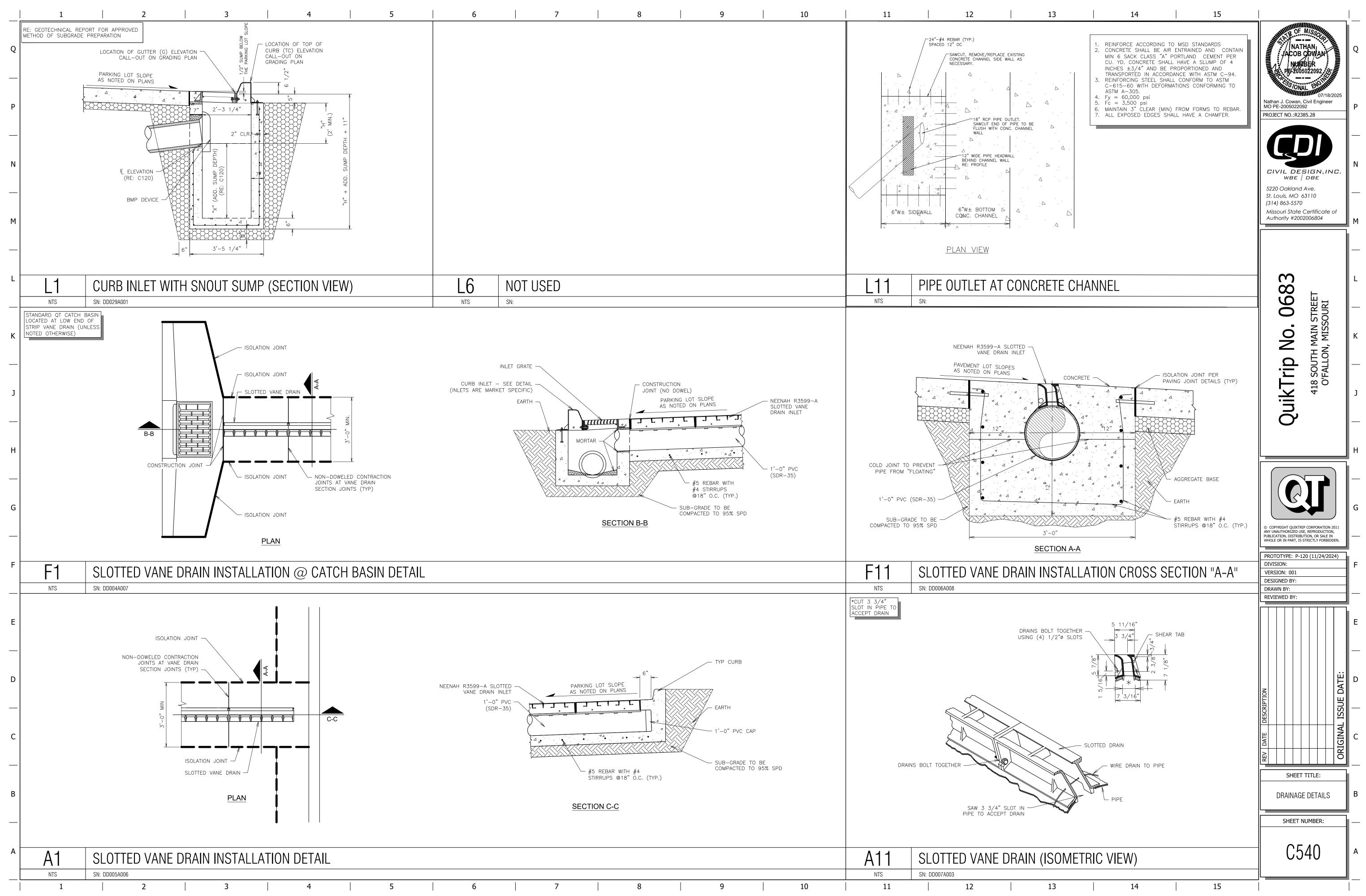
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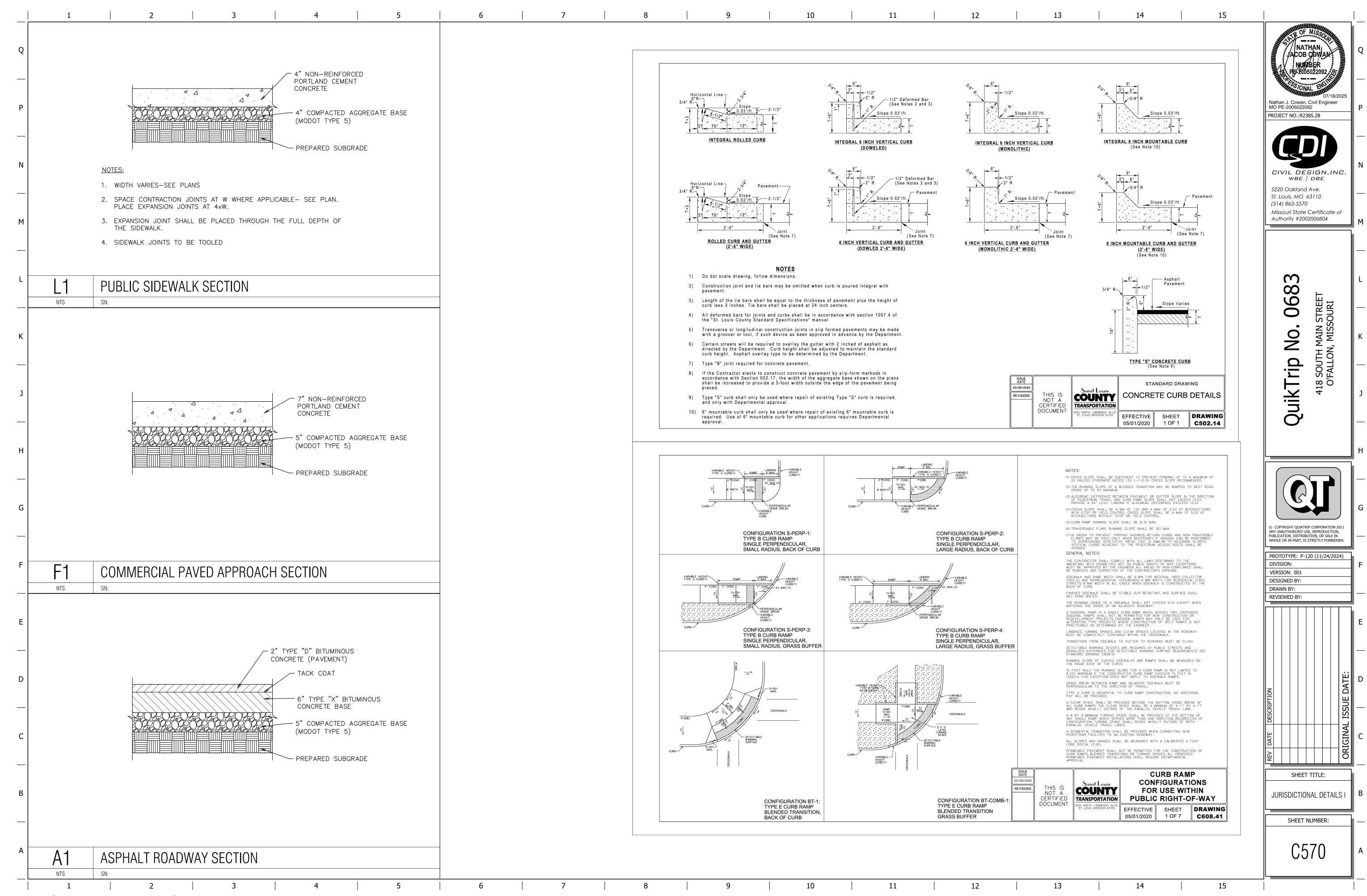


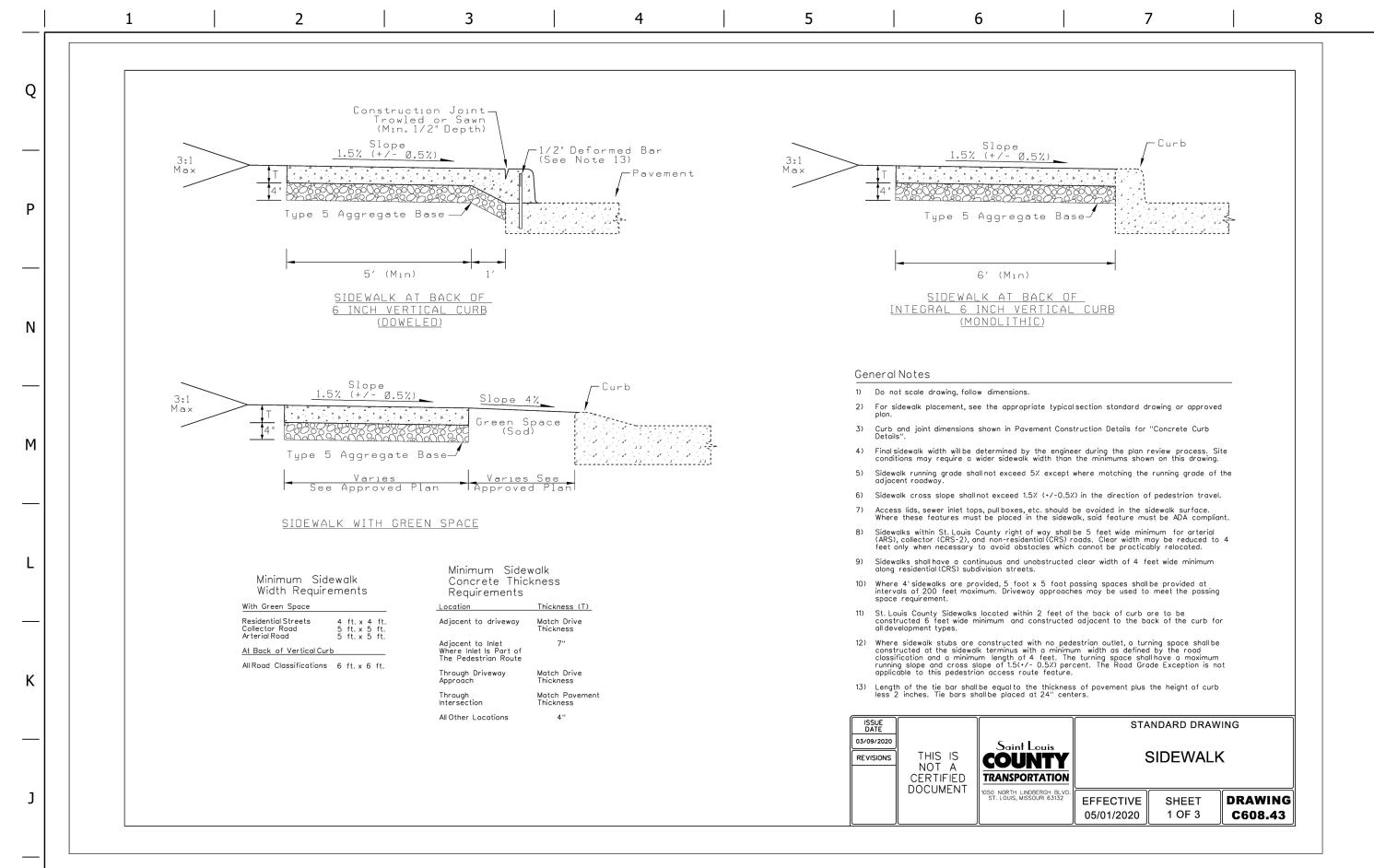
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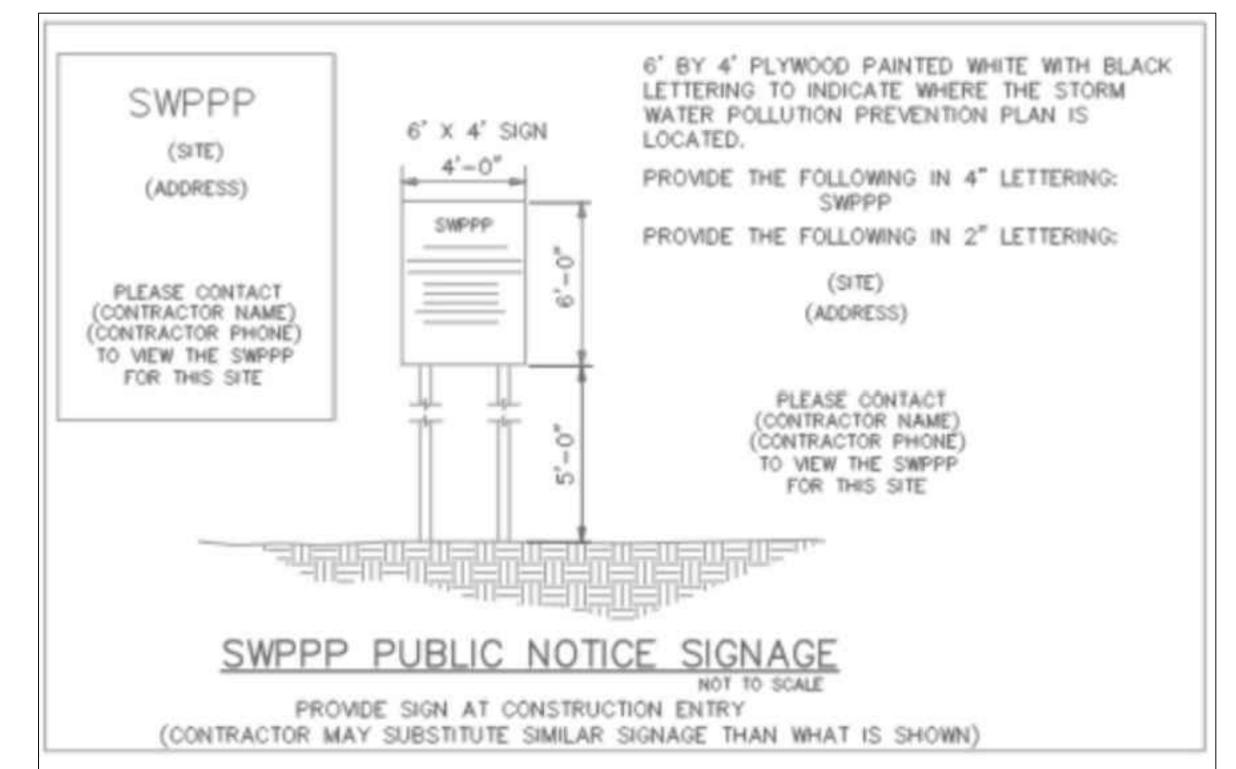


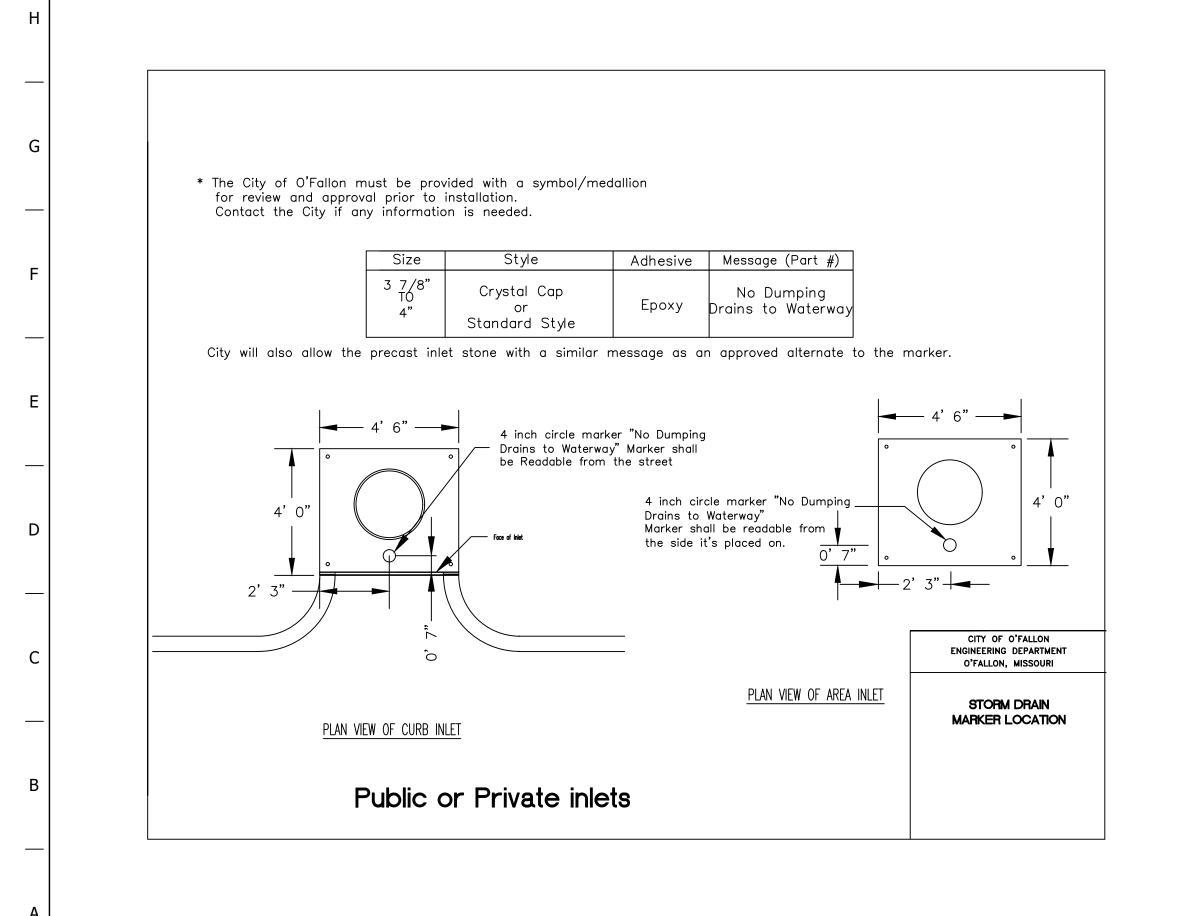


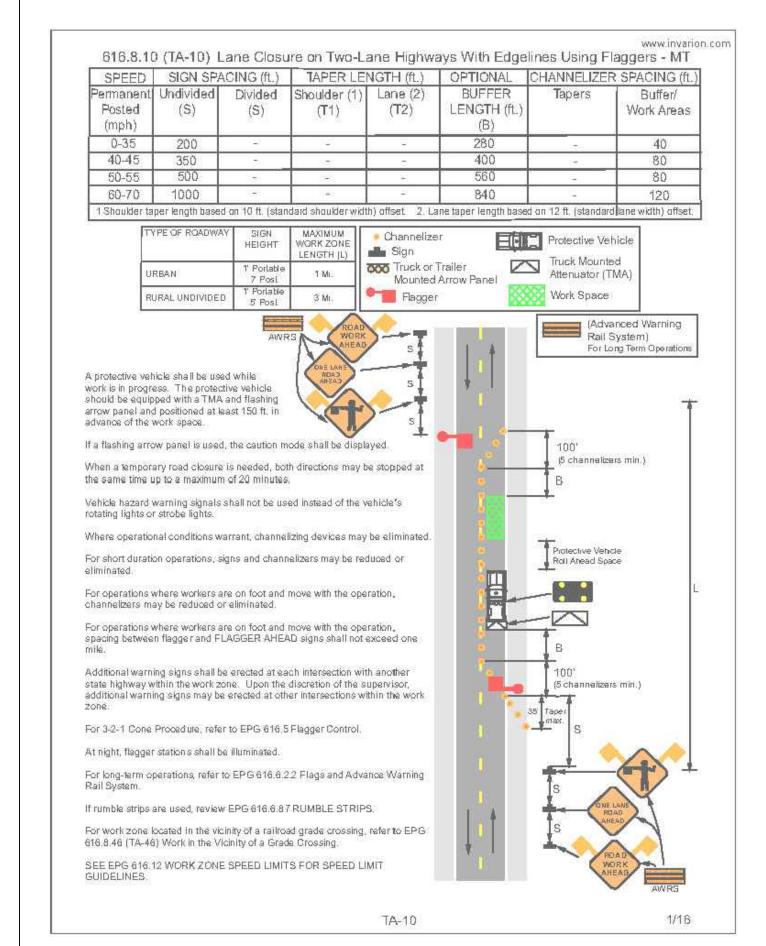


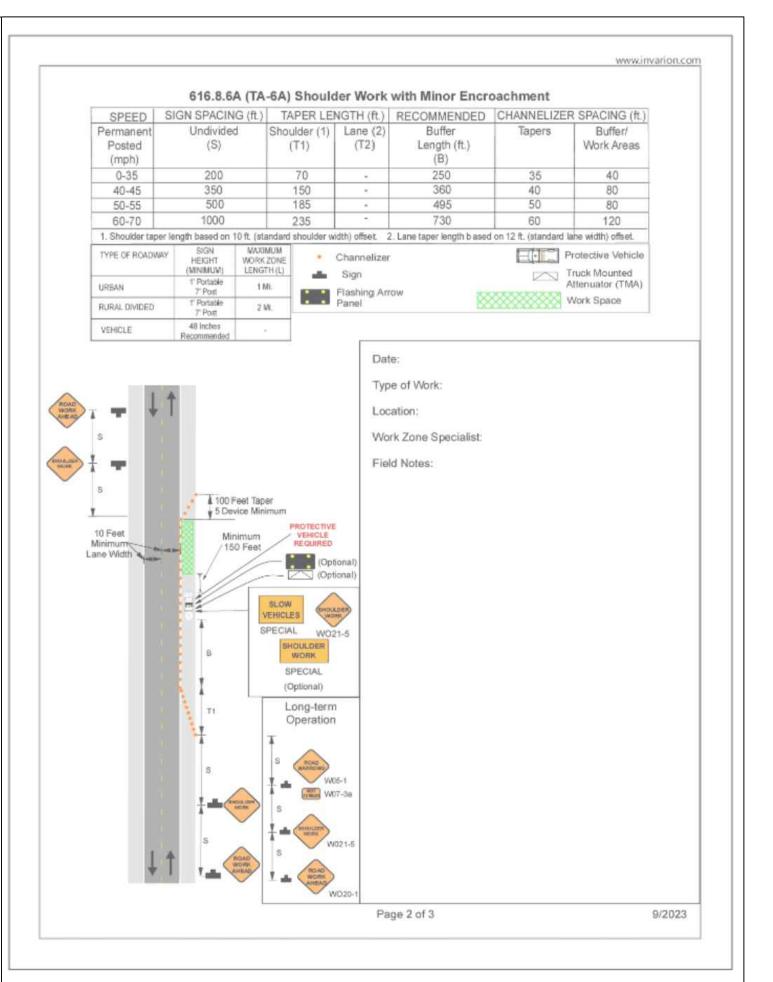


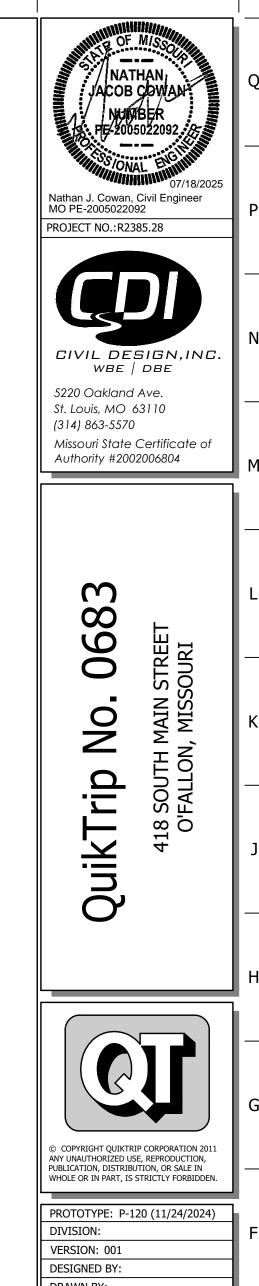


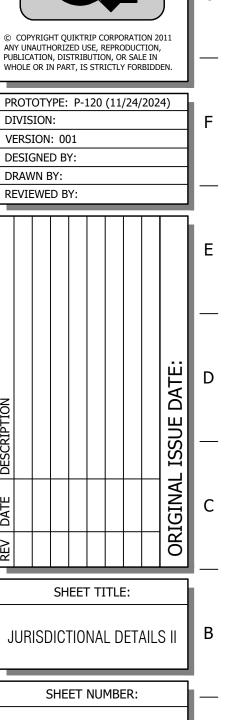


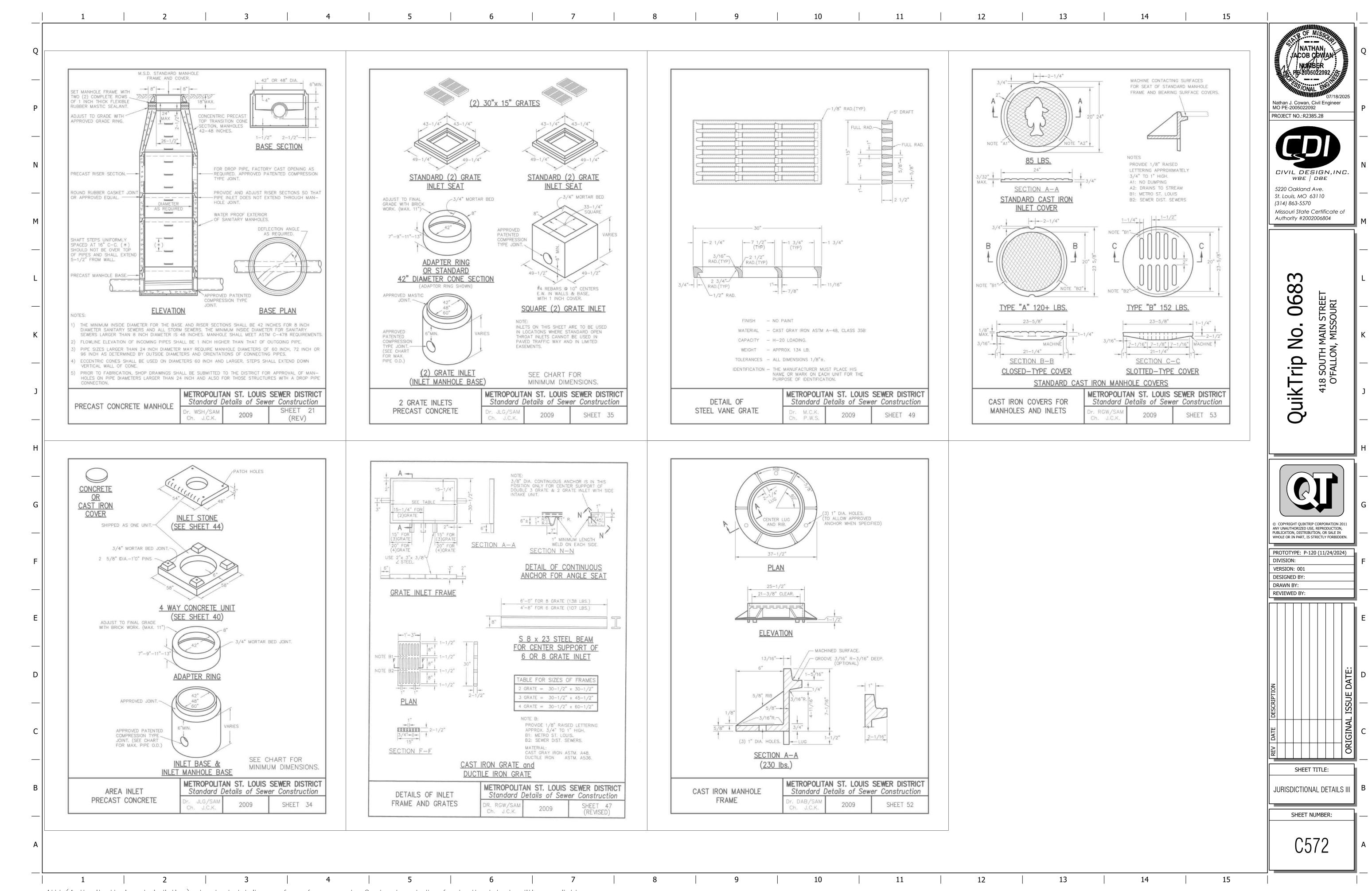


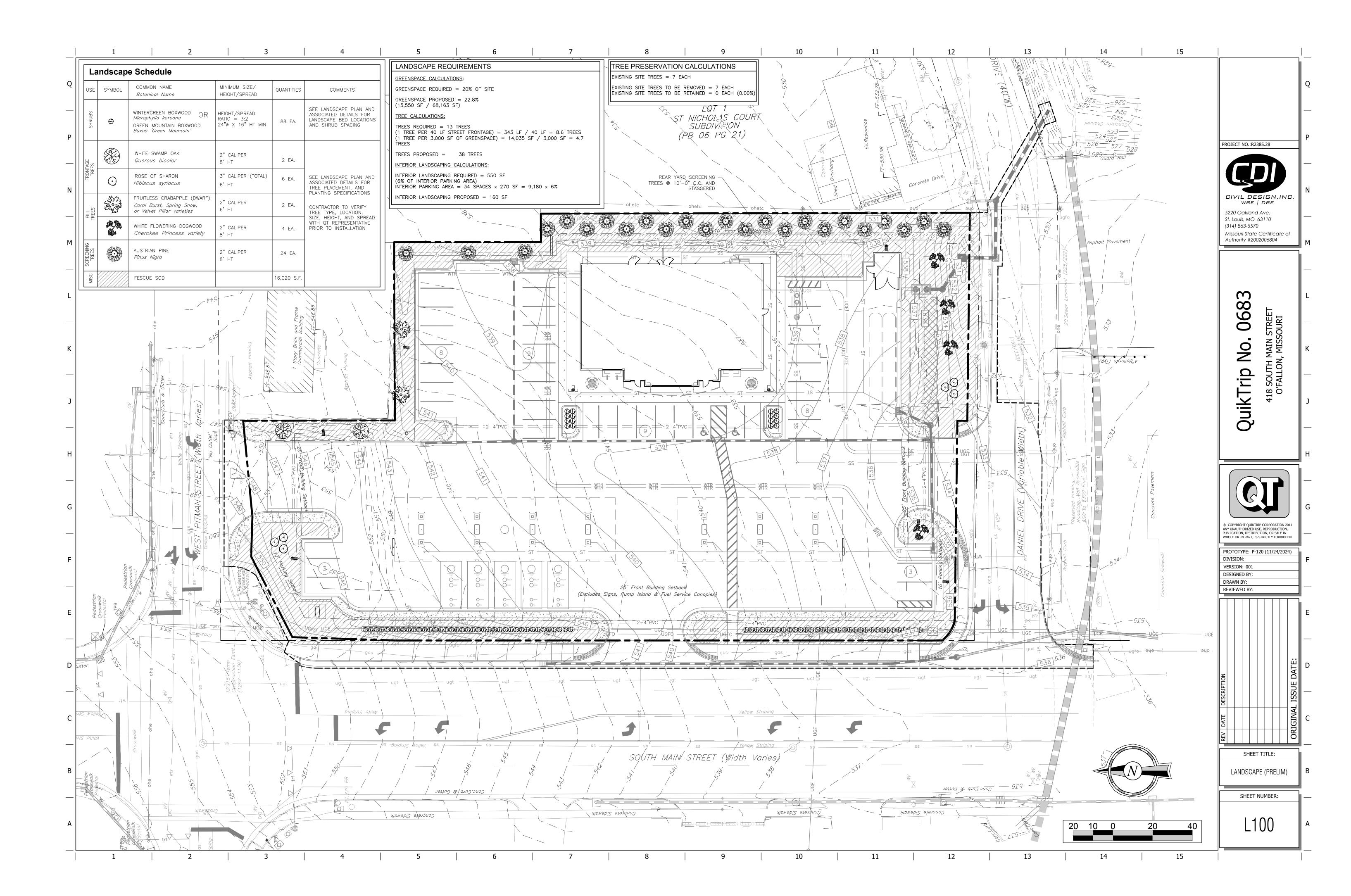


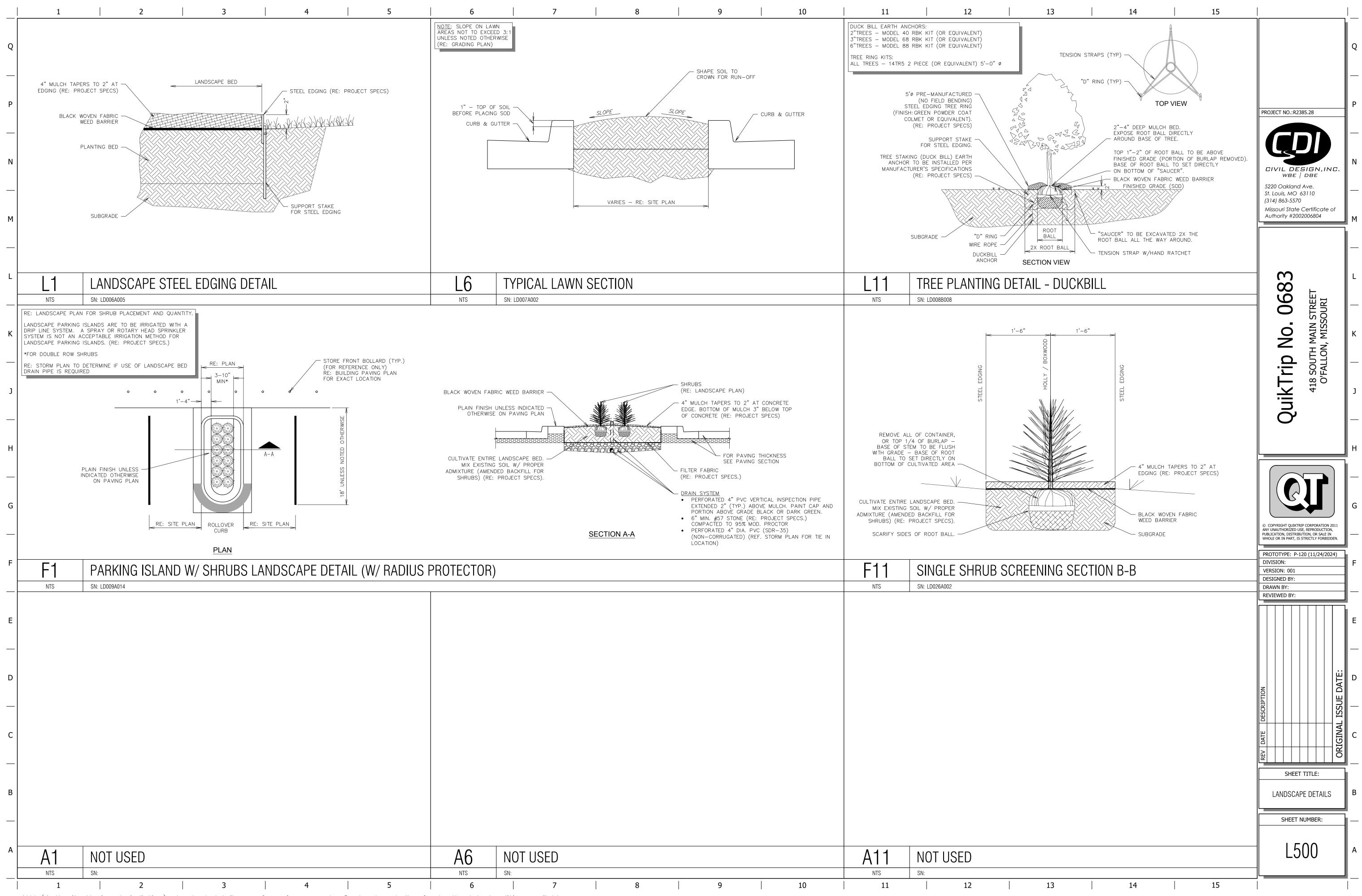




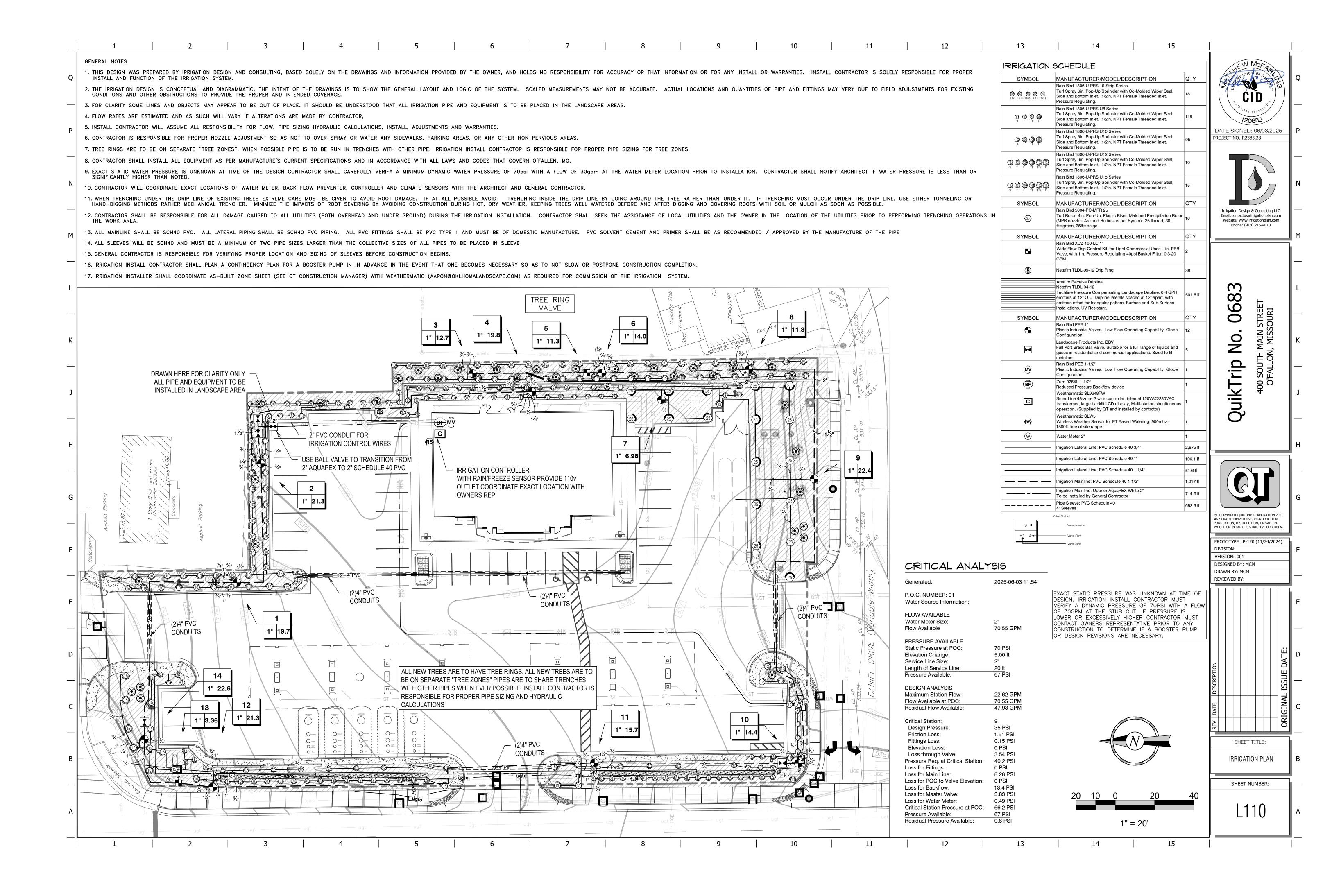


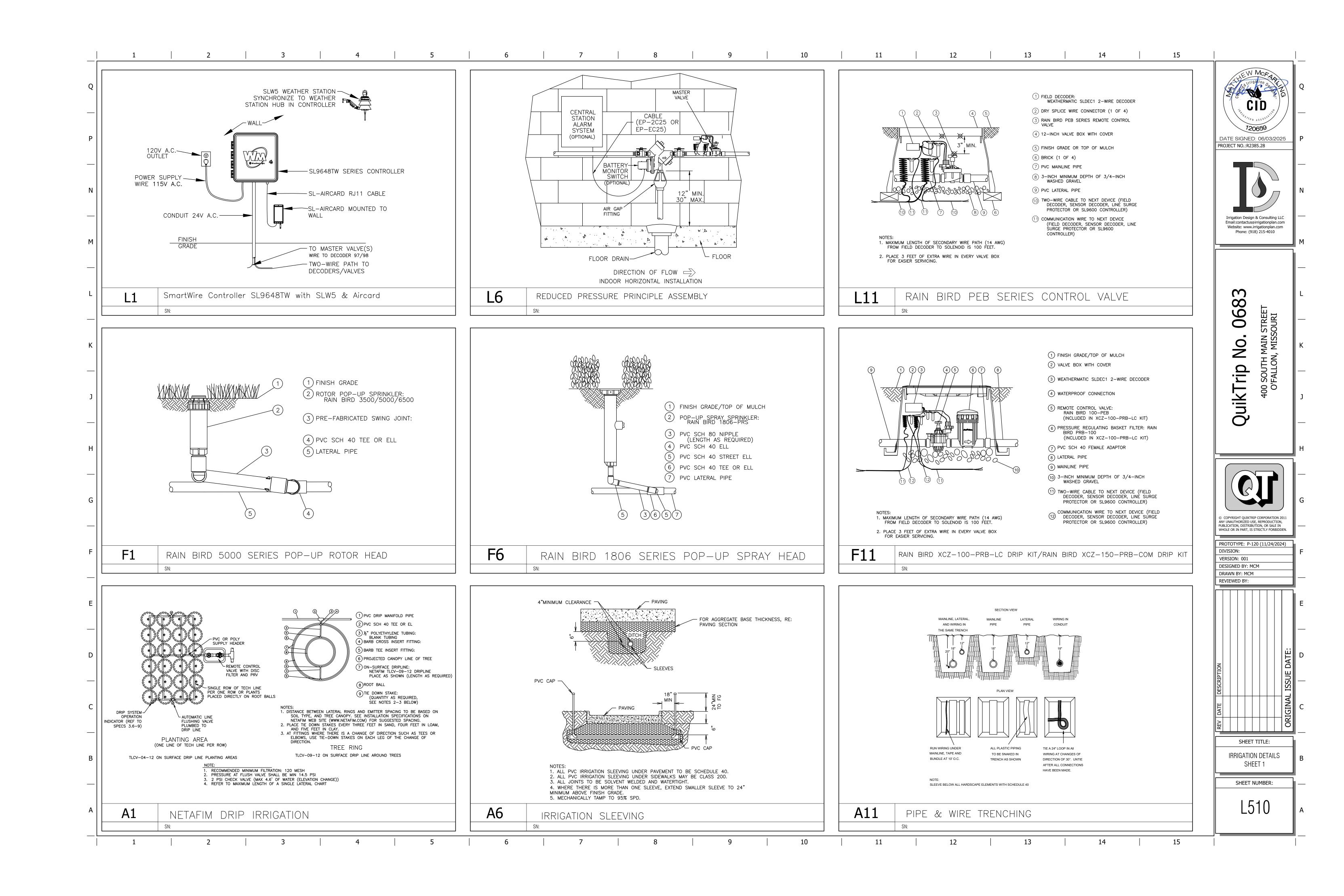




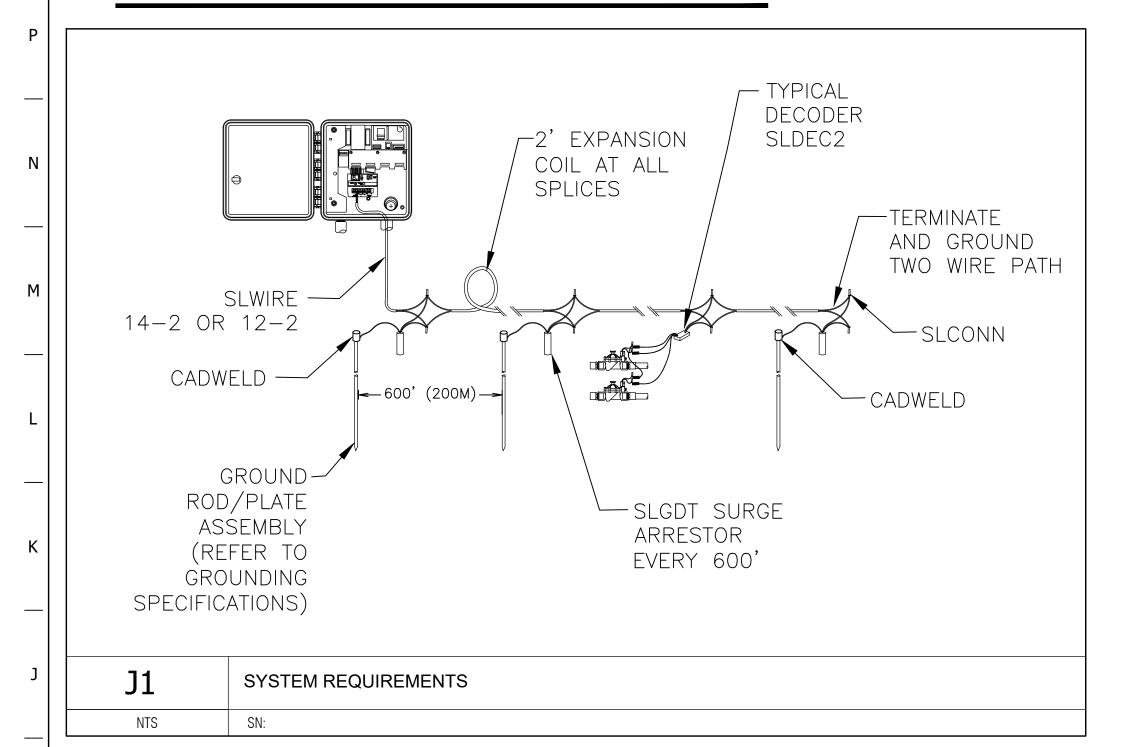


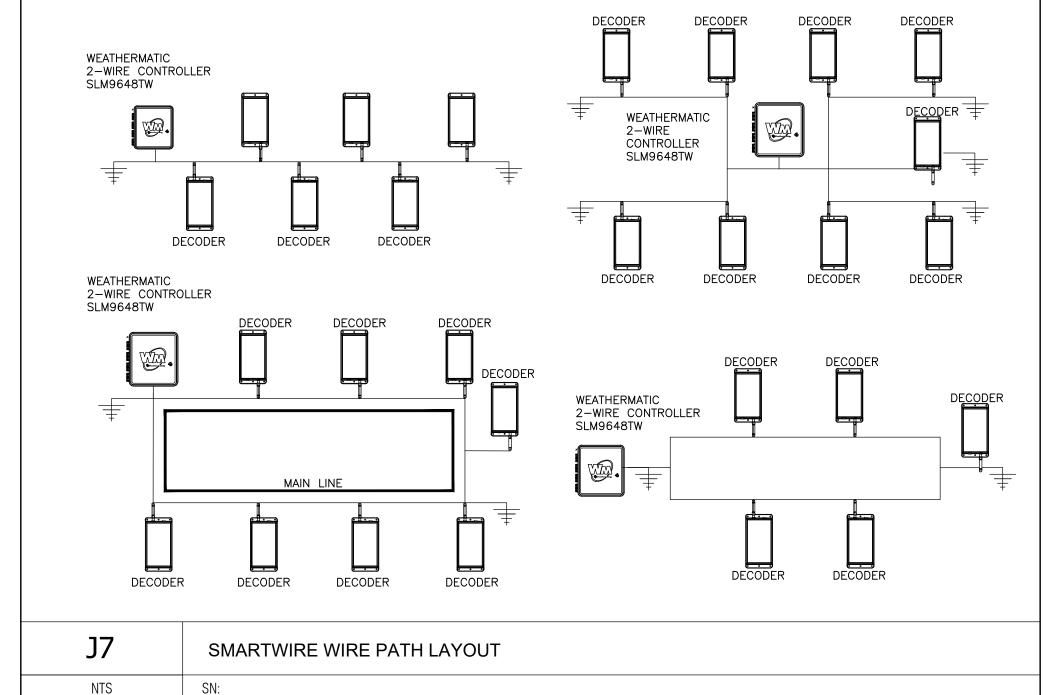
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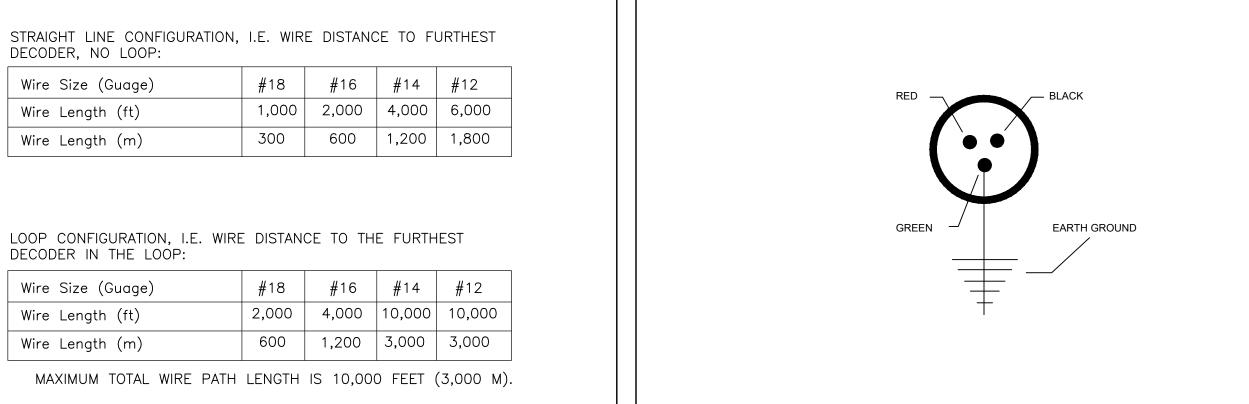


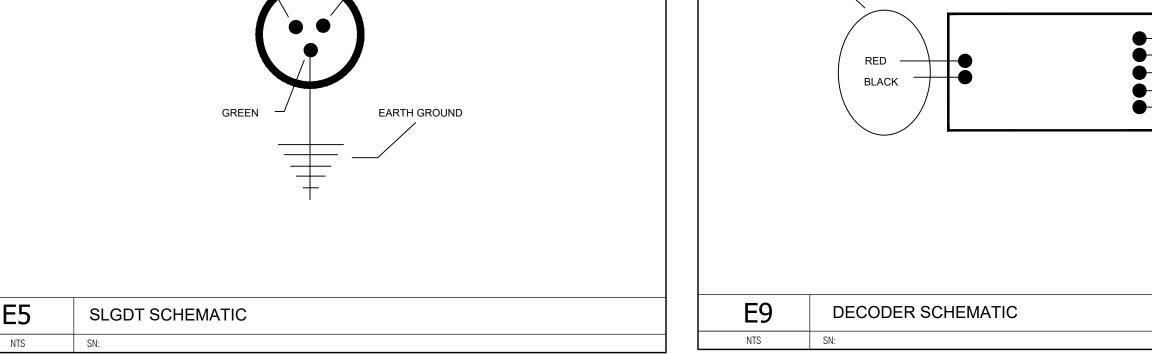
Two-wire irrigation details:

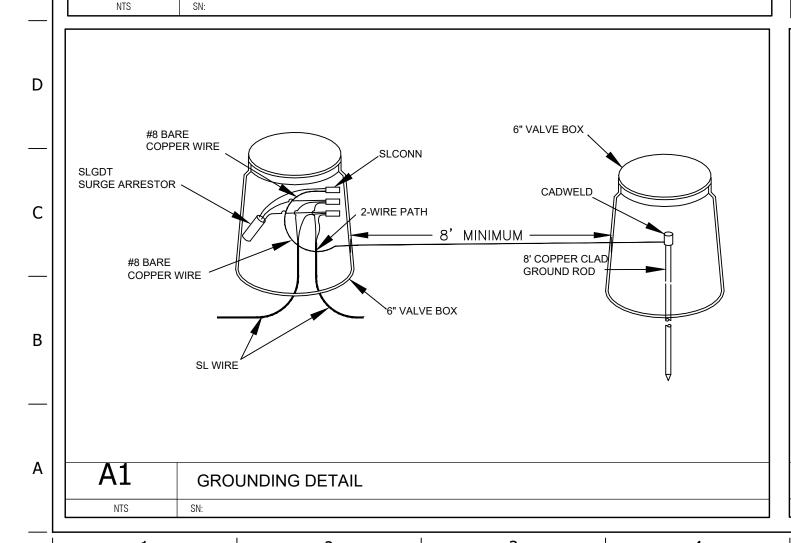




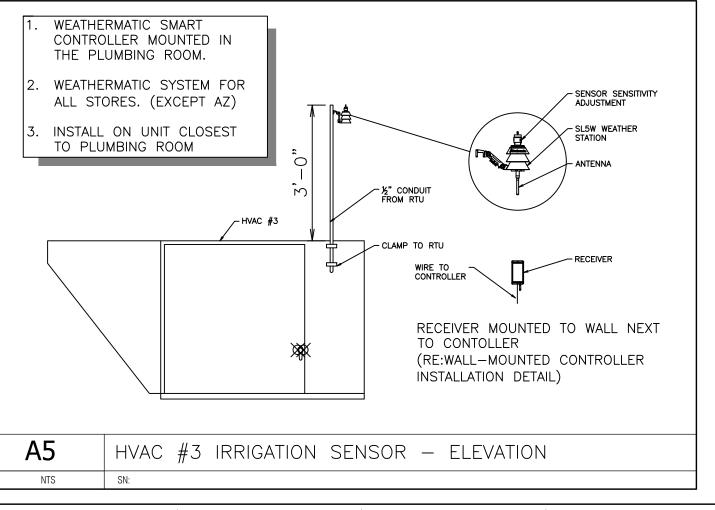
TWO WIRE PATH

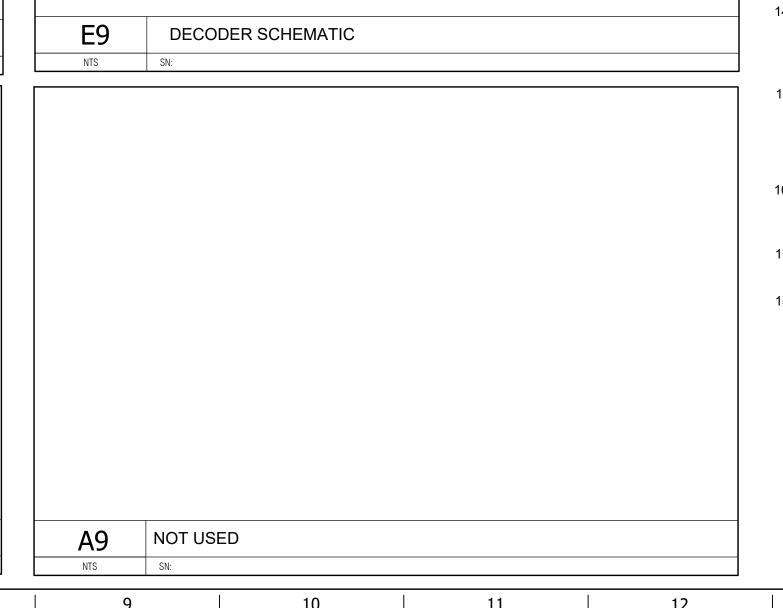






Wire Sizing





BLUE, FOURTH VALVEGREEN, THIRD VALVE

YELLOW, SECOND VALVE

WHITE, VALVE COMMON

ORANGE, FIRST VALVE

Two-wire notes:

NOTES:

1. Specific wire polarity is required. Maintain red and black wire polarity throughout installation

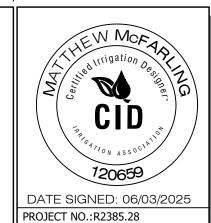
- Maintain independent wire path for each controller. No splicing or joining wire paths for multiple controls.
- 3. Up to three independent wire paths may be run for each SLM9648TW.
- 4. Maximum distance from SLDEC decoders to remote control valve not to exceed 100' (30,5m).
- Each 2-wire run may be laid out in straight run, complete loop, star, or combination layouts as shown in details. Always terminate and ground each leg of the wire path using manufacturer's recommended grounding methods.
- Use a continuous loop wire path whenever possible to provide the best communication and power path for the system. Looping provides a redundant path for the power and signal allowing the system to continue operation in the event the wire path is cut.
- 7. Mark each zone number on the SLDEC decoder. For multi-valve decoders record the zone in order of wire color code: orange, yellow, green, and blue.
- 8. Mark all zone numbers and decoder locations on the as-built plan for final submittal.
- 9. Secure all Red and Black 2-wire connections using only manufacturer's supplied SLCONN aluminum connectors and grease filled waterproof connectors.
- 10. All remote control valve wire splices shall be secured using an encapsulated, gel-filled waterproof wire connector suitable for direct burial and installed according to manufacturer's specifications.
- 11. Use SmartWire SLWIRE cable supplied by Weathermatic sprinkler division of Telsco Industries to comply with the following specifications:
- a. Conductors must be soft drawn, annealed, solid copper conforming
- b. Conductor insulation must be 4/64-inch thick polyvinyl chloride (PVC) conforming to UL #493.
- c. The two insulated conductors laid in parallel and encased in a single outer jacket of 3/64-inch thick, high-density, sunlight resistant polyethylene conforming to ICEA S-61-402 and NEMA WC5, having
- a minimum wall thickness of .045-inch.d. The two conductors must be color-coded: normally one conductor
- red and the other black. Both conductors shall be the same size.
- e. The following brands meet the above specifications for direct burial cable: Weathermatic SLWIRE12; Weathermatic SLWIRE14.
- 12. Wire sizing and maximum distance shall not exceed manufacturer's
- 13. All 2-wire paths under hardscape surfaces shall be sleeved and include no splices of the 2-wire path within 5' of sleeves. Secure a 2' (0,6m) expansion loop in the two-wire path at each change of direction and entry and exit of sleeve locations

during wire installation. Release the expansion loops prior to direct burial.

recommendations to avoid voltage loss and insure proper operation of the system.

- 14. Install one SLGDT surge arrestor at the following locations along the 2-wire path: within 25' (7.5m) of each controller, the farthest distance on the 2-wire path from the controller, every 600' along the wire path, at any independent branch termination greater than 50' (15m) from than main 2-wire path.
- 15. Connect each SLGDT to an 8' copper clad UL approved grounding round or other approved grounding method to obtain 12 ohms or less resistance. Use CADWELD TM permanent welded, low-resistance connection to assure best grounding performance. Consult ASIC specification 100-2002 for additional grounding specifications for appropriate for local soil conditions.
- 16. Use care in stripping all wire connections to avoid damage to the 2-wire shield and red and black wire path. Remove and discard any damaged 2-wire sections and repair using two SLCONN splice kits.
- 17. Use expansion coils on the red and black wires at each decoder installation. No expansion coils are necessary for the SLGDT.
- 18. Adhere to all local and national building and electrical codes.

Scale: Not to Scale



Irrigation Design & Consulting LLC Email:contactus@irrigationplan.com

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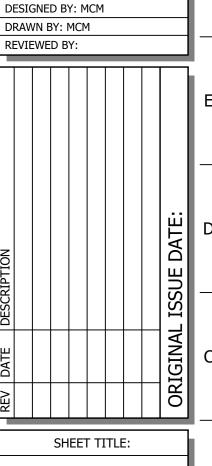
QuikTrip No. 0683
400 SOUTH MAIN STREET
O'FALLON, MISSOURI



PROTOTYPE: P-120 (11/24/2024)

DIVISION:

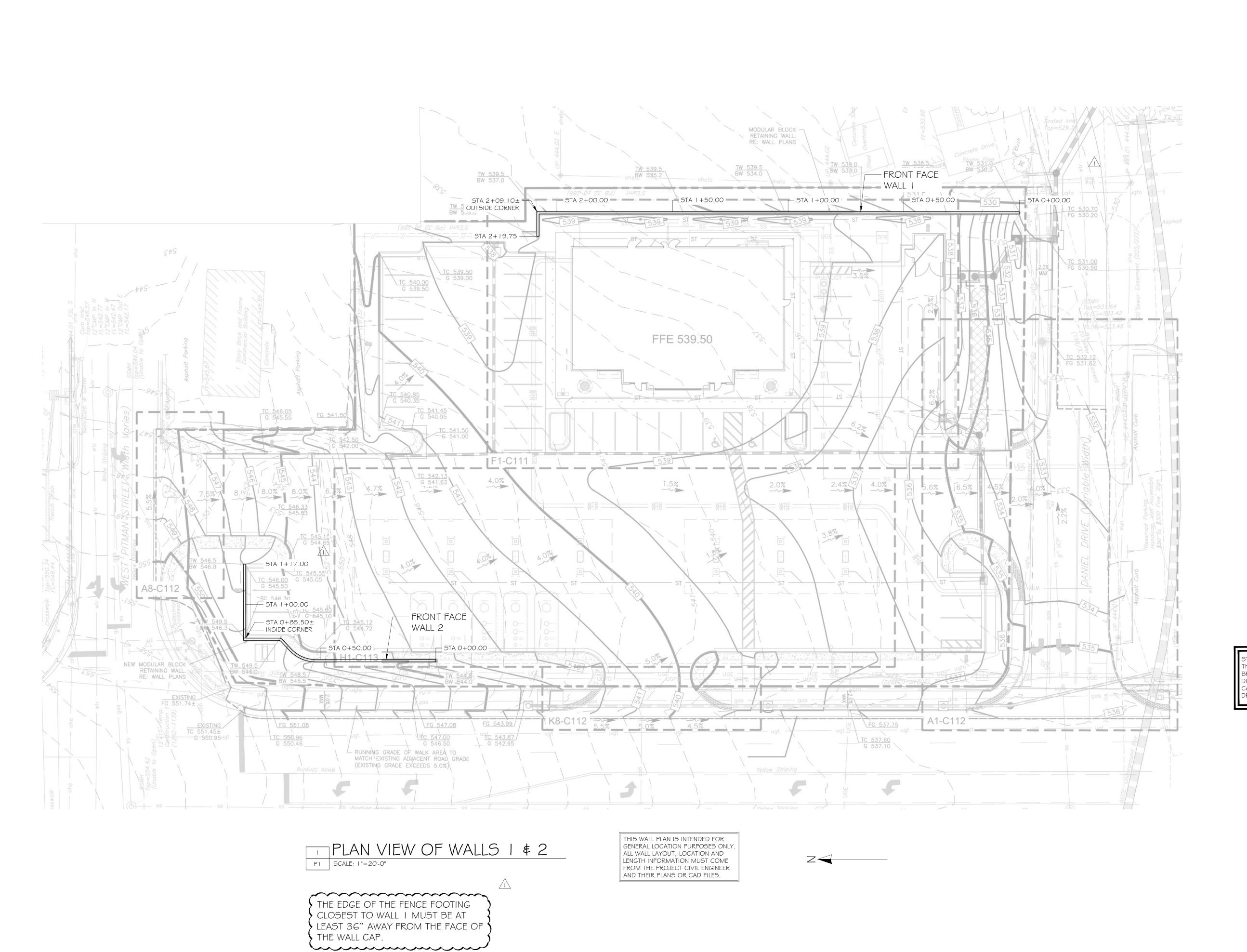
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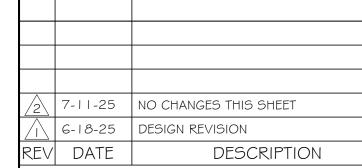
SHEET NUMBER:

IRRIGATION DETAILS SHEET 2

QUIK TRIP 0683 RETAINING WALLS THE GENERAL CONTRACTOR/OWNER SHALL PROVIDE ALL INTERIM BRACING, SHORING, INTERIM DRAINAGE PROVISIONS, DRAINAGE DIVERSION AND EROSION PROTECTION REQUIRED UNTIL FINAL CAPPING, PAVING, CURBING AND COMPLETION OF FINAL STORM DRAIN SYSTEM IS COMPLETE. THESE PLANS ARE FOR THE EXCLUSIVE USE OF ROSCH CONSTRUCTION. USE OF THESE PLANS BY ANY OTHER ENTITY TO CONSTRUCT THE SUBJECT STRUCTURES WILL RENDER THE ENGINEERING SEAL SHOWN NULL AND VOID. NEW MODULAR BLOCK RE: WALL PLANS 7-11-25 UPDATED GENERAL NOTES 6-18-25 DESIGN REVISION DESCRIPTION EXISTING TC 551.45± FL(W)=527.54 G 550.95ugt **ROSCH ENGINEERING** - RUNNING GRADE OF WALK AREA TO 18390 WINGS CORPORATE DRIVE MATCH EXISTING ADVACENT ROAD, GRADE CHESTERFIELD, MO 63005 (EXISTING GRADE EXCEEDS 5.0%) PHONE: 636-519-7770 **ROSCH** FAX: 636-532-7773 MO CERTIFICATE OF AUTHORITY #E-20 | 2039663 QUIK TRIP 0683 SOUTH MAIN STREET (Width Varies) O'FALLON, MO RETAINING WALL COVER SHEET DESIGNED: DRAWN: DESIGN ENGINEER: AMR BRIAN SCHALLER JOB NO.: 25-201 SHEET: COVER



STRUCTURAL DESIGN HEREIN REPRESENTS A FINISHED STRUCTURE THE GENERAL CONTRACTOR/OWNER SHALL PROVIDE ALL INTERIM BRACING, SHORING, INTERIM DRAINAGE PROVISIONS, DRAINAGE DIVERSION AND EROSION PROTECTION REQUIRED UNTIL FINAL CAPPING, PAVING, CURBING AND COMPLETION OF FINAL STORM DRAIN SYSTEM IS COMPLETE.





ROSCH ENGINEERING 18390 WINGS CORPORATE DRIVE CHESTERFIELD, MO 6300/ PHONE: 636-519-7770 CHESTERFIELD, MO 63005

ROSCH FAX: 636-532-7773

MO CERTIFICATE OF AUTHORITY #E-20 | 2039663

QUIK TRIP 0683

O'FALLON, MO

RETAINING WALL

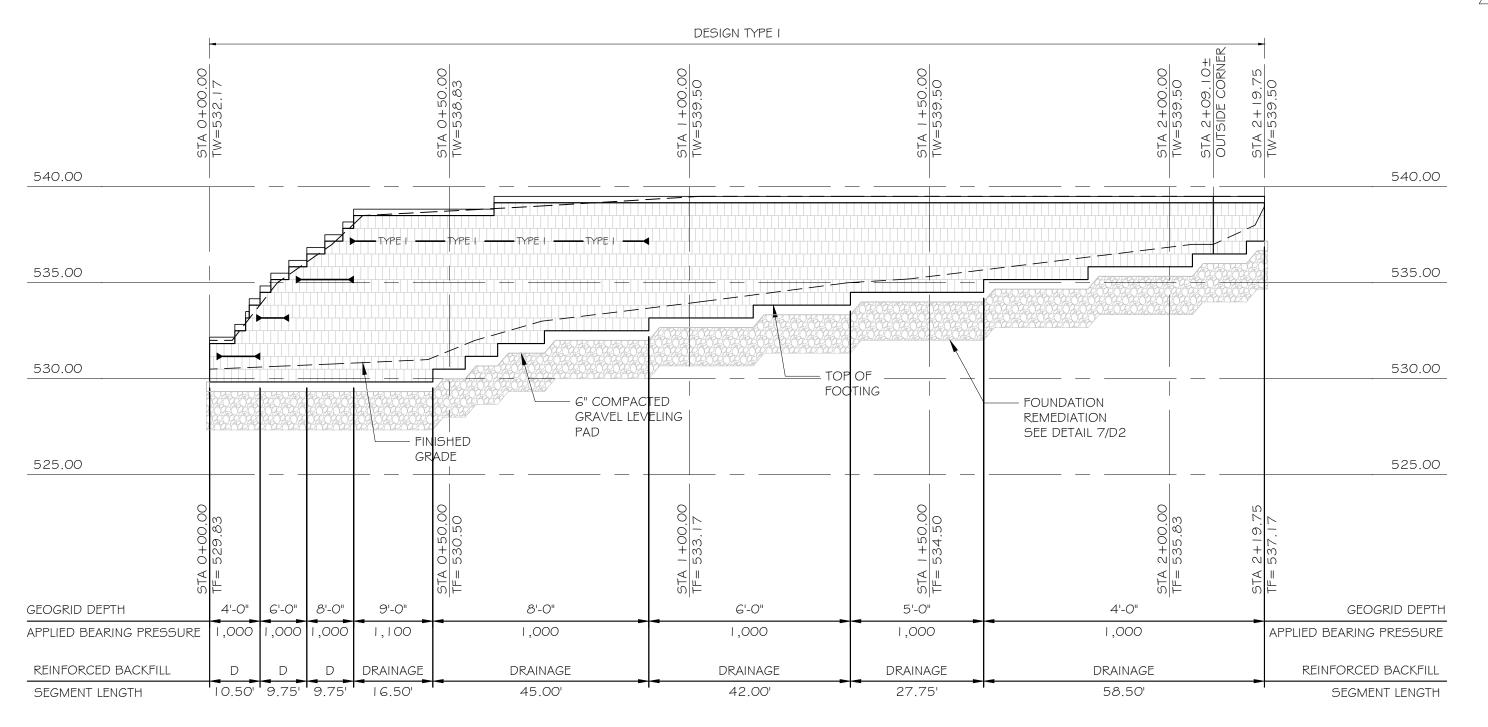
PLANS

BRIAN SCHALLER / SHEET:

4-23-2025

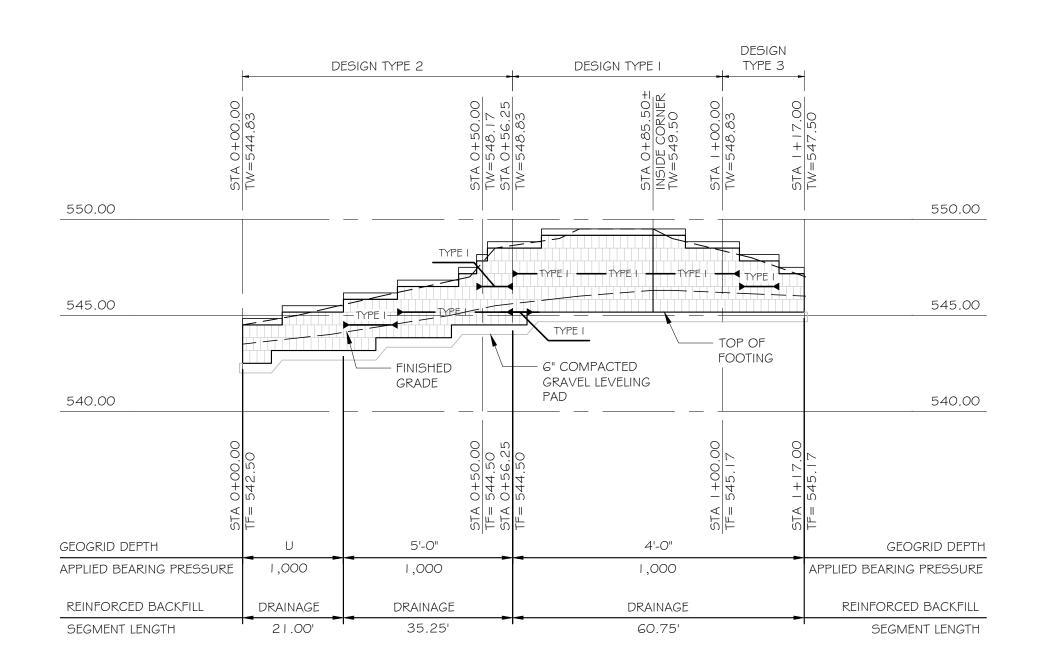
DESIGNED: DRAWN: DESIGN ENGINEER: AMR REVIEWED: 4-23-25 JOB NO.: 25-201

 \triangle



WALL I ELEVATION

SCALE: I"=20'-0" HORIZONTAL SCALE: I"=5'-0" VERTICAL



WALL 2 ELEVATION

P2 | SCALE: I"=20'-0" HORIZONTAL SCALE: I"=5'-0" VERTICAL

BLOCK PROPERTIES BLOCK TYPE **ANCHOR BLOCK STYLE DIAMOND PRO

**OR APPROVED EQUIVALENT

ASSUMED DESIGN SOIL PARAMETERS					
	DESCRIPTION -	Ф	γ	С	
		(DEGREES)	(PCF)	(PSF)	
FOUNDATION SOIL	LEAN CLAY	22	120	100	
RETAINED SOIL	LEAN CLAY	22	120	N/A	
REINFORCED BACKFILL	DRAINAGE*	38	105	N/A	
*CEE NOTEC FOR ARRITONIAL INFORMATION					

*SEE NOTES FOR ADDITIONAL INFORMATION

WALL ELEVATION NOTES:

- I. GEOGRID DEPTH IS MEASURED FROM THE FRONT FACE OF BLOCK 2. SEGMENT LENGTH IS THE DISTANCE BETWEEN GRID DEPTH TRANSITIONS 3. REINFORCED BACKFILL
- 3.1. (D)RAINAGE ROCK, SEE DETAIL 1/D1 3.2. (U)NREINFORCED, SEE DETAIL 2/D I
- 4. APPLIED BEARING PRESSURE IS IN PSF

THE EDGE OF THE FENCE FOOTING CLOSEST TO WALL I MUST BE AT LEAST 36" AWAY FROM THE FACE OF THE WALL CAP.

DESIGN TYPE I

WALL PROPERTIES			
BACKSLOPE	- /-	H:V DEGREES	
TOESLOPE		H:V DEGREES	
BATTER	7.13	DEGREES	
SETBACK	1.0	INCHES	
SURCHARGE			
DEAD LOAD	0	PSF	
LIVE LOAD	100	PSF	

DESIGN TYPE 3

	WALL PROPERTIES				
5	BACKSLOPE	6:1 9.46	H:V DEGREES		
	TOESLOPE	- / -	H:V DEGREES		
	BATTER	7.13	DEGREES		
	SETBACK	1.0	INCHES		
SURCHARG		GE			
	DEAD LOAD	0	PSF		
	LIVE LOAD	60	PSF		

PER GEOTECHNICAL REPORT, DATED 4/9/2025 PREPARED BY SCI ENGINEERING, INC. EXISTING FAT CLAY IS TO BE REMEDIATED TO A MINIMUM DEPTH OF 2 FEET BELOW THE BOTTOM OF THE WALL AND REINFORCED ZONE. OVEREXCAVATION SHOULD BE BACKFILLED WITH COMPACTED LOW PLASTIC SOIL OR 1-INCH MINUS CRUSHED ROCK. SEE DETAIL 8/D2.

DESIGN TYPE 2

WALL PROPERTIES					
ACKSLOPE	3:1	H:V DEGREES			
OESLOPE	- / -	H:V DEGREES			
ATTER	7.13	DEGREES			
ETBACK	1.0	INCHES			
SURCHARGE					
EAD LOAD	0	PSF			
IVE LOAD	25	PSF			

STORM PIPES, STORM STRUCTURES, LIGHT POLES BASES, ETC ARE SHOWN FOR INFORMATIONAL PURPOSES ONLY AND ARE BASED ON THE INFORMATION SHOWN ON THE PROJECT CIVIL PLANS REFERENCED IN GENERAL NOTE 1.4. REFER TO CURRENT PROJECT CIVIL PLANS FOR ALL SPECIFIC INFORMATION INCLUDING BUT NOT LIMITED TO SIZE AND LOCATION.

STRUCTURAL DESIGN HEREIN REPRESENTS A FINISHED STRUCTURE THE GENERAL CONTRACTOR/OWNER SHALL PROVIDE ALL INTERIM BRACING, SHORING, INTERIM DRAINAGE PROVISIONS, DRAINAGE DIVERSION AND EROSION PROTECTION REQUIRED UNTIL FINAL CAPPING, PAVING, CURBING AND COMPLETION OF FINAL STORM DRAIN SYSTEM IS COMPLETE.

<u>\</u> 2	7-11-25	NO CHANGES THIS SHEET
	6-18-25	DESIGN REVISION
REV	DATE	DESCRIPTION



ROSCH ENGINEERING 18390 WINGS CORPORATE DRIVE CHESTERFIELD, MO 63005 PHONE: 636-519-7770

ROSCH FAX: 636-532-7773 MO CERTIFICATE OF AUTHORITY #E-20 | 2039663

QUIK TRIP 0683

O'FALLON, MO

RETAINING WALL ELEVATIONS



REVIEWED: 4-23-25 JOB NO.: 25-201

DESIGN ENGINEER: AMR

SHEET:

DESIGNED:

4-23-2025

GENERAL NOTES: RETAINING WALL DESIGN: 1.1. STRUCTURAL DESIGN HEREIN REPRESENTS A FINISHED STRUCTURE. THE GENERAL CONTRACTOR/OWNER SHALL PROVIDE ALL INTERIM BRACING, SHORING, INTERIM DRAINAGE PROVISIONS, DRAINAGE DIVERSION AND EROSION PROTECTION REQUIRED UNTIL FINAL CAPPING, PAVING, CURBING AND COMPLETION OF FINAL STORM DRAIN SYSTEM I.I.I. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR/OWNER TO ENSURE THAT THE FINISHED SITE DRAINAGE IS DIRECTED AWAY FROM THE RETAINING WALL SYSTEM FROM ADJACENT CONSTRUCTION AREAS IS NOT ALLOWED TO ENTER THE RETAINING WALL AREA OF THE CONSTRUCTION SITE. 1.2. THE PROJECT GEOTECHNICAL ENGINEER SHALL REVIEW THESE DRAWINGS TO CONFIRM THE ASSUMPTIONS MEET THE INTENT OF THE GEOTECHNICAL REPORT. ADDITIONALLY, THE PROJECT GEOTECHNICAL ENGINEER SHALL REVIEW 6.4. LEVELING PAD SHALL BE CONSTRUCTED TO INSURE FULL BEARING OF RETAINING WALL UNITS. GLOBAL STABILITY OF THE RETAINING WALLS AND PROVIDE RECOMMENDATIONS AS NEEDED. 1.3. THE DESIGN OF THE SEGMENTAL RETAINING WALLS IS IN ACCORDANCE WITH NCMA DESIGN MANUAL FOR SEGMENTAL RETAINING WALLS 3RD EDITION AND NCMA'S SEGMENTAL RETAINING WALLS BEST PRACTICES GUIDE AND INCLUDES EXTERNAL STABILITY; SLIDING AND OVERTURNING OF THE REINFORCED MASS, AND INTERNAL STABILITY; PULLOUT, CONNECTION STRENGTH AND TENSILE STRENGTH OF THE GEOGRID AS WELL AS FACIAL STABILITY OF THE FACE UNITS. THE APPLIED BEARING PRESSURES ARE LISTED ON THE INDIVIDUAL WALL PROFILES. I.4. SCOPE OF DESIGN SERVICES ARE LIMITED TO THOSE DEFINED FOR "SRW (RETAINING WALL) ENGINEER" IN THE NCMA BEST PRACTICES MANUAL (2017) AND NCMA TEK 15-03A. ALL OTHER ENGINEERING SERVICES ARE EXCLUDED. 1.5. THE DESIGN OF THE SEGMENTAL RETAINING WALLS IS BASED ON THE FOLLOWING DOCUMENTS

DRAWING SP5 DATED 5/14/2024 PREPARED BY CIVIL DESIGN, INC.

 $/_{
m I}ackslash$ DRAWING C I IO DATED TI/24/2024 PREPARED BY CIVIL DESIGN, INC

THESE PLANS AS WELL AS THE FOLLOWING CRITERIA:

SURCHARGE LOADING = SEE WALL ELEVATION(S)

SEISMIC ACCELERATION = N/A

HYDROSTATIC LOADING = NONE

/2\\SCI_NO. 2025-0320.10

MATERIAL PROPERTIES:

SIEVE SIZE

¾ INCH

NO. 40

NO. 200

SIEVE SIZE

NO. 4

NO. 40

NO. 200

AS FOLLOWS:

EXCAVATION

NO. 4

 $^{\prime}$ GEOTECHNICAL REPORT DATED 4/25/2025 PREPARED BY SCI ENGINEERING, INC. $^{\circ}$

SETTLEMENT ANALYSIS IS OUTSIDE OF ROSCH ENGINEERING'S SCOPE OF WORK.

FOLLOWING GRADATION AS DETERMINED IN ACCORDANCE WITH ASTM D 422:

PERCENT PASSING

75-100

PERCENT PASSING

20-100

0-60

0-15

SF35 BY SYNTEEN TECHNICAL FABRICS, INC.

SHALL TAKE PRECAUTIONS TO MINIMIZE OVER-EXCAVATION.

MIRAGRID 3XT AS MANUFACTURED BY TENCATE GEOSYNTHETICS

MINIMUM FLOW RATE OF 140 GPM/FT2 WHEN TESTED ACCORDING TO ASTM D 4491.

3.7. DRAINAGE PIPE SHALL BE A 4"Ø PERFORATED OR SLOTTED PVC OR CORRUGATED HDPE PIPE.

3.8. DRAINAGE PIPE SHALL BE MANUFACTURED IN ACCORDANCE WITH ASTM F 405 OR ASTM F 758.

3.9. CONSTRUCTION ADHESIVE SHALL BE EXTERIOR GRADE ADHESIVE AS RECOMMENDED BY THE SEGMENTAL

HP200 AS MANUFACTURED BY GEOSTAR TECHNOLOGIES, LLC.

SGU 60 BY STRATA SYSTEMS, INC.

CONCRETE WALL UNIT MANUFACTURER.

0-60

0-50

0-5

I.G. THE DESIGN OF THE SEGMENTAL RETAINING WALL IS BASED ON THE INDIVIDUAL SOIL PROPERTIES AS LISTED WITHIN

GROUND WATER LOCATION = 2H/3 BELOW THE TOP OF LEVEL PAD (WHERE H = HEIGHT OF WALL)

2.1. SEGMENTAL RETAINING WALLS ARE FLEXIBLE MASSES THAT CAN TOLERATE MINOR SETTLEMENT. SETTLEMENT

REQUIRED, A SETTLEMENT ANALYSIS SHOULD BE PERFORMED BY THE PROJECT GEOTECHNICAL ENGINEER.

3.1. SEGMENTAL CONCRETE WALL UNITS SHALL BE MANUFACTURED IN ACCORDANCE WITH ASTM C1372 HAVING A

UNITS SHALL BE SOUND AND FREE OF CRACKS OR OTHER DEFECTS THAT WOULD INTERFERE WITH THE PROPER

PLACING OF THE UNIT OR SIGNIFICANTLY IMPAIR THE STRENGTH OR PERFORMANCE OF THE CONSTRUCTION.

3.2. DRAINAGE ROCK SHALL BE A CLEAN CRUSHED STONE OR GRANULAR FILL SUCH AS I" CLEAN MEETING THE

3.3. COMPACTED ROCK SHALL BE FREE OF ORGANIC MATERIAL. THE ROCK SHALL BE A WELL GRADED GRAVEL OR

LIMESTONE MEETING THE FOLLOWING GRADATION AS DETERMINED IN ACCORDANCE WITH ASTM D 422 AND A PI<6:

IO% SHALL BE RETAINED ON A NO. 4 SIEVE AND NO LESS THAN 35% SHALL PASS A NO. 200 SIEVE. MATERIAL

SPECIFICALLY FABRICATED FOR USE AS SOIL REINFORCEMENT. ACCEPTABLE GEOGRID TYPES AND MANUFACTURER

3.5. THE GEOGRID SHALL BE A HIGH DENSITY POLYETHYLENE EXPANDED SHEET OR POLYESTER WOVEN FIBER MATERIAL

3.6. GEOTEXTILE FILTER FABRIC SHALL BE A NONWOVEN GEOTEXTILE COMPOSED OF POLYPROPYLENE FIBERS WITH A

WITH A USC DESIGNATION OF ML, CL, OR OL ARE ACCEPTABLE FOR USE AS LOW PERMEABLE SOIL.

- BASE LEVELING PAD INSTALLATION: 6.1. LEVELING PAD SHALL BE PLACED AS SHOWN ON THE DRAWINGS AND CONSIST OF EITHER: LEAN CONCRETE (2,000 PSI) - 6" MINIMUM THICK WELL GRADED I" GRAVEL OR DRAINAGE ROCK WITH FINES - 6" MINIMUM THICK
- 1.1.2. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR/OWNER TO ENSURE THAT THE SURFACE WATER RUNOFF 6.2. SAND OR GRAVEL BASE SHALL BE COMPACTED WITH 3 PASSES OF A VIBRATORY COMPACTOR TO PROVIDE A FIRM, LEVEL BEARING PAD.
 - 6.3. LEAN CONCRETE SHALL CURE A MINIMUM OF 12 HOURS PRIOR TO UNIT PLACEMENT

 - 7.1. THE FIRST COURSE OF SEGMENTAL CONCRETE WALL UNITS SHALL BE PLACED ON THE LEVELING PAD AND CHECKED FOR LEVEL, ALIGNMENT, AND FULL CONTACT WITH BASE.
 - 7.2. UNITS SHALL BE PLACED SIDE BY SIDE FOR FULL LENGTH OF WALL. ALIGNMENT SHALL BE DONE BY MEANS OF A STRING LINE OR OFFSET MEASUREMENT FROM BASE LINE.
 - 7.3. PLACE DRAINAGE AGGREGATE WITHIN THE BLOCK CORES DIRECTLY BEHIND AND BETWEEN THE UNITS AS SHOWN IN DETAILS. WHERE THE REINFORCED BACKFILL IS COMPACTED ROCK, PLACE REINFORCED BACKFILL DIRECTLY AGAINST DRAINAGE FILL. CONSOLIDATE DRAINAGE AGGREGATE WITH 2 PASSES OF A VIBRATORY COMPACTOR. COMPACTION TESTING OF DRAINAGE AGGREGATE IS NOT REQUIRED. EXCESS MATERIAL SHALL BE REMOVED FROM TOP OF UNITS PRIOR TO INSTALLATION OF NEXT COURSE.
 - 7.4. LAY UP EACH COURSE INSURING POSITIVE CONTACT BETWEEN PREVIOUS COURSE IS ACHIEVED.
 - 8. GEOGRID INSTALLATION: 8.1. GEOGRID SHALL BE LAID AT THE PROPER ELEVATION AND ORIENTATION AS SHOWN ON THE DRAWINGS.
 - 8.2. THE GEOGRID REINFORCEMENT SHALL BE LAID HORIZONTALLY ON LEVEL, COMPACTED BACKFILL, AND EMBEDDED IN THE BLOCK.
 - 8.3. PLACE GEOGRID ON CONCRETE WALL UNITS, PLACE THE NEXT COURSE OF UNITS, PLACE THE DRAINAGE FILL, PULL GEOGRID TIGHT PRIOR TO BACKFILLING.
 - 8.4. CORRECT ORIENTATION OF THE GEOGRID SHALL BE VERIFIED.
 - 8.5. 3" OF REINFORCED BACKFILL SHALL BE PLACED BETWEEN ALL LOCATIONS OF OVERLAPPING GEOGRID.
 - BACKFILL PLACEMENT:
- SENSITIVE RIGID MASSES FOUNDED ON OR ABOVE THE SEGMENTAL RETAINING WALL SHOULD BE REVIEWED AND, IF 9.1. COMPACTED ROCK SHALL BE PLACED IN 8" MAXIMUM LIFTS AND COMPACTED TO A MINIMUM 95% OF STANDARD PROCTOR DENSITY PER ASTM 698 UNLESS NOTED OTHERWISE ON THE INDIVIDUAL WALL PROFILES.
 - 9.2. DRAINAGE ROCK SHALL BE PLACED IN 24" MAXIMUM LIFTS AND COMPACTED WITH A MINIMUM OF 2 PASSES OF A VIBRATORY COMPACTOR. FIELD DENSITY TESTING WILL NOT BE REQUIRED FOR DRAINAGE ROCK.
- MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 3,000 PSI AND A MAXIMUM MOISTURE ABSORPTION OF 8%. ALL 9.3. ONLY LIGHTWEIGHT HAND-OPERATED COMPACTION EQUIPMENT SHALL BE USED WITHIN 3' OF THE BACK FACE OF BLOCK.
 - 9.4. REINFORCED BACKFILL SHALL BE PLACED, SPREAD, AND COMPACTED IN SUCH A MANNER THAT ELIMINATES THE
 - DEVELOPMENT OF WRINKLES AND/OR MOVEMENT OF THE GEOGRID.
 - 9.5. REINFORCED BACKFILL SHALL BE PLACED AND COMPACTED FROM THE BACK OF THE WALL REARWARD INTO THE EMBANKMENT TO INSURE THAT THE GEOGRID REMAINS TIGHT.
 - 9.6. TRACKED CONSTRUCTION EQUIPMENT SHALL NOT BE OPERATED DIRECTLY ON THE GEOGRID. A MINIMUM BACKFILL THICKNESS OF 6" SHALL BE MAINTAINED TO OPERATE TRACKED VEHICLES OVER THE GEOGRID. TURNING OF TRACKED CONSTRUCTION EQUIPMENT SHALL BE KEPT TO A MINIMUM TO PREVENT TRACKS FROM DISPLACING THE FILL AND DAMAGING THE GEOGRID.
 - 9.7. AT THE END OF EACH DAY'S OPERATION, SLOPE THE LAST LEVEL OF COMPACTED BACKFILL AWAY FROM THE INTERIOR (CONCEALED) FACE OF THE WALL TO DIRECT SURFACE WATER RUNOFF FROM THE WALL FACE. 9.7.1. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO ENSURE THE FINISHED SITE DRAINAGE IS DIRECTED AWAY FROM THE RETAINING WALL SYSTEM.
 - IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO ENSURE THE SURFACE WATER RUNOFF FROM ADJACENT CONSTRUCTION AREAS IS NOT ALLOWED TO ENTER THE RETAINING WALL AREA OF THE CONSTRUCTION SITE.

3.4. LOW PERMEABLE SOIL SHALL CONSIST OF MATERIAL HAVING A MINIMUM PLASTICITY INDEX OF 10. NO MORE THAN 10. DRAIN PIPE INSTALLATION:

- I O. I . DRAINAGE COLLECTION PIPES SHALL BE INSTALLED TO MAINTAIN GRAVITY FLOW OF WATER OUTSIDE OF THE REINFORCED SOIL ZONE. THE DRAINAGE COLLECTION PIPE SHOULD CONNECT INTO A STORM SEWER MANHOLE OR DAYLIGHT THROUGH THE FACE OF THE WALL.
- CAP INSTALLATION II.I.CAP UNITS SHALL BE ADHERED TO THE TOP UNITS USING MANUFACTURER SUPPLIED ADHESIVE BY PLACING TWO I/4" BEADS OF ADHESIVE ON EACH UNIT ALONG THE ENTIRE LENGTH OF THE WALL. PRESS THE CAP UNITS FIRMLY INTO THE ADHESIVE AND ALLOW TO CURE.

12. FIELD QUALITY CONTROL:

- I 2. I . THE OWNER OR OWNER'S REPRESENTATIVE IS RESPONSIBLE FOR ENGAGING THE SERVICES OF AN INDEPENDENT THIRD PARTY INSPECTOR TO OBSERVE AND VERIFY ALL SOIL PROPERTIES AS WELL AS VERIFY CORRECT INSTALLATION OF ALL SYSTEM COMPONENTS TO MEET THE REQUIREMENTS OF THESE GENERAL NOTES AND DRAWINGS.
- I 2.2. TESTING METHODS. FREQUENCY AND VERIFICATION OF MATERIAL SPECIFICATIONS SHALL BE THE RESPONSIBILITY OF THE INDEPENDENT THIRD PARTY INSPECTOR. AT A MINIMUM, COMPACTION TESTING SHALL INCLUDE ONE COMPACTION TEST FOR EACH LIFT BUT NOT LESS THAN ONE TEST FOR EVERY 50 CUBIC YARDS OF COMPACTED FILL AT A SPACING NOT TO EXCEED 100'.

13. ABBREVIATIONS:

TYP

UNO

- FINISHED GRADE EXTERIOR FINISHED GRADE INTERIOR FLOW LINE MAXIMUM
- ON CENTER PROPERTY LINE STA STATION

UNLESS NOTED OTHERWISE

IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR.

FOUNDATION SOIL PREPARATION:

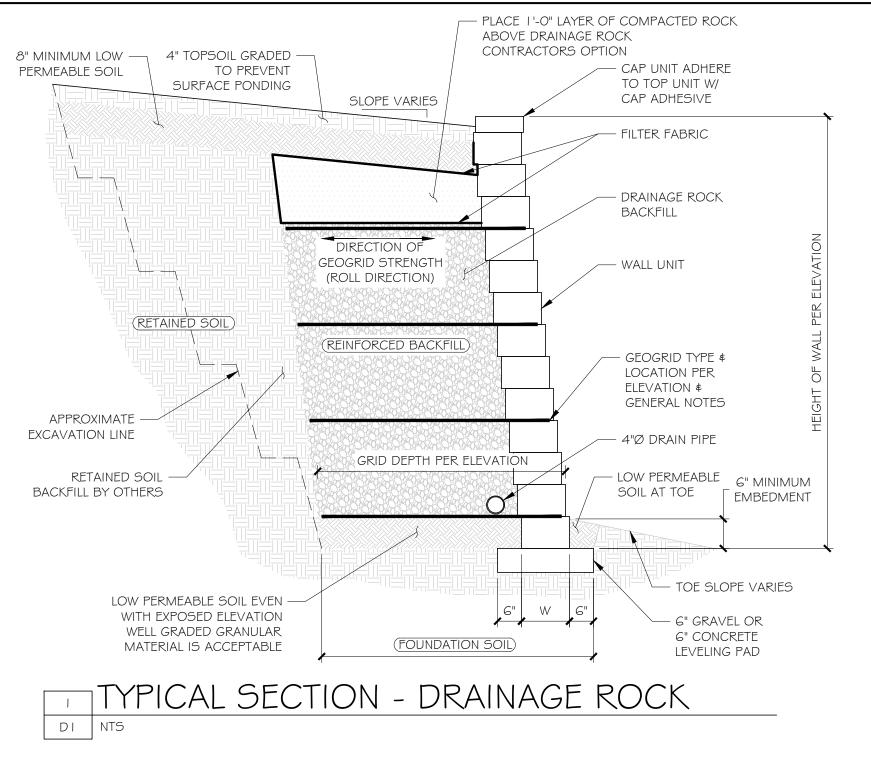
5.1. FOLLOWING EXCAVATION FOR THE LEVELING PAD AND THE REINFORCED SOIL ZONE, FOUNDATION SOIL SHALL BE EXAMINED BY THE OWNER'S GEOTECHNICAL ENGINEER TO ASSURE THE ACTUAL FOUNDATION SOIL STRENGTH MEETS OR EXCEEDS THE REQUIRED BEARING STRENGTH. SOIL NOT MEETING THE REQUIRED STRENGTH SHALL BE REMOVED AND REPLACED WITH SOIL MEETING THE DESIGN CRITERIA, AS DIRECTED BY THE OWNER'S GEOTECHNICAL ENGINEER.

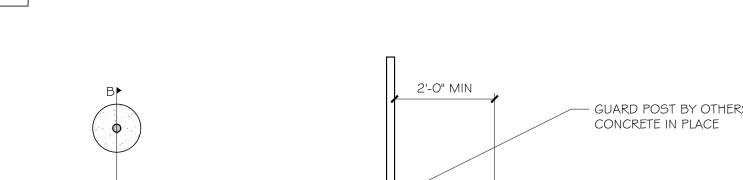
4.1. THE CONTRACTOR SHALL EXCAVATE TO THE LINES AND GRADES SHOWN ON THE PLANS. THE GENERAL CONTRACTOR

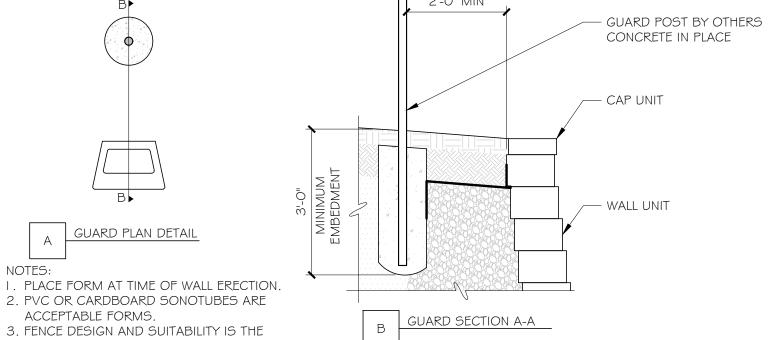
4.2. EXCAVATION SUPPORT, INCLUDING THE STABILITY OF THE EXCAVATION AND ITS INFLUENCE ON ADJACENT PROPERTY

- 5.2. IF HIGH PLASTIC SOILS ARE ENCOUNTERED IN THE FOUNDATION ZONE OF ANY RETAINING WALL, IT SHALL BE REMEDIATED SIMILAR TO ANY BUILDING FOUNDATION, AS DIRECTED BY THE PROJECT GEOTECHNICAL ENGINEER.
- 5.3. FOUNDATION SOIL IS DEFINED AS THE SOIL UNDER THE SEGMENTAL RETAINING WALL VOLUME, EXTENDING FROM THE TOE OF THE LEVELING PAD TO THE BACK OF THE REINFORCED MASS

ROSCH ENGINEERING HAS PERFORMED DESIGN CALCULATIONS MINIMUM BASED ON THE DESIGN CRITERIA, ASSUMED SOIL PARAMETERS, AND KNOWN LOADING CONDITIONS AS LISTED IN THESE DRAWINGS. THE OWNERS REPRESENTATIVE, INDEPENDENT THIRD PARTY SPECIAL INSPECTOR AND INSTALLER SHALL NOTIFY ROSCH ENGINEERING OF TOP OF FOOTING ELEVATION ANY CHANGES OR DIFFERENCES IN ACTUAL SITE CONDITIONS WHICH TOP OF WALL ELEVATION VARY FROM THOSE LISTED, PRIOR TO CONSTRUCTING THE WALL. TYPICAL







- DRAINAGE

AGGREGATE

DRAINAGE AGGREGATE

4"Ø DRAIN PIPE DAYLIGHT

AT LOW POINT OF WALL

- LOW PERMEABLE

SOIL EVEN WITH

EXPOSED ELEVATION

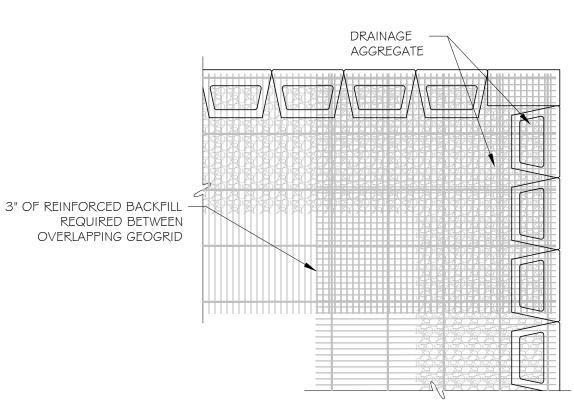
LOW PERMEABLE

SOIL EVEN WITH

EXPOSED ELEVATION

RESPONSIBILITY OF OTHERS TYPICAL POST AT UNREINFORCED WALL

TYPICAL POST AT REINFORCED WALL DI NTS



4" TOPSOIL GRADED -

SURFACE PONDING

8" MINIMUM LOW

PERMEABLE SOIL

LOW PERMEABLE

SOIL EVEN WITH

GUARD PLAN DETAIL

I. PLACE FORM AT TIME OF WALL ERECTION.

2. PVC OR CARDBOARD SONOTUBES ARE

3. FENCE DESIGN AND SUITABILITY IS THE

4. THIS DETAIL IS SUITABLE FOR OPEN TYPE

FENCE ONLY. NOT SUITABLE FOR CLOSED

RESPONSIBILITY OF OTHERS.

ACCEPTABLE FORMS

PRIVACY TYPE FENCES.

EXPOSED ELEVATION

TO PREVENT

TRUCTURAL DESIGN HEREIN REPRESENTS A FINISHED STRUCTURE HE GENERAL CONTRACTOR/OWNER SHALL PROVIDE ALL INTERIM RACING, SHORING, INTERIM DRAINAGE PROVISIONS, DRAINAGE DIVERSION AND EROSION PROTECTION REQUIRED UNTIL FINAL CAPPING, PAVING, CURBING AND COMPLETION OF FINAL STORM DRAIN SYSTEM IS COMPLETE.

GUARD SECTION A-A

- CAP UNIT ADHERE

TO TOP UNIT W/

CAP ADHESIVE

4"Ø DRAIN PIPE

– LOW PERMEABLE

- TOE SLOPE VARIES

GUARD POST BY OTHERS

CONCRETE IN PLACE

- CAP UNIT

- CUT GEOGRID

WALL UNIT

WITHIN 2" OF FORM

- 6" GRAVEL OR

6" CONCRETE

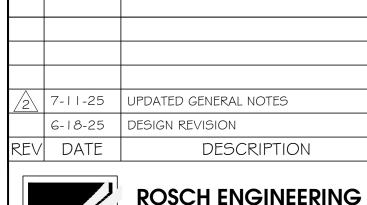
SOIL AT TOE

— FILTER FABRIC

DRAINAGE

AGGREGATE

TYPICAL SECTION - UNREINFORCED WALL



18390 WINGS CORPORATE DRIVE CHESTERFIELD, MO 63005 PHONE: 636-519-7770 **ROSCH** FAX: 636-532-7773

MO CERTIFICATE OF AUTHORITY #E-20 | 2039663

QUIK TRIP 0683 O'FALLON, MO

RETAINING WALL NOTES & DETAILS

DESIGNED: BRIAN SCHALLER E-28703

DRAWN DESIGN ENGINEER: AMF REVIEWED 4-23-2 25-201 JOB NO.:

SHEET

TYPICAL INSIDE CORNER DETAIL

TYPICAL OUTSIDE CORNER DETAIL

WALLS > 50'-0" LONG

- 4"Ø DRAIN PIPE

- DRAINAGE LATERAL

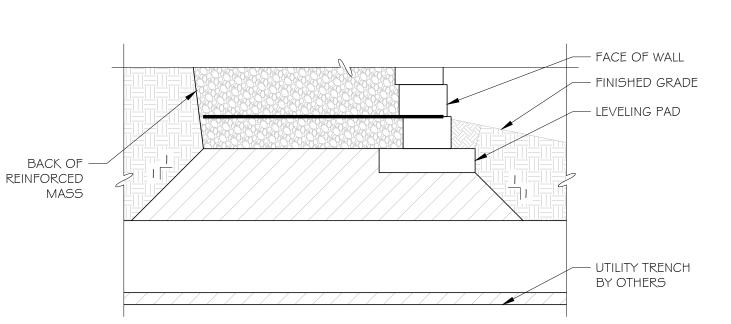
- LOW PERMEABLE

SOIL EVEN WITH

EXPOSED ELEVATION

DAYLIGHT THROUGH

WALL @ 50'-0" OC MAX



FACE OF WALL TOP OF LEVELING PAD TRENCH MATERIAL, DESIGN, AND SUITABILITY, BY OTHERS, COMPACTED AND TESTED PER GEOTECH REPORT. FLOWABLE FILL RECOMMENDED. MATERIAL SHALL MEET REQUIRED BEARING CAPACITY AS SHOWN IN THESE PLANS. UTILITY TRENCH UTILITY TRENCH CROSSING DETAIL BY OTHERS

I " FOR STRAIGHT FACE UNITS 2" FOR TRI PLANE UNITS

GUARD PLAN DETAII

EXTEND GEOGRID 4'-0" -

EACH DIRECTION

ACCEPTABLE FORMS.

3" OF REINFORCED BACKFILL -

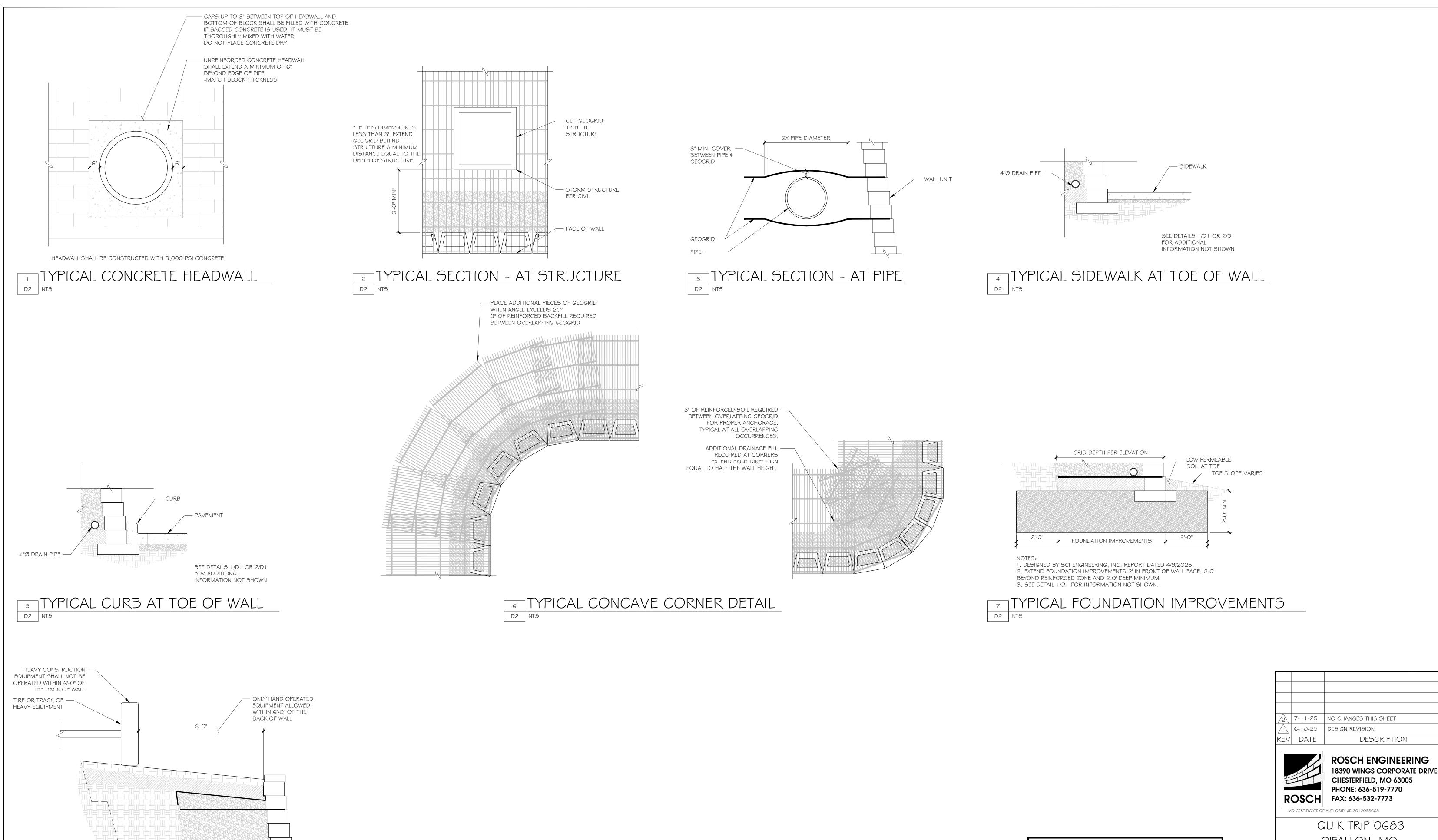
REQUIRED BETWEEN OVERLAPPING GEOGRID

> LOW PERMEABLE SOIL EVEN WITH EXPOSED ELEVATION WALLS < 50'-0" LONG

GEOGRID TYPE \$ LOCATION PER ELEVATION \$ GENERAL NOTES

GEOGRID PLACEMENT

DRAIN PIPE INSTALLATION



B HEAVY EQUIPMENT AT TOP OF WALL

SEE DETAIL I/D I FOR ADDITIONAL INFORMATION NOT SHOWN

ROSCH
FAX: 636-532-7773

QUIK TRIP 0683

O'FALLON, MO

RETAINING WALL

DETAILS

DESIGNED: BCS

DRAWN: JTM

DESIGN ENGINEER: AMR

REVIEWED: BCS

DATE: 4-23-25

JOB NO.: 25-2012

SHEET: D2

STRUCTURAL DESIGN HEREIN REPRESENTS A FINISHED STRUCTURE

THE GENERAL CONTRACTOR/OWNER SHALL PROVIDE ALL INTERIM BRACING, SHORING, INTERIM DRAINAGE PROVISIONS, DRAINAGE

DIVERSION AND EROSION PROTECTION REQUIRED UNTIL FINAL CAPPING, PAVING, CURBING AND COMPLETION OF FINAL STORM

DRAIN SYSTEM IS COMPLETE.