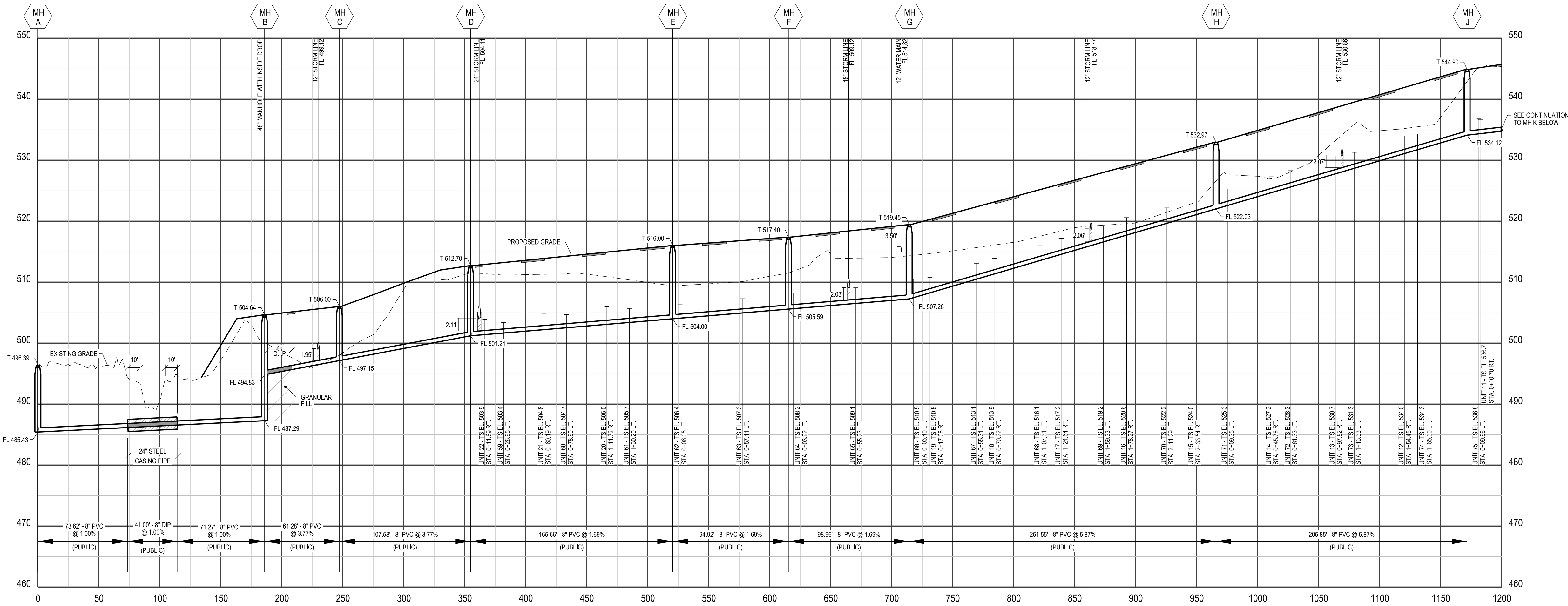


H:\CAD\23450-23499\23444\mgPhotos\mgp\overment\Plan\23444 - C15 Storm Sewer Hydraulics.dgn : Master Model 7/1/2025 10:59:37 AM Plotted by eharmon Plot Scale: 50.000000' = 1" in: Plot Driver: Volz's PDF plotter Pen Table: Volz's Plot File.tbl

15 Year-20 Minute Storm Hydraulic Calculations (Rev. 2)																						
LineNo.	LineID	Pipe Length (ft)	Pipe Size (in)	Invert Down (ft)	Invert Up (ft)	Slope (%)	n Value	Top El Down (ft)	Top El Up (ft)	HGL (Down) (ft)	HGL (Up) (ft)	Juntion Loss	Velocity Average (ft/s)	Deflec. Angle (Deg)	Energy Loss (ft)	Known Q (cfs)	Flow Rate (cfs)	Minor Loss (ft)	Capacity (Full) (cfs)	Free- board (ft)		
1	FE 01 to OS 03	68	30	492.00	493.02	1.50	0.013	494.50	499.90	494.27	495.29	1.00	10.22	-9.00	0.81	47.89	47.89	1.62	50.23	4.61		
2	FE 04 to MH 05	66	30	498.00	498.66	1.00	0.013	500.50	505.01	499.84	500.50	0.75	7.83	51.40	0.45	0.00	30.30	0.71	41.01	4.51		
3	MH 05 to 2CI 06	132	30	498.66	504.65	4.54	0.013	505.01	511.59	501.21	506.44	1.50	6.75	-34.70	0.76	3.22	28.77	1.36	87.36	5.15		
4	2CI 06 to MH 07	182	24	504.65	506.60	1.07	0.013	511.59	515.81	507.80	508.88	0.15	5.54	86.30	1.08	0.00	17.39	0.07	23.41	6.93		
5	MH 07 to CI 08	78	24	506.60	507.43	1.06	0.013	515.81	516.66	508.95	509.38	0.70	5.55	-9.10	0.44	1.18	17.39	0.34	23.33	7.28		
6	CI 08 to MH 09	46	24	507.43	507.93	1.09	0.013	516.66	518.44	509.72	509.93	1.00	5.16	-14.60	0.23	0.00	16.21	0.41	23.58	8.51		
7	MH 09 to MH 10	168	24	507.93	509.73	1.07	0.013	518.44	521.80	510.34	510.98	1.00	5.02	-10.50	0.76	0.00	12.53	0.57	23.41	10.82		
8	MH 10 to CI 11	164	18	513.51	519.37	3.57	0.013	521.80	524.58	514.20	520.49	1.50	8.45	-91.10	3.58	1.12	8.63	0.87	19.85	4.09		
9	CI 11 to CI 12	208	15	519.37	526.77	3.56	0.013	524.58	531.98	521.36	527.78	1.50	5.61	0.00	2.06	0.65	6.39	0.84	12.18	4.20		
10	CI 12 to MH 13	52	15	526.77	527.72	1.83	0.013	531.98	535.44	528.62	528.92	1.00	4.18	0.00	0.30	0.00	5.09	0.28	8.73	6.52		
11	MH 13 to AI 14	132	12	527.72	530.12	1.82	0.013	535.44	534.62	529.19	530.74	1.50	3.42	90.00	0.71	0.71	2.11	0.40	4.80	3.88		
12	AI 14 to AI 15	156	12	530.12	533.24	2.00	0.013	534.62	540.71	531.14	533.74	0.50	2.67	-90.00	0.60	0.72	1.40	0.10	5.04	6.97		
13	AI 15 to AI 16	190	12	536.61	543.47	3.61	0.013	540.71	547.90	536.83	543.82	1.00	4.13	-3.30	3.86	0.68	0.68	0.12	6.77	4.08		
14	MH 13 to CI 17	65	12	530.93	532.88	3.00	0.013	535.44	537.84	531.42	533.61	1.50	6.29	0.00	1.25	0.56	2.98	0.55	6.17	4.23		
15	CI 17 to CI 18A	36	12	532.88	533.59	1.97	0.013	537.84	537.84	534.16	534.25	1.50	3.73	-90.00	0.22	0.46	2.42	0.45	5.00	3.59		
16	CI 18A to CI 188	91	12	533.59	537.90	4.74	0.013	537.84	542.31	534.70	538.49	1.25	3.26	90.00	0.45	0.63	1.96	0.32	7.75	3.82		
17	CI 188 to CI 18C	37	12	537.90	538.63	1.97	0.013	542.31	542.70	538.81	539.12	1.00	2.63	75.00	0.13	1.33	1.33	0.19	5.00	3.58		
18	CI 12 to CI 19	36	12	526.77	527.48	1.97	0.013	531.98	531.98	528.62	528.63	1.00	0.83	-90.00	0.01	0.65	0.65	0.01	5.00	3.35		
19	CI 11 to CI 20	36	12	519.37	520.08	1.97	0.013	524.58	524.58	521.36	521.39	1.00	1.43	-90.00	0.04	1.12	1.12	0.03	5.00	3.19		
20	MH 10 to CI 21	39	18	509.73	510.18	1.15	0.013	521.80	521.51	511.55	511.60	1.50	2.23	0.70	0.05	0.29	3.90	0.12	11.28	9.91		
21	CI 21 to CI 22	36	18	510.18	510.59	1.14	0.013	521.51	521.51	511.72	511.73	1.50	2.27	-91.90	0.05	0.68	3.61	0.15	11.21	9.78		
22	CI 22 to MH 23	71	15	510.59	511.30	1.00	0.013	521.51	522.95	511.88	511.99	0.85	3.31	78.10	0.29	0.00	2.93	0.24	6.46	10.96		
23	MH 23 to AI 24	118	15	511.30	512.48	1.00	0.013	522.95	523.29	512.22	513.17	0.50	3.63	-66.30	0.51	0.13	2.93	0.14	6.46	10.12		
24	AI 24 to 2AI 25	44	15	512.48	512.92	1.00	0.013	523.29	517.75	513.31	513.59	1.00	3.72	8.30	0.20	2.80	2.80	0.27	6.46	4.16		
25	MH 09 to CI 26	35	15	509.87	510.57	2.00	0.013	518.44	517.50	510.42	511.34	0.50	5.83	-101.30	0.46	1.69	3.68	0.17	9.13	6.16		
26	CI 26 to AI 27	157	12	510.57	513.70	1.99	0.013	517.50	518.20	511.51	514.30	0.50	3.33	-0.40	0.76	1.04	1.99	0.13	5.03	3.90		
27	AI 27 to AI 28	176	12	513.70	520.47	3.85	0.013	518.20	524.97	514.43	520.88	1.00	2.33	0.00	0.57	0.95	0.95	0.15	6.98	4.09		
28	2CI 06 to 2CI 29	35	24	504.65	505.15	1.43	0.013	511.59	511.59	507.80	507.85	0.50	2.60	-3.30	0.05	2.67	8.16	0.05	27.03	3.74		
29	2CI 29 to AI 30	125	24	505.15	506.92	1.42	0.013	511.59	511.58	507.90	507.94	1.50	2.57	10.00	0.17	1.87	5.49	0.27	26.91	3.64		
30	AI 30 to AI 31	136	12	506.92	515.77	6.51	0.013	511.58	523.23	508.21	516.35	1.10	3.17	-48.20	0.65	0.85	1.87	0.27	9.08	6.88		
31	AI 31 to AI 32	203	12	515.77	528.98	6.51	0.013	523.23	534.52	516.62	529.41	1.00	2.31	41.60	0.65	1.02	1.02	0.16	9.08	5.11		
32	AI 30 to AI 33	18	12	506.92	507.27	1.94	0.013	511.58	511.58	508.21	508.25	1.00	2.23	-100.00	0.04	1.75	1.75	0.08	4.97	3.33		
33	MH 05 to AI 34	68	12	498.66	500.10	2.12	0.013	505.01	505.04	501.21	501.34	0.50	1.95	52.50	0.13	0.83	1.53	0.03	5.18	3.70		
34	AI 34 to MH 35	170	12	500.10	503.70	2.12	0.013	505.04	509.00	501.37	504.05	0.45	1.85	-2.20	0.48	0.00	0.70	0.06	5.18	4.95		
35	MH 35 to AI 36	68	12	503.70	505.14	2.12	0.013	509.00	509.64	504.11	505.49	1.00	2.56	-18.50	0.29	0.70	0.70	0.12	5.18	4.15		
36	FE 37 to AI 38	166	24	498.00	506.65	5.21	0.013	500.00	514.15	499.52	507.90	1.50	5.43	-0.50	0.78	1.00	12.40	0.84	51.63	6.25		
37	AI 38 to MH 39	105	24	506.65	508.75	2.00	0.013	514.15	518.10	508.74	509.95	0.45	4.72	89.20	0.43	0.00	11.40	0.24	31.99	8.15		
38	MH 39 to CI 40	28	18	508.75	509.31	2.00	0.013	518.10	516.72	510.18	510.60	0.70	6.80	-11.70	0.30	1.25	11.40	0.54	14.85	6.12		
39	CI 40 to CI 41	36	18	509.31	510.02	1.97	0.013	516.72	516.72	511.14	511.43	1.50	5.82	14.00	0.31	0.91	10.15	0.81	14.75	5.29		
40	CI 41 to CI 42A	152	18	510.02	518.00	5.25	0.013	516.72	523.73	512.24	519.16	1.50	5.76	-79.30	1.25	0.50	9.24	0.93	24.06	4.57		
41	CI 42A to CI 42B	124	15	518.00	524.65	5.36	0.013	523.73	531.24	520.09	525.73	1.50	6.19	5.20	1.51	0.50	7.26	0.97	14.95	5.51		
42	CI 42B to CI 43A	108	15	524.65	530.42	5.34	0.013	531.24	537.77	526.70	531.42	1.50	5.52	4.00	1.04	0.50	6.26	0.82	14.93	6.35		
43	CI 43A to CI 43B	100	12	530.42	538.73	8.31	0.013	537.77	543.69	532.24	539.61	1.50	5.75	3.50	1.42	0.64	4.35	0.83	10.27	4.08		
44	CI 43B to CI 44	203	12	538.73	543.87	2.53	0.013	543.69	549.06	540.43	544.61	1.25	4.36	4.10	1.65	1.24	3.02	0.46	5.67	4.45		
45	CI 44 to CI 45	37	12	543.87	544.60	1.97	0.013	549.06	549.10	545.07	545.17	1.00	3.08	-74.30	0.17	1.78	1.78	0.23	5.00	3.93		
46	CI 43B to CI 43C	35	12	538.73	539.44	2.03	0.013	543.69	543.69	540.43	540.44	1.00	0.88	-88.20	0.01	0.69	0.69	0.01	5.07	3.25		
47	CI 43A to CI 46	36	12	530.42	530.77	0.97	0.013	537.77	537.77	532.24	532.30	0.50	1.80	-88.20	0.06	0.50	1.41	0.03	3.51	5.47		
48	CI 46 to AI 47	138	12	530.77	532.15	1.00	0.013	537.77	536.15	532.33	532.58	1.00	2.00	0.00	0.36	0.91	0.91	0.13	3.56	3.57		
49	CI 42B to CI 42C	36	12	526.73	527.45	2.00	0.013	531.24	531.70	526.94	527.75	1.00	3.30	-75.70	0.45	0.50	0.50	0.10	5.04	3.95		
50	CI 42A to MH 48	34	12	518.00	518.35	1.03	0.013	523.73	524.54	520.09	520.14	1.00	1.88	-87.00	0.06	0.00	1.48	0.06	3.61	4.40		
51	MH 48 to CI 49	32	12	518.35	518.67	1.00	0.013	524.54	525.64	520.20	520.26	1.50	1.88	88.20	0.06	0.50	1.48	0.08	3.56	5.38		
52	CI 49 to AI 50	132	12	518.67	519.99	1.00	0.013	525.64	523.99	520.34	520.50	1.00	1.83	-87.10	0.23	0.98	0.98	0.09	3.56	3.49		

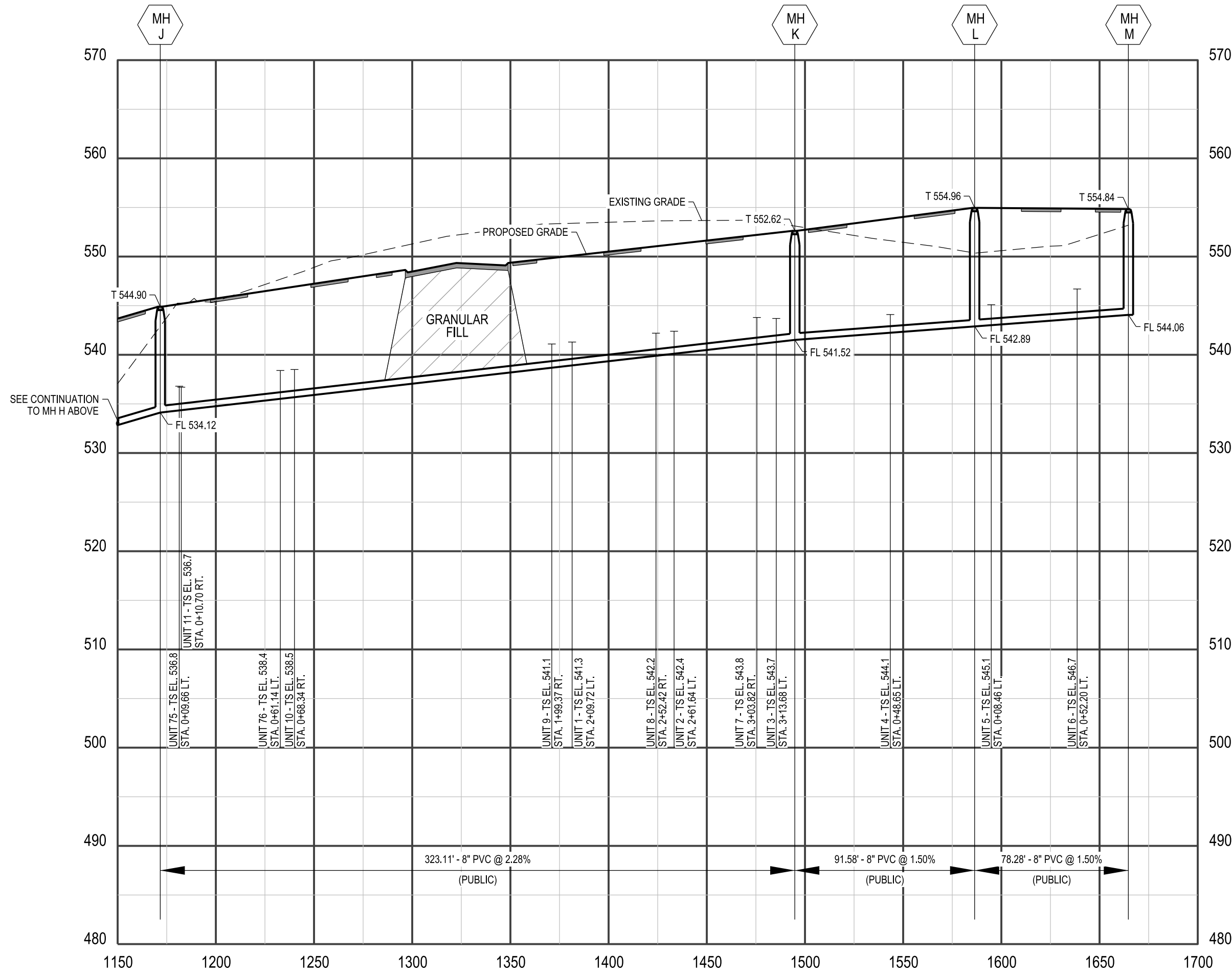
200 Year-20 Minute Storm Hydraulic Calculations (Rev. 2)																						
LineNo.	LineID	Pipe Length (ft)	Pipe Size (in)	Invert Down (ft)	Invert Up (ft)	Slope (%)	n Value	Top El Down (ft)	Top El Up (ft)	HGL (Down) (ft)	HGL (Up) (ft)	Juntion Loss	Velocity Average (ft/s)	Deflec. Angle (Deg)	Energy Loss (ft)	Known Q (cfs)	Flow Rate (cfs)	Minor Loss (ft)	Capacity (Full) (cfs)	Free-board (ft)		
1	FE 01 to OS 03	68	30	492.00	493.02	1.50	0.013	494.50	499.90	494.44	496.32	1.00	14.19	-9.00	1.84	69.44	69.44	3.11	50.23	3.58		
2	FE 04 to MH 05	66	30	498.00	498.66	1.00	0.013	500.50	505.01	500.93	501.69	0.75	8.95	51.40	0.76	0.00	43.93	0.93	41.01	3.32		
3	MH 05 to 2C1 06	132	30	498.66	504.65	4.54	0.013	505.01	511.59	502.62	506.82	1.50	8.86	-34.70	1.31	4.67	41.71	1.98	87.36	4.77		
4	2C1 06 to MH 07	182	24	504.65	506.60	1.07	0.013	511.59	515.81	508.80	511.06	0.15	8.03	86.30	2.26	0.00	25.21	0.15	23.41	4.75		
5	MH 07 to CI 08	78	24	506.60	507.43	1.06	0.013	515.81	516.66	511.21	512.18	0.70	8.03	-9.10	0.97	1.71	25.21	0.70	23.33	4.48		
6	CI 08 to MH 09	46	24	507.43	507.93	1.09	0.013	516.66	518.44	512.89	513.38	1.00	7.48	-14.60	0.50	0.00	23.50	0.87	23.58	5.06		
7	MH 09 to MH 10	168	24	507.93	509.73	1.07	0.013	518.44	521.80	514.25	515.34	1.50	5.78	-10.10	1.08	0.00	18.16	0.52	23.41	6.46		
8	MH 10 to CI 11	164	18	513.51	519.37	3.57	0.013	521.80	524.58	515.86	520.71	1.50	7.29	-9.10	2.19	1.62	12.12	1.32	19.85	3.87		
9	CI 11 to CI 12	208	15	519.37	526.77	3.56	0.013	524.58	531.98	522.02	527.94	1.50	7.66	0.00	3.99	0.94	9.26	0.14	12.18	4.04		
10	CI 12 to MH 13	52	15	526.77	527.72	1.83	0.013	531.98	535.44	529.34	530.02	1.00	6.01	0.00	0.68	0.00	7.38	0.56	8.73	5.42		
11	MH 13 to AI 14	132	12	527.72	530.12	1.82	0.013	535.44	534.62	530.59	531.56	1.50	3.90	90.00	0.98	1.03	3.06	0.35	4.80	3.06		
12	AI 14 to AI 15	156	12	530.12	533.24	2.00	0.013	534.62	540.71	531.92	533.84	0.50	3.34	-90.00	0.80	1.04	2.03	0.13	5.04	6.87		
13	AI 15 to AI 16	190	12	536.61	543.47	3.61	0.013	540.71	547.90	536.87	543.89	1.00	4.62	-3.30	3.89	0.99	0.99	0.15	6.77	4.01		
14	MH 13 to CI 17	65	12	530.93	532.88	3.00	0.013	535.44	537.84	531.55	533.76	1.50	7.20	0.00	1.40	0.81	4.32	0.82	6.17	4.08		
15	CI 17 to CI 18A	36	12	532.88	533.59	1.97	0.013	537.84	537.84	534.57	534.92	1.50	4.47	-90.00	0.35	0.67	3.51	0.47	5.00	2.92		
16	CI 18A to CI 18B	91	12	533.59	537.90	4.74	0.013	537.84	542.31	535.39	538.61	1.25	4.17	90.00	0.68	0.91	2.84	0.43	7.75	3.70		
17	CI 18B to CI 18C	37	12	537.90	538.63	1.97	0.013	542.31	542.70	539.05	539.22	1.00	3.23	75.00	0.18	1.93	1.93	0.25	5.00	3.48		
18	CI 12 to CI 19	36	12	526.77	527.48	1.97	0.013	531.98	531.98	529.34	529.37	1.00	1.20	-90.00	0.03	0.94	0.94	0.02	5.00	2.61		
19	CI 11 to CI 20	36	12	519.37	520.08	1.97	0.013	524.58	524.58	522.02	522.10	1.00	2.06	-90.00	0.08	1.62	1.62	0.07	5.00	2.48		
20	MH 10 to CI 21	39	18	509.73	510.18	1.15	0.013	521.80	521.51	515.86	515.97	1.50	3.20	0.70	0.11	0.42	5.66	0.24	11.28	5.54		
21	CI 21 to CI 22	36	18	510.18	510.59	1.14	0.013	521.51	521.51	516.21	516.30	1.50	2.97	-91.90	0.09	0.99	5.24	0.21	11.21	5.21		
22	CI 22 to MH 23	71	15	510.59	511.30	1.00	0.013	521.51	522.95	516.50	516.81	0.85	3.46	78.10	0.31	0.00	4.25	0.16	6.46	6.14		
23	MH 23 to AI 24	118	15	511.30	512.48	1.00	0.013	522.95	523.29	516.97	517.48	0.50	3.46	-66.30	0.51	0.19	4.25	0.09	6.46	5.81		
24	AI 24 to 2AI 25	44	15	512.48	512.92	1.00	0.013	523.29	517.57	517.57	517.75	1.00	3.31	8.30	0.17	4.06	4.06	0.17	6.46	0.00		
25	MH 05 to CI 26	35	15	509.87	510.57	2.00	0.013	518.44	517.50	514.25	514.49	0.50	4.35	-101.30	0.24	2.45	5.34	0.15	9.13	3.01		
26	CI 26 to AI 27	157	12	510.57	513.70	1.99	0.013	517.50	518.20	514.64	515.67	0.50	3.68	-4.00	1.03	1.51	2.89	0.11	5.03	2.53		
27	AI 27 to AI 28	176	12	513.70	520.47	3.85	0.013	518.20	524.97	515.78	520.97	1.00	2.64	0.00	0.67	1.38	1.38	0.19	6.98	4.00		
28	2C1 06 to 2C1 29	35	24	504.65	505.15	1.43	0.013	511.59	511.59	508.80	508.90	0.50	3.77	-3.30	0.10	3.87	11.83	0.11	27.03	2.69		
29	2C1 29 to AI 30	125	24	505.15	506.92	1.42	0.013	511.59	511.58	509.01	509.16	1.50	2.53	10.00	0.16	2.71	7.96	0.15	26.91	2.42		
30	AI 30 to AI 31	136	12	506.92	515.77	6.51	0.013	511.58	523.23	509.31	516.47	1.10	4.04	-48.20	0.96	1.23	2.71	0.37	9.08	6.76		
31	AI 31 to AI 32	203	12	515.77	528.98	6.51	0.013	523.23	534.52	516.83	529.50	1.00	2.75	41.60	0.81	1.48	1.48	0.20	9.08	5.02		
32	AI 30 to AI 33	18	12	506.92	507.27	1.94	0.013	511.58	511.58	509.31	509.40	1.00	3.23	-100.00	0.09	2.54	2.54	0.16	4.97	2.18		
33	MH 05 to AI 34	68	12	498.66	500.10	2.12	0.013	505.01	505.04	502.62	502.89	0.50	2.83	52.50	0.26	1.20	2.22	0.06	5.18	2.15		
34	AI 34 to MH 35	170	12	500.10	503.70	2.12	0.013	505.04	509.00	502.95	504.13	0.45	2.24	-2.20	0.55	0.00	1.02	0.07	5.18	4.87		
35	MH 35 to AI 36	68	12	503.70	505.14	2.12	0.013	509.00	509.64	504.20	505.15	1.00	2.89	-18.50	0.30	1.02	1.02	0.16	5.18	4.07		
36	FE 37 to AI 38	166	24	498.00	506.65	5.21	0.013	500.00	514.15	500.93	508.57	1.50	6.42	-0.50	1.16	1.45	18.01	1.18	51.63	6.00		
37	AI 38 to MH 39	105	24	506.65	508.75	2.00	0.013	514.15	518.10	509.33	510.19	0.45	6.05	89.20	0.65	0.00	16.56	0.33	31.99	7.91		
38	MH 39 to CI 40	28	18	508.75	509.31	2.00	0.013	518.10	516.72	510.52	511.21	0.70	9.37	-11.70	0.70	1.81	16.56	0.96	14.85	5.51		
39	CI 40 to CI 41	36	18	509.31	510.02	1.97	0.013	516.72	516.72	512.17	512.88	1.50	8.35	14.00	0.71	1.32	14.75	1.62	14.75	3.84		
40	CI 41 to CI 42A	152	18	510.02	518.00	5.25	0.013	516.72	523.73	514.51	519.37	1.50	7.78	-79.30	2.33	0.73	13.43	1.47	24.06	4.36		
41	CI 42A to CI 42B	124	15	518.00	524.65	5.36	0.013	523.73	531.24	520.84	525.85	1.50	8.66	5.20	3.09	0.73	10.55	1.78	14.95	5.39		
42	CI 42B to CI 43A	108	15	524.65	530.42	5.34	0.013	531.24	537.77	527.62	531.58	1.50	7.53	4.00	2.00	0.73	9.09	1.36	14.93	6.19		
43	CI 43A to CI 43B	100	12	530.42	538.73	8.31	0.013	537.77	543.69	532.95	539.69	1.50	8.08	3.50	2.94	0.93	6.31	1.54	10.27	4.00		
44	CI 43B to CI 44	203	12	538.73	543.87	2.53	0.013	543.69	549.06	541.23	544.75	1.25	5.78	4.10	2.92	1.80	4.38	0.70	5.67	4.31		
45	CI 44 to CI 45	37	12	543.87	544.60	1.97	0.013	549.06	549.10	545.45	545.60	1.00	3.29	-74.30	0.19	2.58	2.58	0.17	5.00	3.50		
46	CI 43B to CI 43C	35	12	538.73	539.44	2.03	0.013	543.69	543.69	541.23	541.26	1.00	1.27	-88.20	0.03	1.00	1.00	0.03	5.07	2.43		
47	CI 43A to CI 46	36	12	530.42	530.77	0.97	0.013	537.77	537.77	532.95	533.06	0.50	2.61	-88.20	0.12	0.73	2.05	0.05	3.51	4.71		
48	CI 46 to AI 47	138	12	530.77	532.15	1.00	0.013	537.77	536.15	533.12	533.31	1.00	1.68	0.00	0.19	1.32	1.32	0.04	3.56	2.84		
49	CI 42B to CI 42C	36	12	526.73	527.45	2.00	0.013	531.24	531.70	527.62	527.81	1.00	1.91	-75.70	0.10	0.73	0.73	0.13	5.04	3.89		
50	CI 42A to MH 48	34	12	518.00	518.35	1.03	0.013	523.73	524.54	520.84	520.96	1.00	2.74	-87.00	0.12	0.00	2.15	0.12	3.61	3.58		
51	MH 48 to AI 49	32	12	518.35	518.67	1.00	0.013	524.54	525.64	521.08	521.20	1.50	2.74	88.20	0.12	0.73	2.15	0.17	3.56	4.44		
52	CI 49 to AI 50	132	12	518.67	519.99	1.00	0.013	525.64	523.99	521.37	521.58	1.00	1.81	-87.10	0.21	1.42	1.42	0.05	3.56	2.41		

H:\CADD\234500-234500-234443\img\Sanitary\Improvement\Plan\23444 - C16 Sanitary Profiles - Sheet 1.dgn - Master Model 7/1/2025 10:39:49 AM Plotted by: eherman Plot Scale: 50.000000 1/4" = 1' Plot Driver: Volz15 PDF.plt c16 Pen Table: Volz15 Plot Full.bti



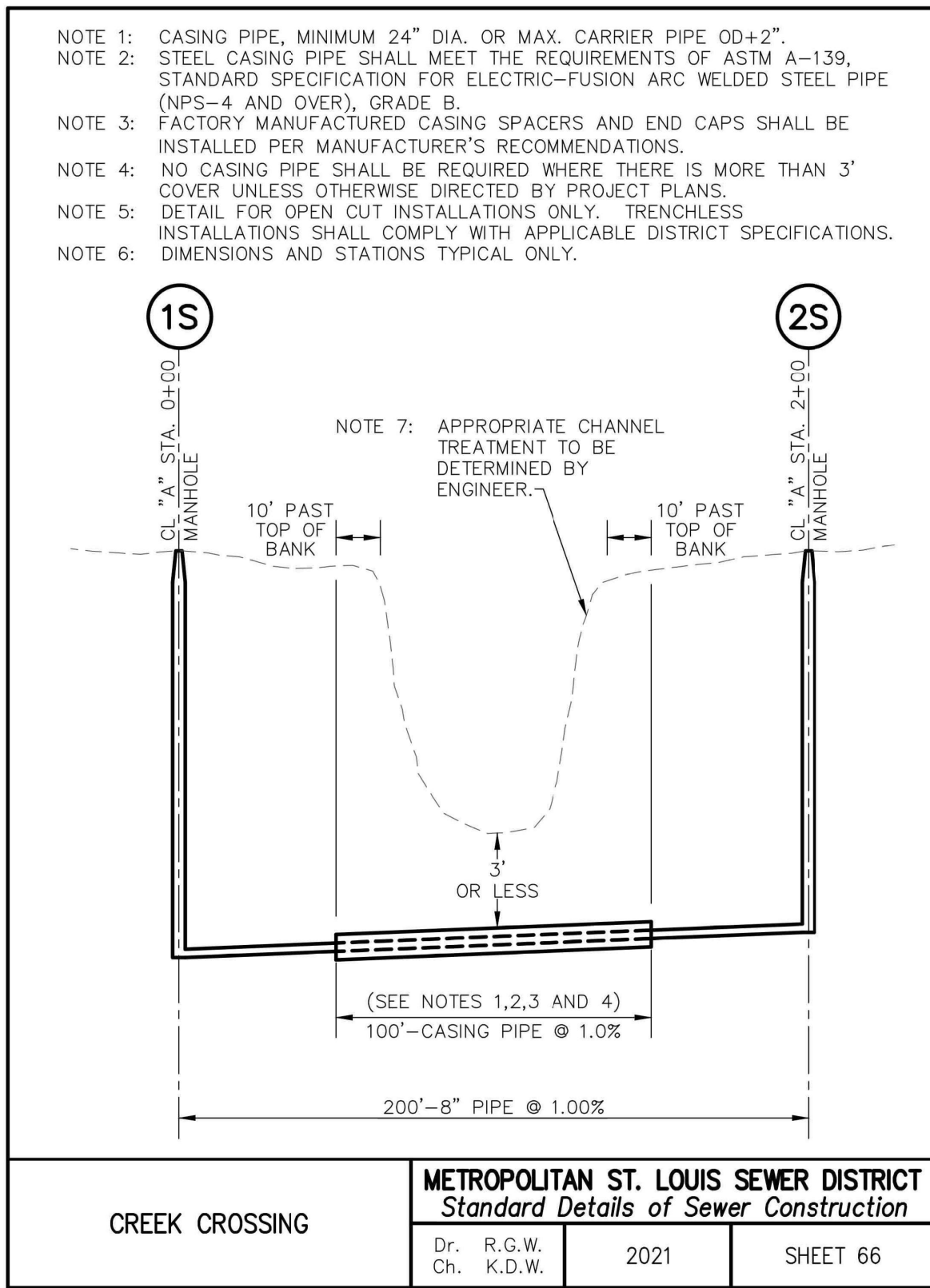
SANITARY PROFILE: MH A - MH M

HORIZONTAL SCALE: 1" = 50'
VERTICAL SCALE: 1" = 10'



SANITARY PROFILE: MH A - MH M

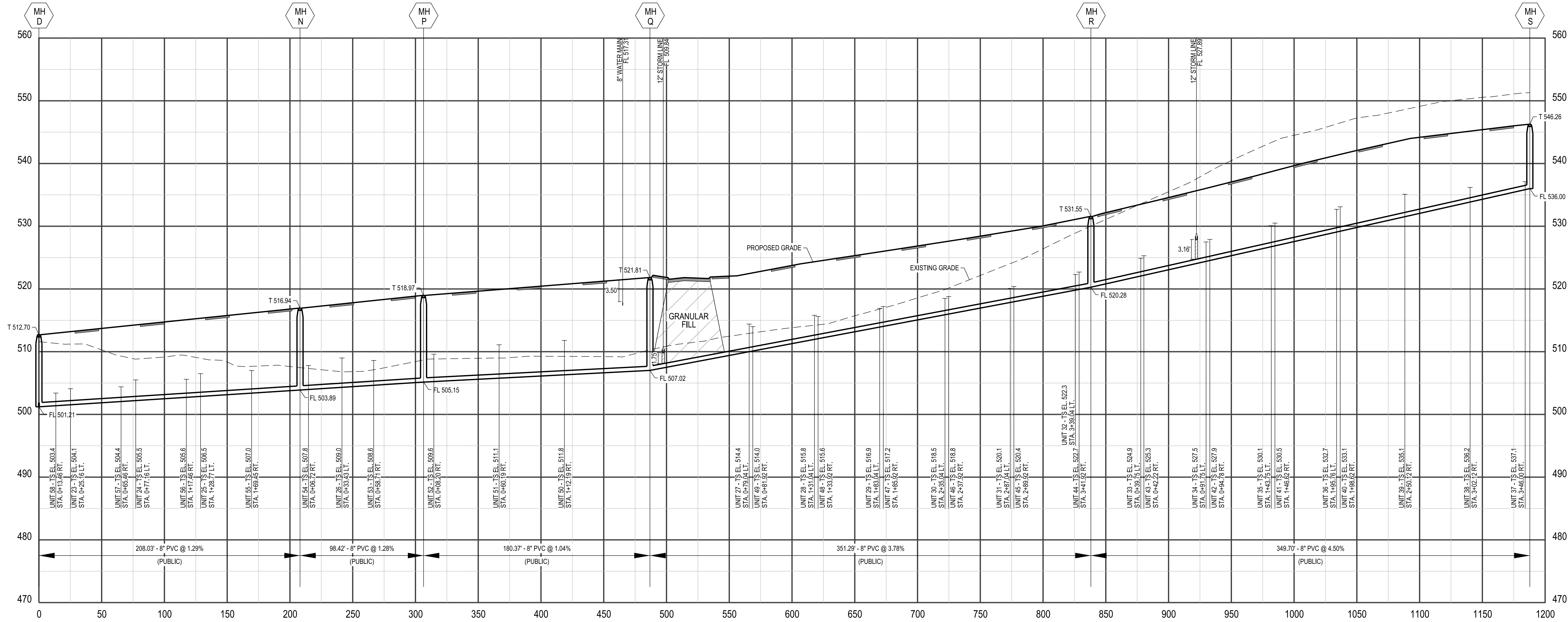
HORIZONTAL SCALE: 1" = 50'
VERTICAL SCALE: 1" = 10'



GENERAL NOTES

1. PROVIDE GRANULAR BACKFILL TO ALL STORM SEWER, SANITARY SEWER, SANITARY LATERALS, WATER MAIN, AND/OR WATER SERVICE LINE TRENCHES THAT CROSS THE PAVEMENT, LIE WITHIN TEN (10) FEET OF THE EDGE OF PAVEMENT AND/OR THE 1:1 SHEAR PLANE OF THE ROAD.
2. WHEN A STORM SEWER CROSSES ABOVE A SANITARY SEWER PIPE, THE UPPER RCP SHALL BE CONCRETE CRADLED ACROSS THE FULL WIDTH OF THE SANITARY SEWER TRENCH. GRANULAR FILL SHALL BE PLACED IN THE TRENCH BETWEEN THE LOWER SANITARY SEWER PIPE AND THE UPPER STORM SEWER PIPE.
3. ALL SANITARY SEWER LATERALS SHALL BE CONSTRUCTED OF SCHEDULE 40 PVC WITH A MINIMUM SLOPE OF 2.0%.
4. ANY SANITARY MANHOLE BUILT WITHIN THE 100 YEAR FLOOD PLAIN LIMITS MUST HAVE LOCK-TYPE WATERTIGHT MANHOLE COVERS. (100 YEAR HWL: 493.56)

H:\CADD\234300\234300-234443\img\Bent\Improvement\Plan\23444 - C17 Sanitary Profiles - Sheet 2.dgn: I:Master Model 7/11/2023 10:39:54 AM Plotted by: sherman Plot Scale: 50.000000 / in. Plot Driver: Volz15 PDF.plt19 Pen Table: Volz15 Plot Full Bit



SANITARY PROFILE: MH D - MH S

HORIZONTAL SCALE: 1" = 50'
VERTICAL SCALE: 1" = 10'

UNIT	TOP OF FOUNDATION	BASEMENT FLOOR ELEVATION	TAIL-STAKE ELEVATION	LATERAL STATION & DIRECTION	DOWNSTREAM MANHOLE	ADDRESS (O'FALLON, MO 63376)
1	552.31	543.31	541.3	2+09.72 LT.	MH J	71 STRAWBERRY FARMS CT.
2	553.35	544.35	542.4	2+61.64 LT.	MH J	73 STRAWBERRY FARMS CT.
3	554.71	545.71	543.7	3+13.68 LT.	MH J	75 STRAWBERRY FARMS CT.
4	555.61	546.61	544.1	0+48.65 LT.	MH K	77 STRAWBERRY FARMS CT.
5	556.60	547.60	545.1	0+08.46 LT.	MH L	79 STRAWBERRY FARMS CT.
6	558.19	549.19	546.7	0+52.20 LT.	MH L	81 STRAWBERRY FARMS CT.
7	554.77	545.77	543.8	3+03.82 RT.	MH J	76 STRAWBERRY FARMS CT.
8	553.15	544.15	542.2	2+52.42 RT.	MH J	74 STRAWBERRY FARMS CT.
9	552.10	543.10	541.1	1+99.37 RT.	MH J	72 STRAWBERRY FARMS CT.
10	549.46	540.46	538.5	0+68.34 RT.	MH J	374 STRAWBERRY GROVE DR.
11	547.73	538.73	536.7	0+10.70 RT.	MH J	372 STRAWBERRY GROVE DR.
12	545.00	536.00	534.0	1+54.45 RT.	MH H	370 STRAWBERRY GROVE DR.
13	541.65	532.65	530.7	0+97.82 RT.	MH H	368 STRAWBERRY GROVE DR.
14	538.30	529.30	527.3	0+45.78 RT.	MH H	366 STRAWBERRY GROVE DR.
15	534.95	525.95	524.0	2+33.54 RT.	MH G	364 STRAWBERRY GROVE DR.
16	531.60	522.60	520.6	1+78.27 RT.	MH G	362 STRAWBERRY GROVE DR.
17	528.24	519.24	517.2	1+24.64 RT.	MH G	360 STRAWBERRY GROVE DR.
18	524.89	515.89	513.9	0+70.22 RT.	MH G	358 STRAWBERRY GROVE DR.
19	521.83	512.83	510.8	0+17.08 RT.	MH G	356 STRAWBERRY GROVE DR.
20	517.00	508.00	506.0	1+11.72 RT.	MH D	334 STRAWBERRY GROVE DR.
21	515.80	506.80	504.8	0+60.19 RT.	MH D	332 STRAWBERRY GROVE DR.
22	514.90	505.90	503.9	0+11.69 RT.	MH D	330 STRAWBERRY GROVE DR.
23	515.10	506.10	504.1	0+25.16 LT.	MH D	326 STRAWBERRY GROVE DR.
24	516.48	507.48	505.5	0+77.16 LT.	MH D	324 STRAWBERRY GROVE DR.
25	517.50	508.50	506.5	1+28.77 LT.	MH D	322 STRAWBERRY GROVE DR.
26	520.03	511.03	509.0	0+33.43 LT.	MH N	316 STRAWBERRY GROVE DR.
27	525.40	516.40	514.4	0+79.04 LT.	MH Q	227 STRAWBERRY FARMS DR.
28	526.83	517.83	515.8	1+31.04 LT.	MH Q	225 STRAWBERRY FARMS DR.
29	528.42	519.42	516.9	1+83.04 LT.	MH Q	223 STRAWBERRY FARMS DR.
30	530.00	521.00	518.5	2+35.04 LT.	MH Q	221 STRAWBERRY FARMS DR.
31	531.64	522.64	520.1	2+87.04 LT.	MH Q	219 STRAWBERRY FARMS DR.
32	533.85	524.85	522.3	3+39.04 LT.	MH Q	217 STRAWBERRY FARMS DR.
33	536.45	527.45	524.9	0+39.75 LT.	MH R	215 STRAWBERRY FARMS DR.
34	539.05	530.05	527.5	0+91.75 LT.	MH R	213 STRAWBERRY FARMS DR.
35	541.65	532.65	530.1	1+43.75 LT.	MH R	211 STRAWBERRY FARMS DR.
36	544.25	535.25	532.7	1+95.76 LT.	MH R	209 STRAWBERRY FARMS DR.
37	548.14	539.14	537.1	3+46.00 RT.	MH R	204 STRAWBERRY FARMS DR.
38	547.23	538.23	536.2	3+02.12 RT.	MH R	206 STRAWBERRY FARMS DR.

UNIT	TOP OF FOUNDATION	BASEMENT FLOOR ELEVATION	TAIL-STAKE ELEVATION	LATERAL STATION & DIRECTION	DOWNSTREAM MANHOLE	ADDRESS (O'FALLON, MO 63376)
39	546.09	537.09	535.1	2+50.12 RT.	MH R	208 STRAWBERRY FARMS DR.
40	544.06	535.06	533.1	1+08.62 RT.	MH R	210 STRAWBERRY FARMS DR.
41	541.46	532.46	530.5	1+46.62 RT.	MH R	212 STRAWBERRY FARMS DR.
42	538.86	529.86	527.9	0+94.78 RT.	MH R	214 STRAWBERRY FARMS DR.
43	536.26	527.26	525.3	0+42.22 RT.	MH R	216 STRAWBERRY FARMS DR.
44	533.66	524.66	522.7	3+41.92 RT.	MH Q	218 STRAWBERRY FARMS DR.
45	531.42	522.42	520.4	2+89.92 RT.	MH Q	220 STRAWBERRY FARMS DR.
46	529.77	520.77	518.8	2+37.92 RT.	MH Q	222 STRAWBERRY FARMS DR.
47	528.18	519.18	517.2	1+85.92 RT.	MH Q	224 STRAWBERRY FARMS DR.
48	526.59	517.59	515.6	1+33.92 RT.	MH Q	226 STRAWBERRY FARMS DR.
49	525.01	516.01	514.0	0+81.92 RT.	MH Q	228 STRAWBERRY FARMS DR.
50	523.28	514.28	511.8	1+12.19 RT.	MH P	305 STRAWBERRY GROVE DR.
51	522.56	513.56	511.1	0+60.19 RT.	MH P	307 STRAWBERRY GROVE DR.
52	521.12	512.12	509.6	0+08.20 RT.	MH P	309 STRAWBERRY GROVE DR.
53	520.14	511.14	508.6	0+58.71 RT.	MH N	313 STRAWBERRY GROVE DR.
54	519.30	510.30	507.8	0+06.72 RT.	MH N	317 STRAWBERRY GROVE DR.
55	518.51	509.51	507.0	1+69.45 RT.	MH D	321 STRAWBERRY GROVE DR.
56	517.07	508.07	505.6	1+17.46 RT.	MH D	323 STRAWBERRY GROVE DR.
57	515.90	506.90	504.4	0+65.46 RT.	MH D	325 STRAWBERRY GROVE DR.
58	514.86	505.86	503.4	0+13.46 RT.	MH D	327 STRAWBERRY GROVE DR.
59	514.86	505.86	503.4	0+26.95 LT.	MH D	331 STRAWBERRY GROVE DR.
60	516.23	507.23	504.7	0+78.60 LT.	MH D	333 STRAWBERRY GROVE DR.
61	517.17	508.17	505.7	1+30.20 LT.	MH D	337 STRAWBERRY GROVE DR.
62	517.91	508.91	506.4	0+06.05 LT.	MH E	341 STRAWBERRY GROVE DR.
63	518.82	509.82	507.3	0+57.11 LT.	MH E	345 STRAWBERRY GROVE DR.
64	519.65	510.65	508.2	0+03.92 LT.	MH F	349 STRAWBERRY GROVE DR.
65	520.06	511.06	509.1	0+55.23 LT.	MH F	353 STRAWBERRY GROVE DR.
66	521.49	512.49	510.5	0+03.40 LT.	MH G	355 STRAWBERRY GROVE DR.
67	524.12	515.12	513.1	0+55.31 LT.	MH G	357 STRAWBERRY GROVE DR.
68	527.14	518.14	516.1	1+07.31 LT.	MH G	359 STRAWBERRY GROVE DR.
69	530.18	521.18	519.2	1+59.33 LT.	MH G	361 STRAWBERRY GROVE DR.
70	533.21	524.21	522.2	2+11.29 LT.	MH G	363 STRAWBERRY GROVE DR.
71	536.25	527.25	525.3	0+09.35 LT.	MH H	365 STRAWBERRY GROVE DR.
72	539.28	530.28	528.3	0+61.33 LT.	MH H	367 STRAWBERRY GROVE DR.
73	542.28	533.28	531.3	1+13.33 LT.	MH H	369 STRAWBERRY GROVE DR.
74	545.32	536.32	534.3	1+65.30 LT.	MH H	371 STRAWBERRY GROVE DR.
75	547.79	538.79	536.8	0+09.66 LT.	MH J	373 STRAWBERRY GROVE DR.
76	549.39	540.39	538.4	0+61.14 LT.	MH J	375 STRAWBERRY GROVE DR.

GENERAL NOTES

- PROVIDE GRANULAR BACKFILL TO ALL STORM SEWER, SANITARY SEWER, SANITARY LATERALS, WATER MAIN, AND/OR WATER SERVICE LINE TRENCHES THAT CROSS THE PAVEMENT. LIE WITHIN TEN (10) FEET OF THE EDGE OF PAVEMENT AND/OR THE 1:1 SHEAR PLANE OF THE ROAD.
- WHEN A STORM SEWER CROSSES ABOVE A SANITARY SEWER PIPE, THE UPPER RCP SHALL BE CONCRETE CRADLED ACROSS THE FULL WIDTH OF THE SANITARY SEWER TRENCH. GRANULAR FILL SHALL BE PLACED IN THE TRENCH BETWEEN THE LOWER SANITARY SEWER PIPE AND THE UPPER STORM SEWER PIPE.
- ALL SANITARY SEWER LATERALS SHALL BE CONSTRUCTED OF SCHEDULE 40 PVC WITH A MINIMUM SLOPE OF 2.0%.
- ANY SANITARY MANHOLE BUILT WITHIN THE 100 YEAR FLOOD PLAIN LIMITS MUST HAVE LOCK-TYPE WATERTIGHT MANHOLE COVERS. (100 YEAR HWL: 493.56)

SANITARY PROFILES: SHEET 2

PARCEL ID: 23115-116-00-00245
CITY OF O'FALLON
PERMIT NO: B5P25-00001
PROJECT E2344

8265 MEXICO ROAD,
O'FALLON, MO 63376

07-01-23
C17

STRAWBERRY FARMS

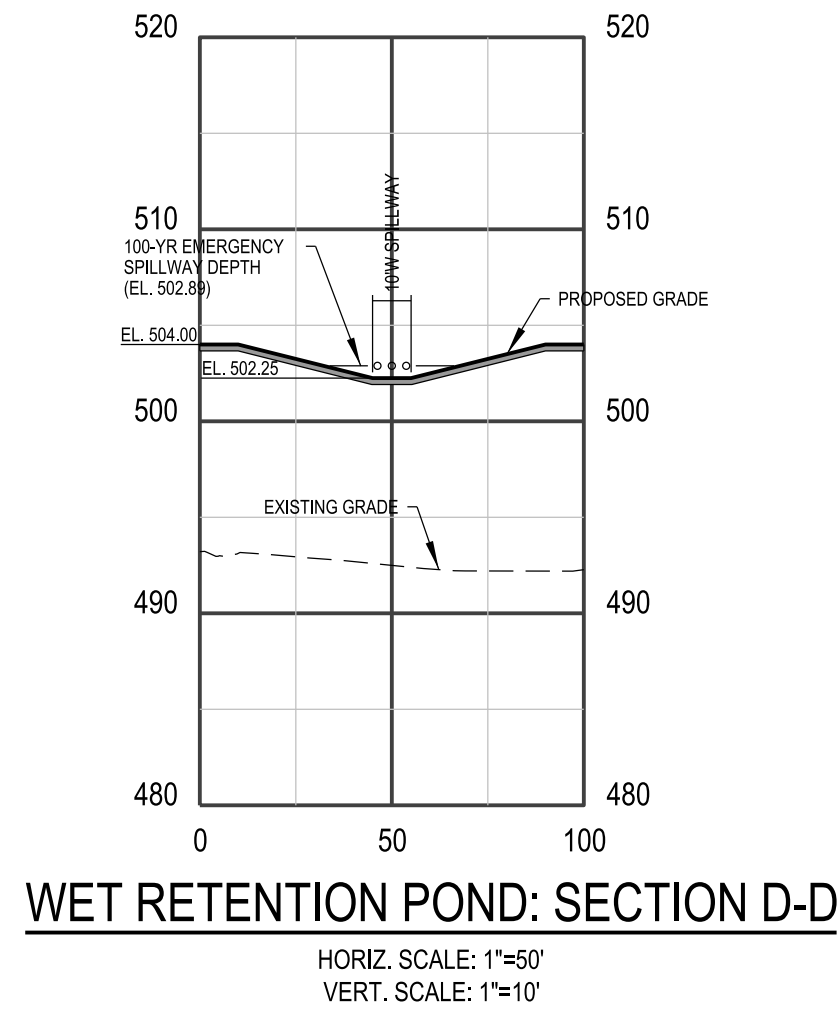
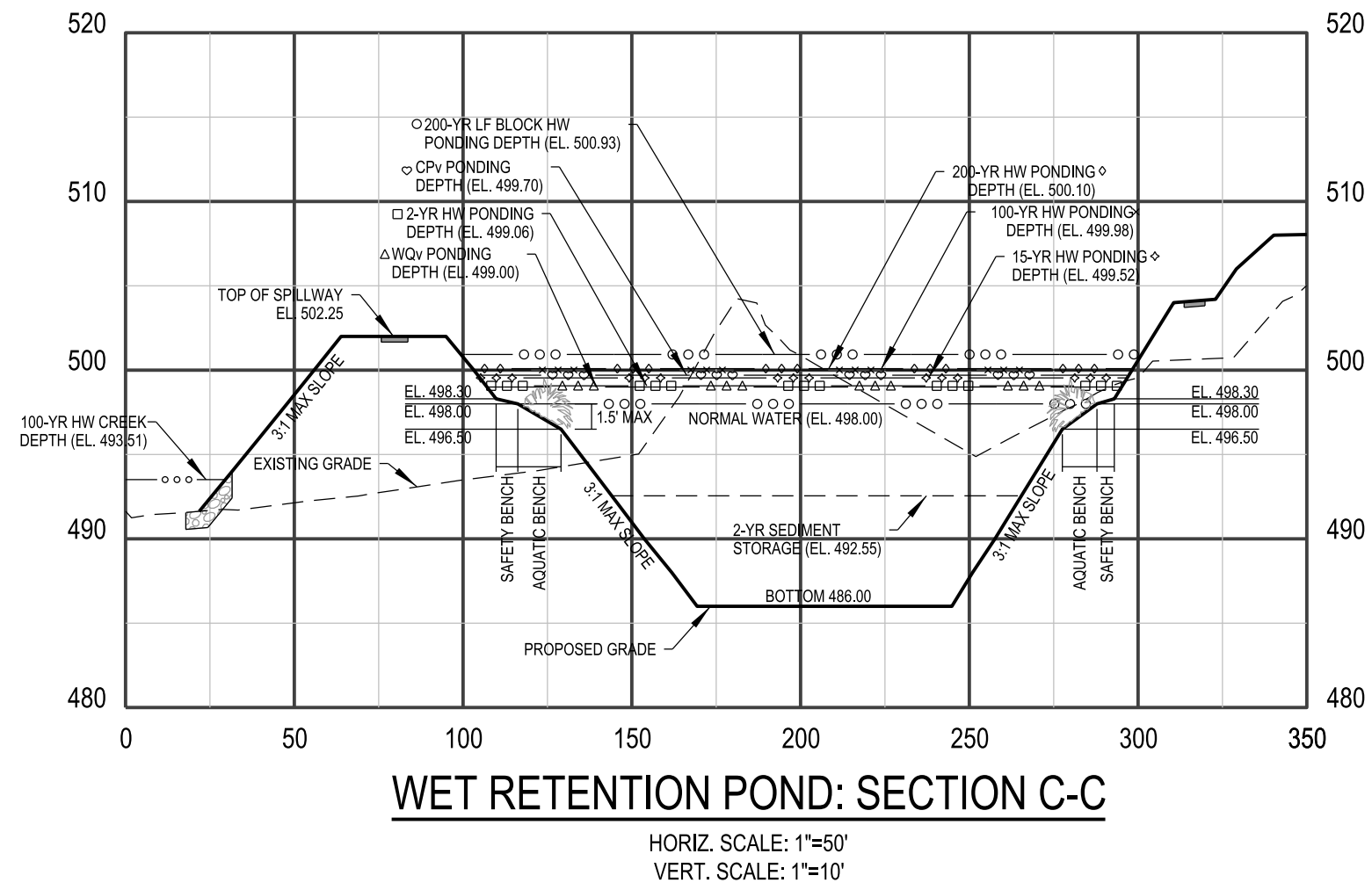
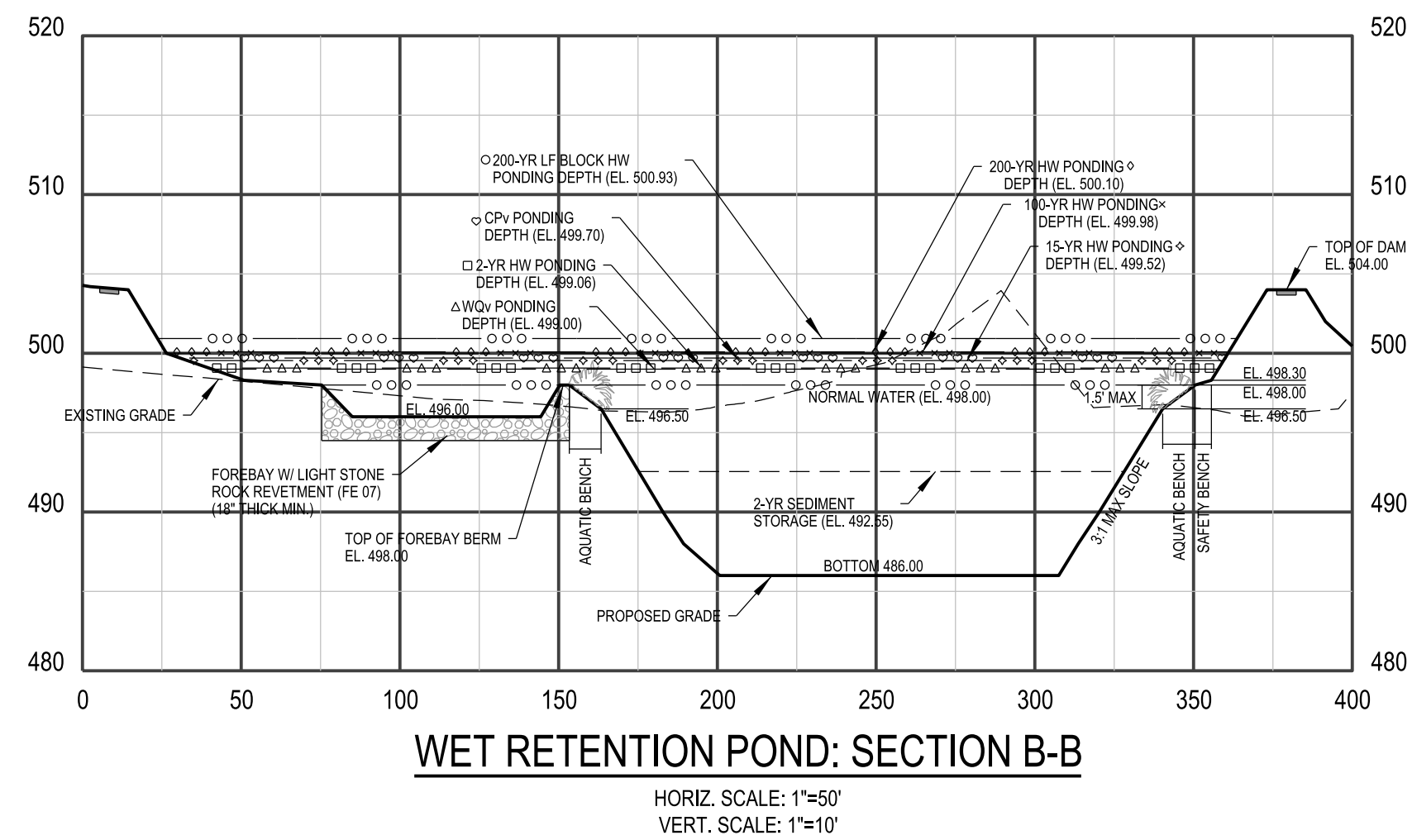
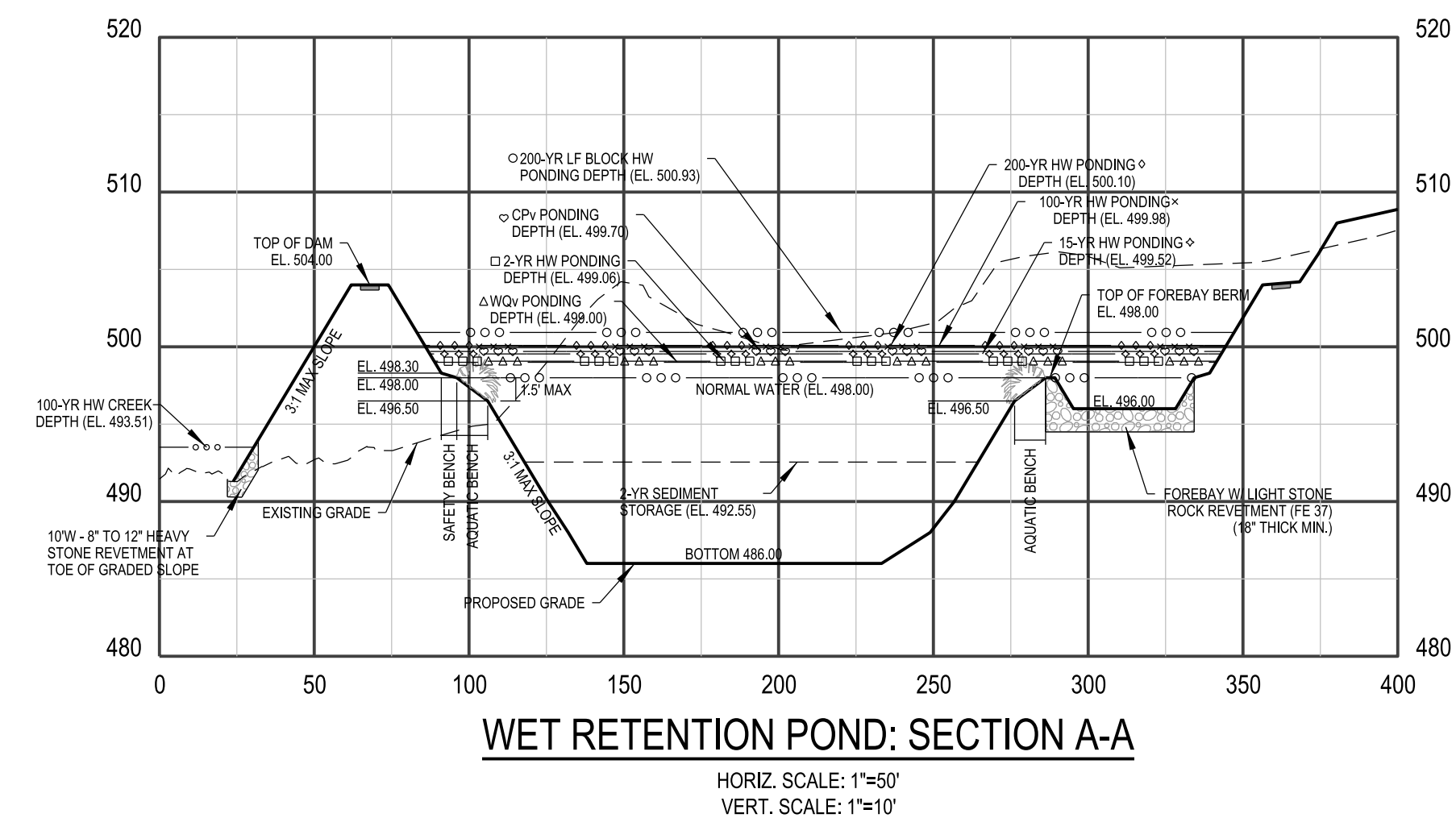
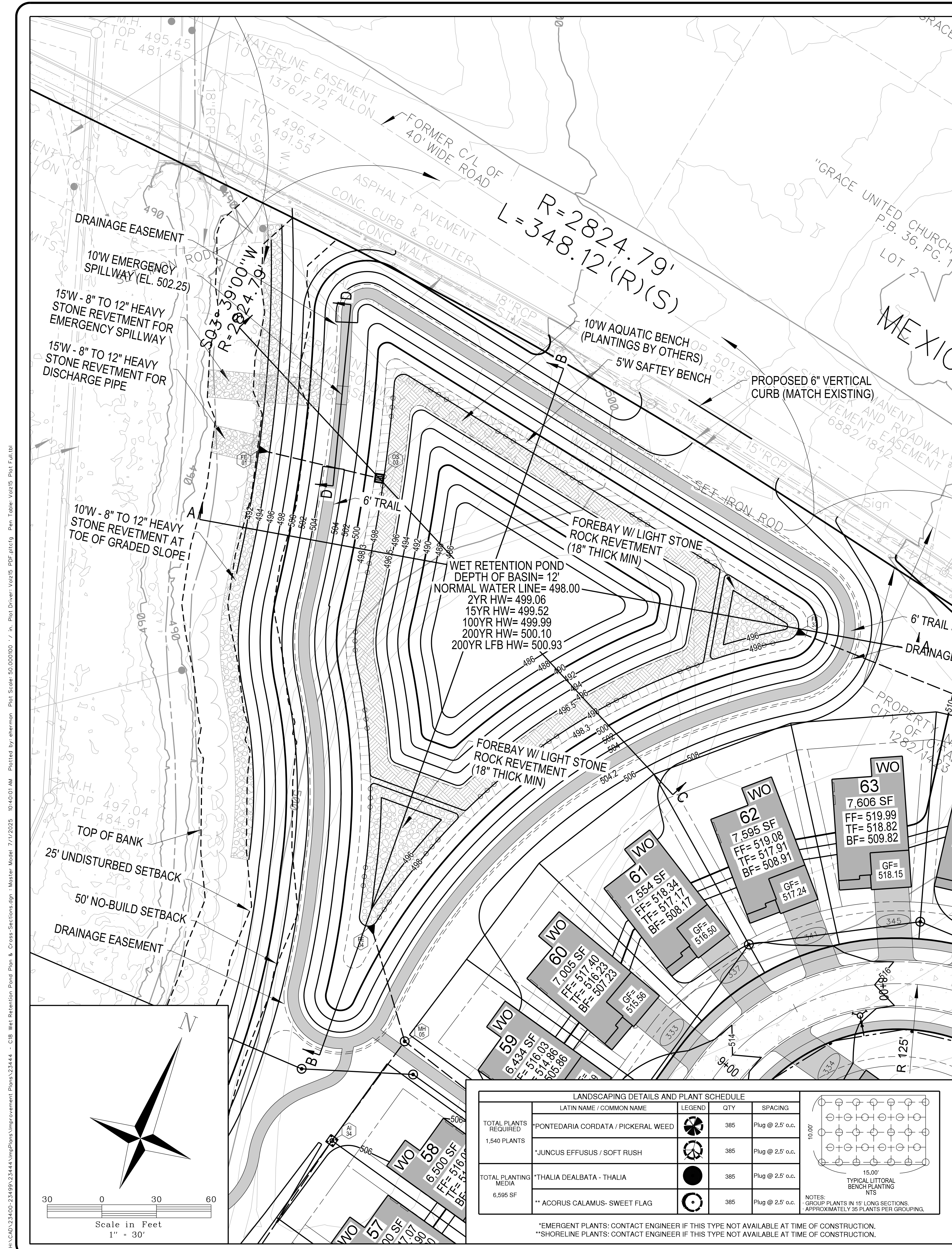


TIMOTHY J. MEYER
Professional Engineer
MO E-24665

VOLZ
Incorporated
10849 Indian Head Trl., Suite 110
St. Charles, Mo 63315
314.426.6212 Main
314.890.1250 Fax
www.volzinc.com
Authority #203

McKELVEY
HOMES
Building A Tradition Of Excellence Since 1888

424 S. WOODS MILL RD., SUITE 110
TOWN & COUNTRY, MISSOURI 63017

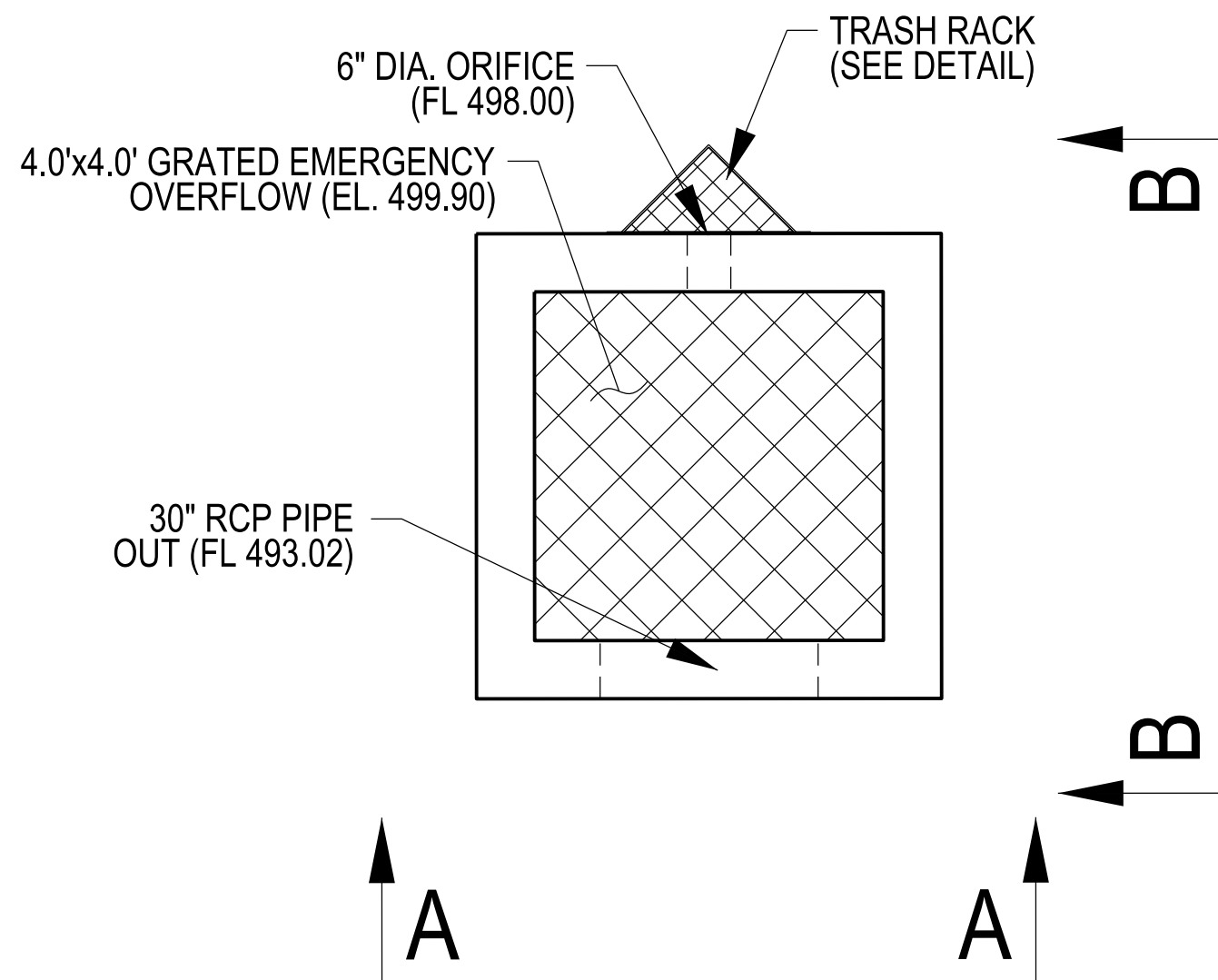


GENERAL NOTES:

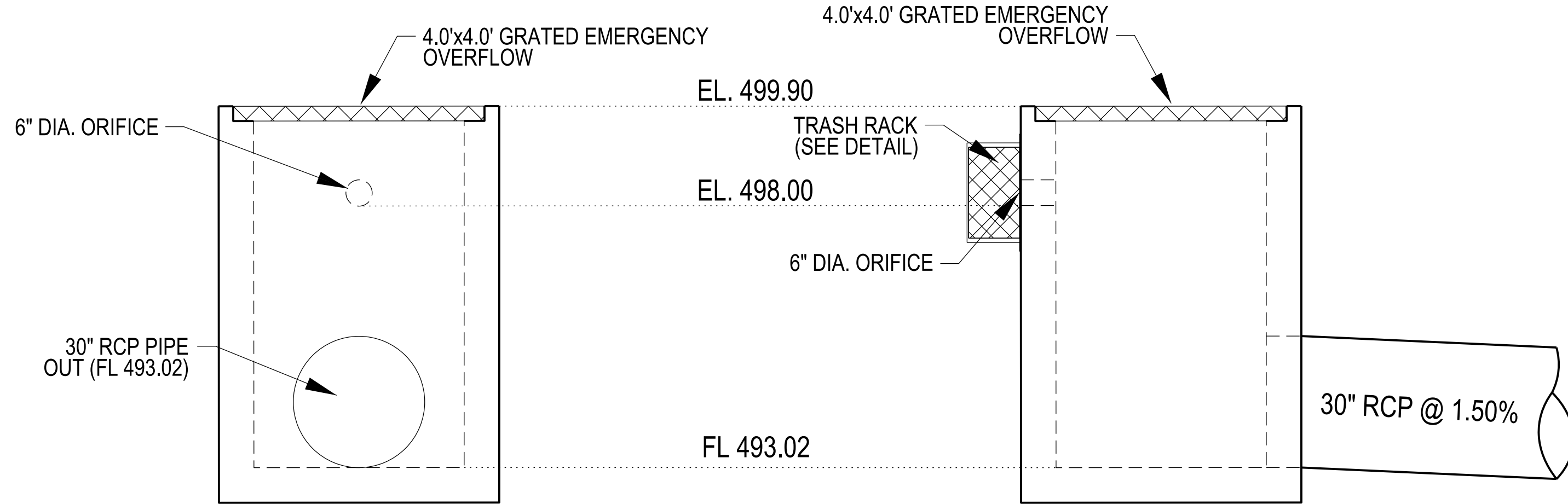
1. THE MINIMUM DEPTH OF THE RETENTION POND IS TO BE 10 FEET. ANY SEDIMENT ACCUMULATION WILL NEED TO BE REMOVED TO ENSURE THIS REQUIREMENT. THIS WILL BE DETERMINED WITH THE AS-BUILTS OF THE RETENTION BASIN.

Wet Retention Pond Design and As-built Verification Information Table										
Basin ID	Design Overflow Sill Elev. (ft)*	As-built Overflow Sill Elev. (ft)*	Design Bypass/ Spill Point Elev. (ft)**	As-built Bypass/ Spill Point Elev. (ft)**	Required WQ Volume (ft³)	Proposed WQ Volume (ft³)	As-built WQ Volume (ft³)	Required CP _v Volume (ft³)	Proposed CP _v Volume (ft³)	CP _v Provided (ft³)
Wet Retention Pond (P-2 Wet Pond)	499.90		502.25		34,266	35,034 (El. 499.00)		61,976	62,529 (El. 499.70)	

* Overflow Sill Elevation = Maximum Water Quality Storage Elevation
 ** Bypass/Spill Point Elevation = Lowest adjacent elevation on the basin perimeter where overland flow would be directed.
 As-built portion of table to be certified by a Professional Engineer or Professional Land Surveyor licensed in Missouri

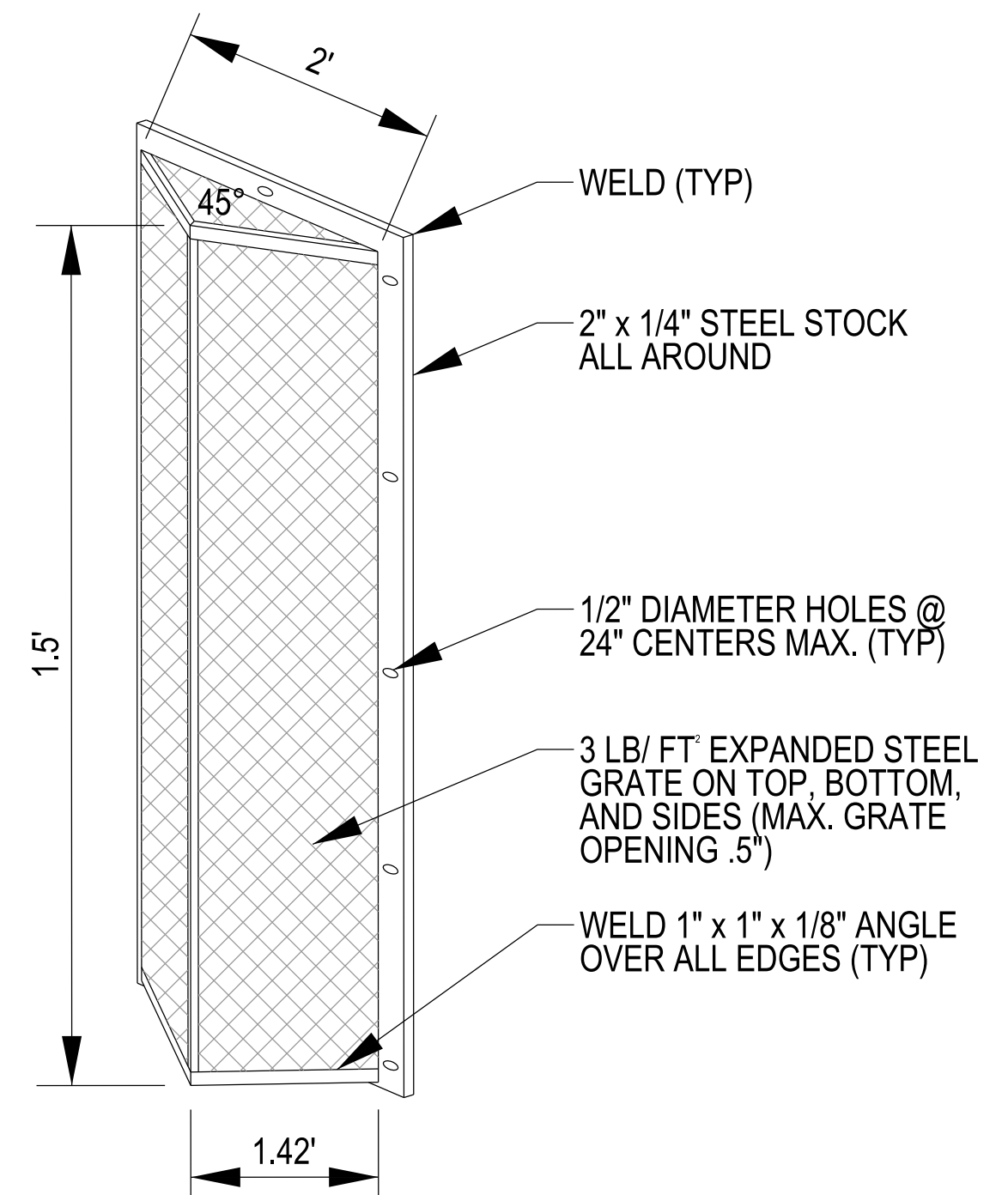


OS 03 TOP VIEW
SCALE: 1" = 2'



SECTION A-A
SCALE: 1" = 2'

SECTION B-B
SCALE: 1" = 2'

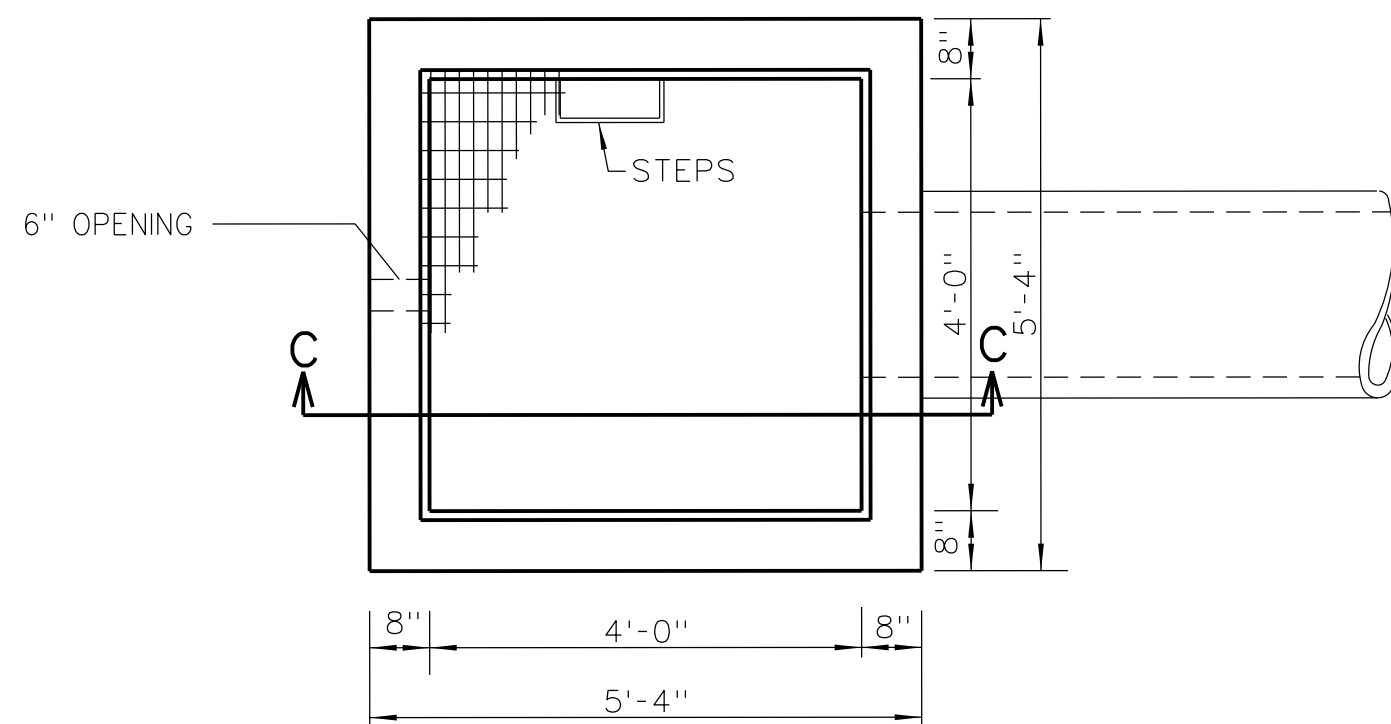


NOTES FOR TRASH RACK:

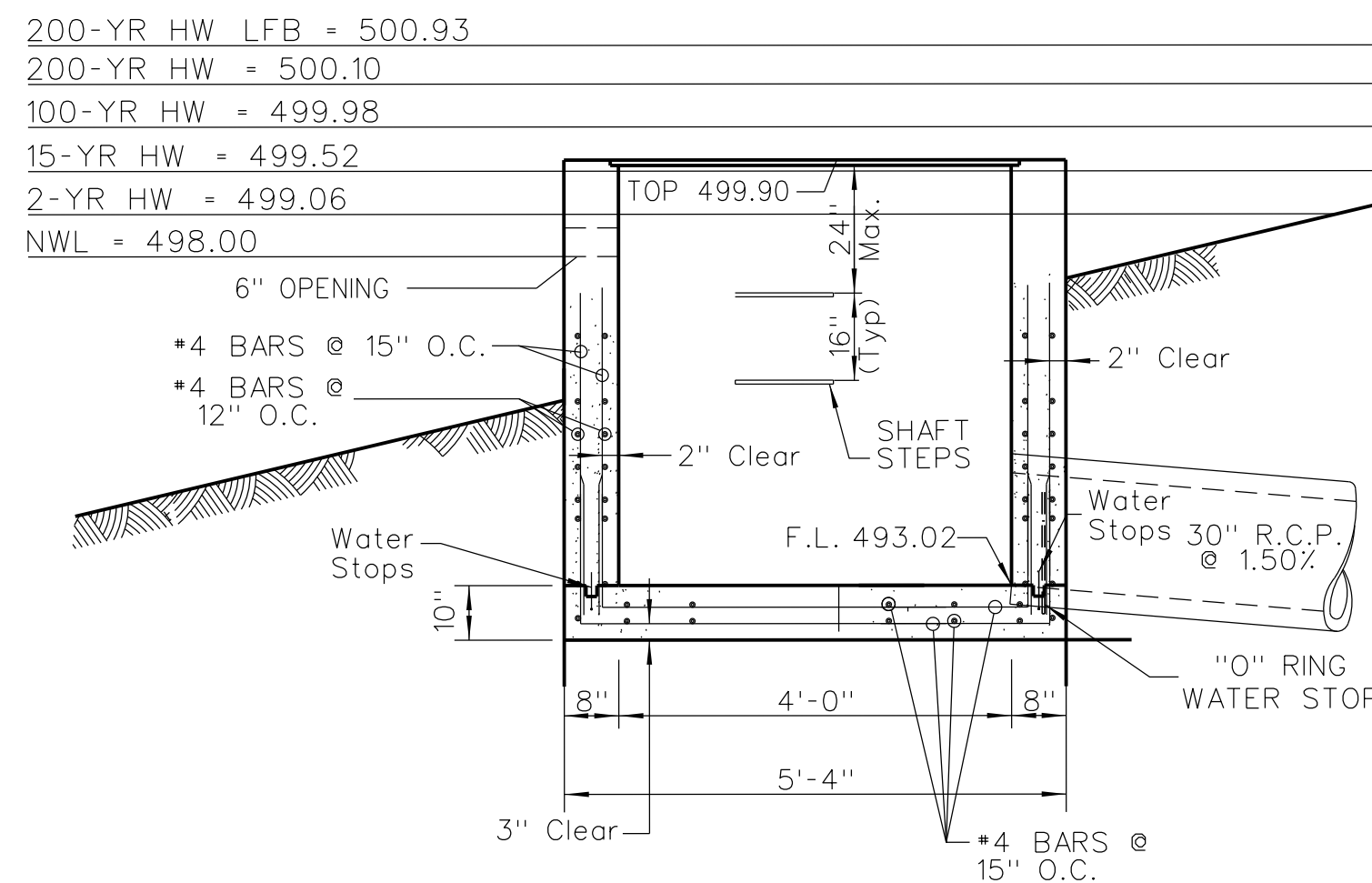
1. TRASH RACK SHALL BE CENTERED OVER OPENING.
2. STEEL TO CONFORM TO ASTM-36.
3. ALL SURFACES SHALL BE COATED WITH ZRC COLD GALVANIZING COMPOUND AFTER WELDING.
4. TRASH RACK SHALL BE FASTENED TO THE WALL WITH 1/2" MASONRY ANCHORS. TRASH RACK SHALL BE REMOVEABLE.

TRASH RACK DETAIL
(OR APPROVED EQUAL)
N.T.S.

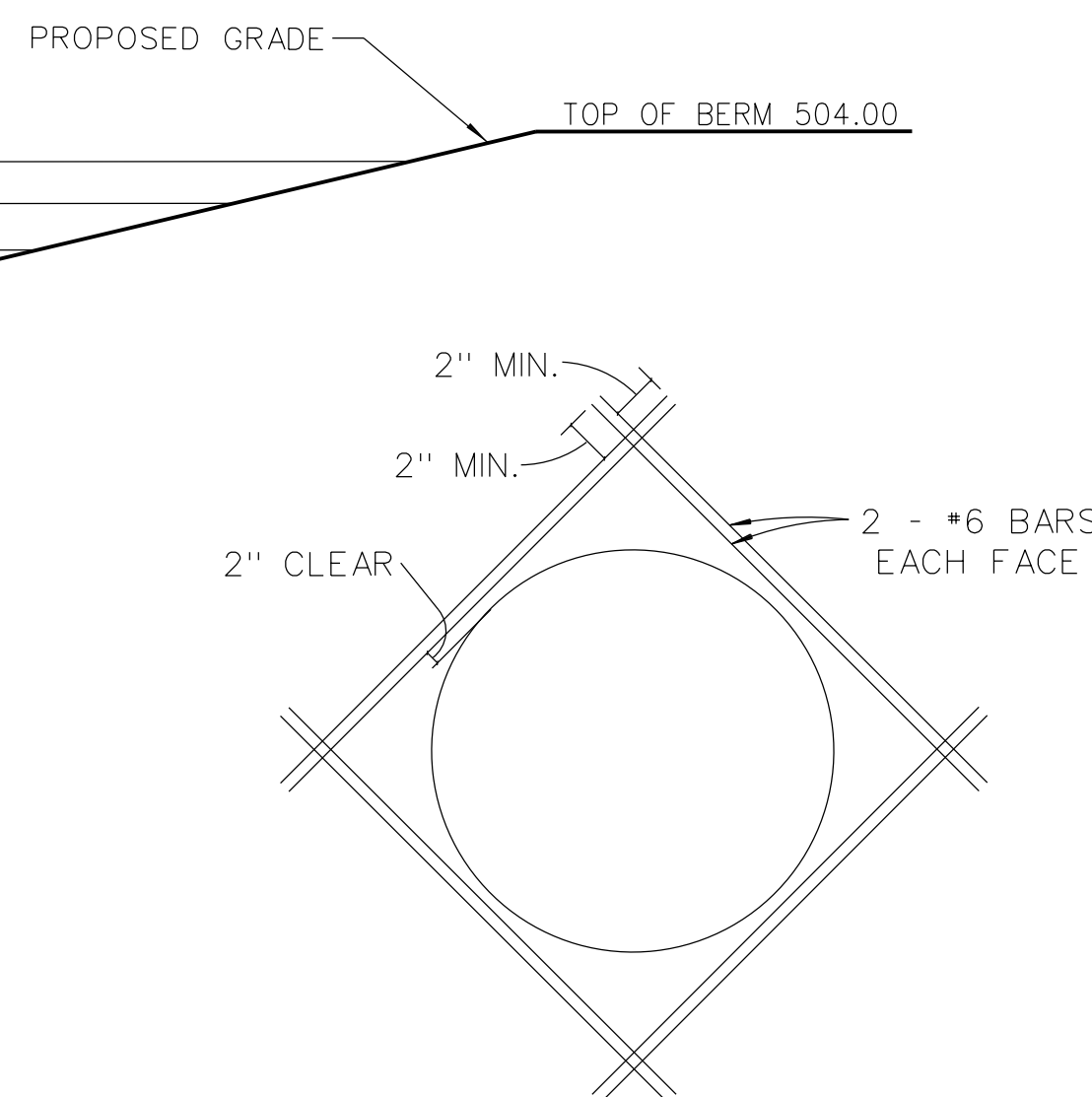
SHOP DRAWING REQUIRED
PRIOR TO CONSTRUCTION



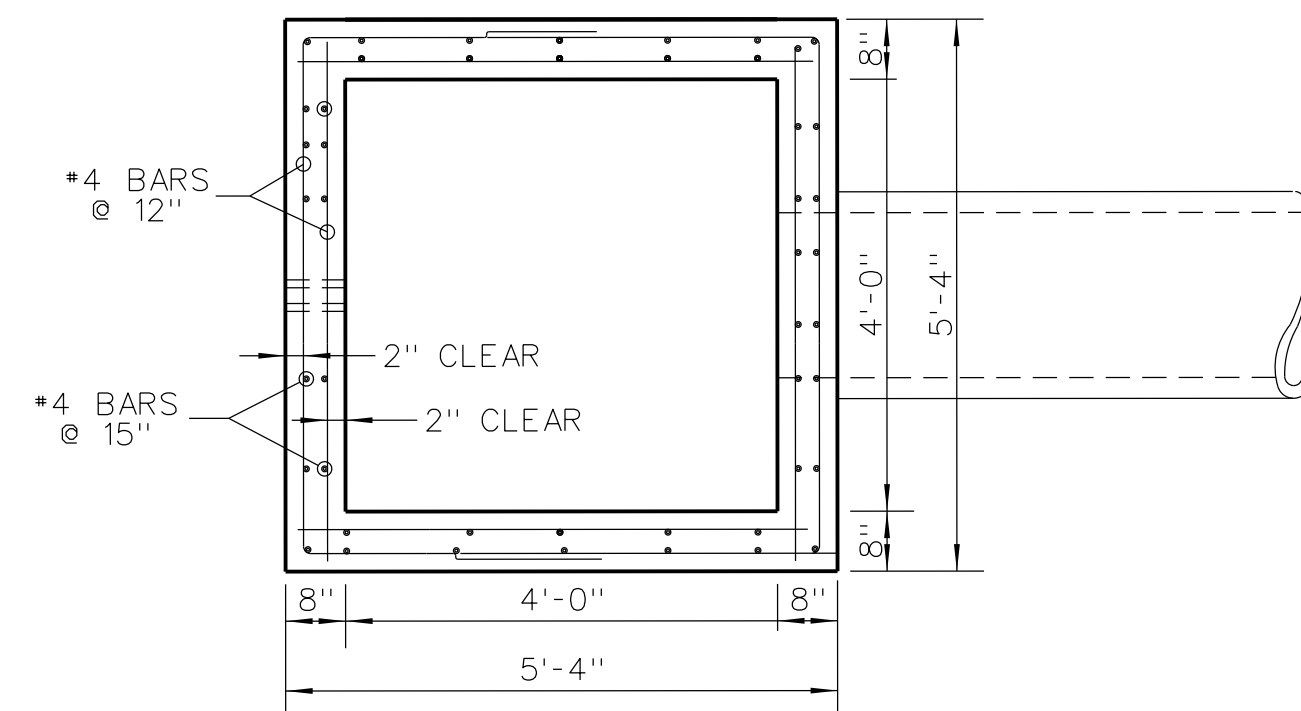
OS 03 TOP VIEW
N.T.S.



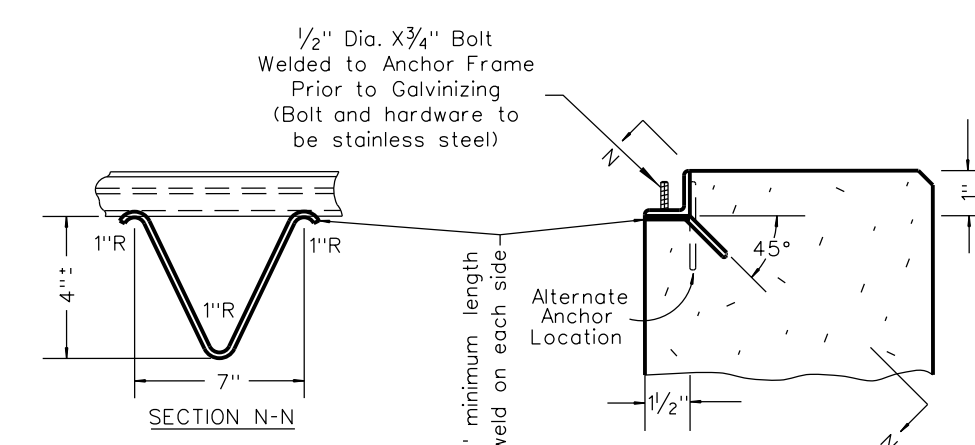
SECTION C-C
OS 03 REINFORCEMENT DETAIL
N.T.S.



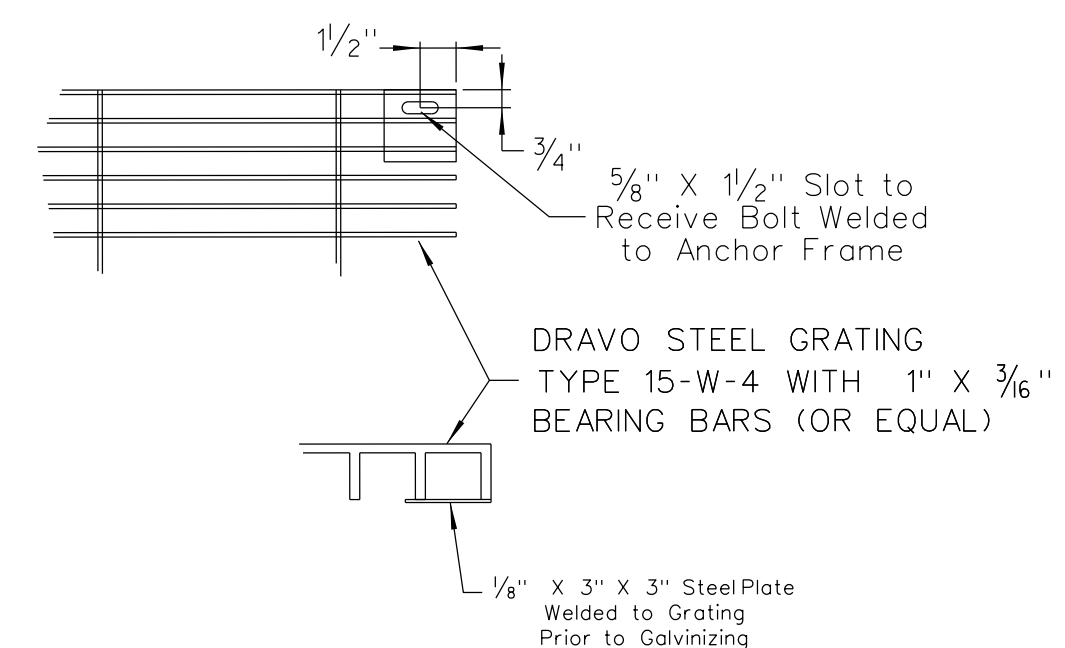
REINFORCEMENT AROUND
OPENING
N.T.S.



OS 03 TOP VIEW
REINFORCEMENT DETAIL
N.T.S.

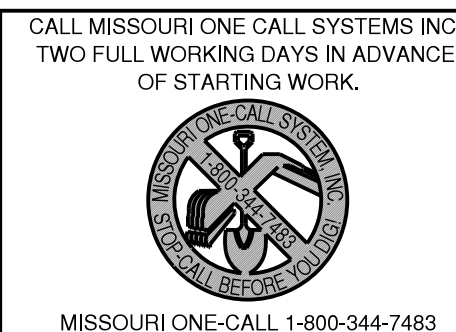


DETAIL OF CONTINUOUS ANCHOR FOR
ANGLE SEAT & BOLT DOWN GRATE
N.T.S.



CONSTRUCTION NOTES:

1. Concrete for the structure shall be "air entrained" and contain at least 6 sacks Class "A" Portland Cement per cubic yard. The concrete shall be placed at a slump of 4 inches +1/2 inch. The concrete shall be proportioned and transported in accordance with ASTM C-94.
2. Reinforcing steel shall conform to ASTM C-615-60 with deformations conforming to ASTM A-305 and shall have a minimum cover of 2 inches except for 3 inches where concrete is poured against earth.
3. Laps and/or splices in reinforcing steel shall be a minimum of 30 bar diameters.
4. Keyed joints shown are to be 2-inch x 2-inch keyed construction joints.
5. $f_s = 24,000$ psi.
6. $f_c = 3,750$ psi.
7. All exposed edges to have a 3/4-inch chamfer.
8. Contractor to provide for bypass of stormwater during construction of structure.
9. All soil specifications shall be directed by soils engineer.



THE UNDERGROUND UTILITIES SHOWN HEREIN WERE PLOTTED FROM AVAILABLE INFORMATION AND FIELD MARKINGS PROVIDED BY THE MEMBER UTILITIES OF THE MISSOURI ONE CALL SYSTEM (1-800-DIG-RI-TE), AND DO NOT NECESSARILY REFLECT THE ACTUAL EXISTENCE, NON-EXISTENCE, SIZE, TYPE, CAPACITY, NUMBER, OR LOCATION OF THESE OR OTHER UTILITIES, NOR THE ABILITY TO SERVE THE EXISTING OR INTENDED USES OF THIS OR ADJACENT SITES. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THE ACTUAL LOCATION OF ALL UNDERGROUND UTILITIES IN THE FIELD, SHOWN OR NOT SHOWN, PRIOR TO ANY GRADING, EXCAVATION, OR CONSTRUCTION OF IMPROVEMENTS. THESE PROVISIONS SHALL IN NO WAY ABSOLVE ANY PARTY FROM COMPLYING WITH THE UNDERGROUND FACILITY SAFETY AND DAMAGE PREVENTION ACT, CHAPTER 319, RSMO.

McKELVEY HOMES
Building A Tradition Of Excellence Since 1888
421 S. WOODS MILL RD., SUITE 110
TOWN & COUNTRY, MISSOURI 63017

VOLZ Incorporated
10849 Indiana Road, Ind. Bldg.
St. Louis, MO 63112
Ph: 314-426-6212 Main
314-890-1250 Fax
www.volzinc.com
Authority #203

TIMOTHY J MEYER
Professional Engineer
MO E-24665

STRAWBERRY FARMS

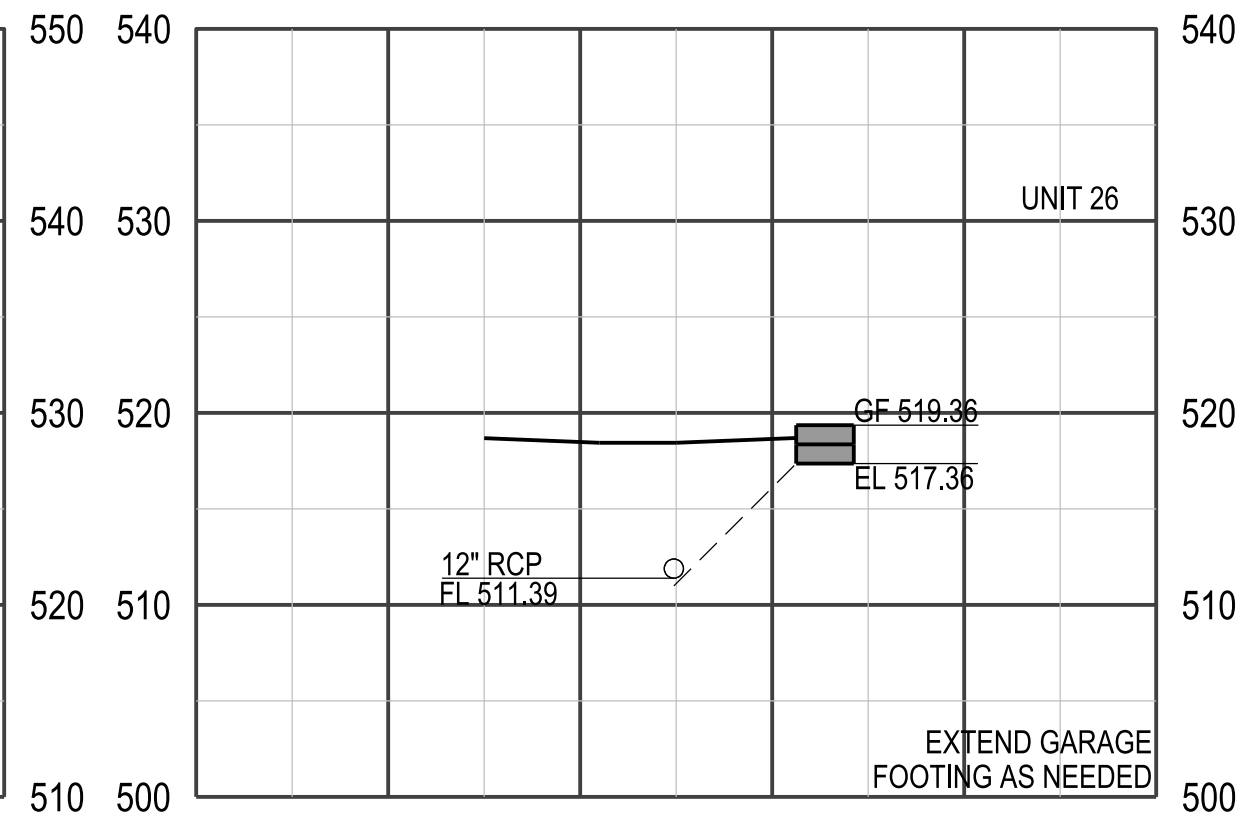
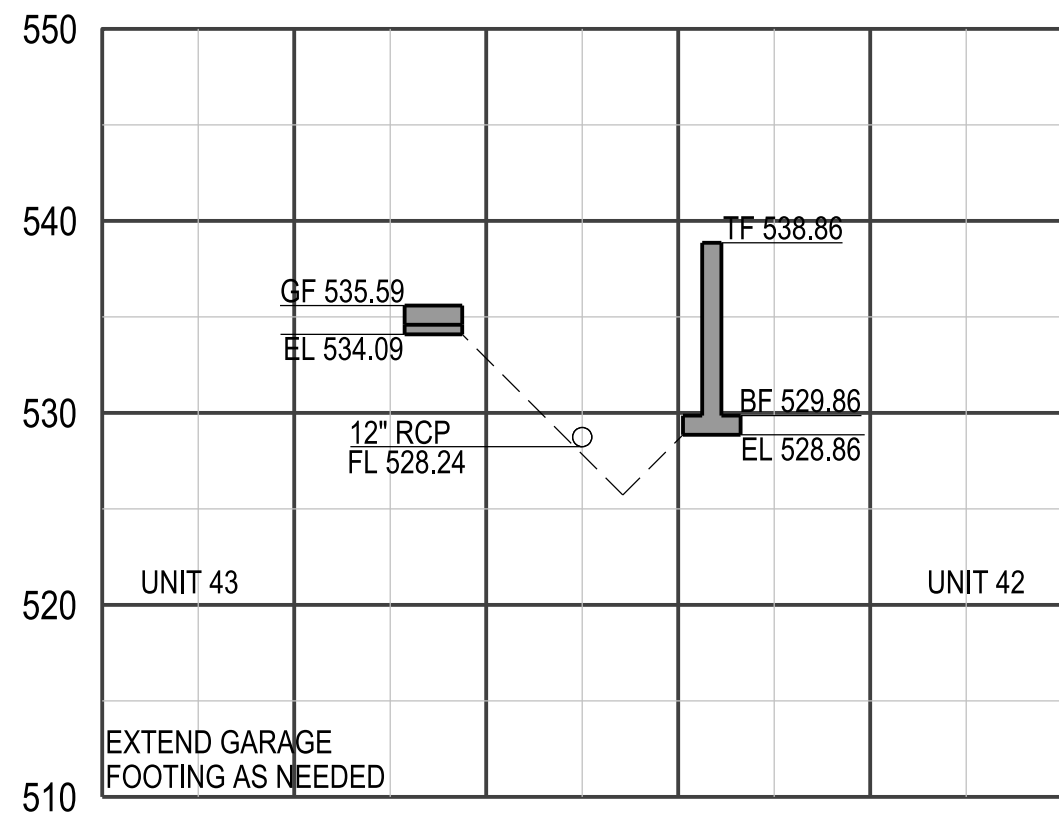
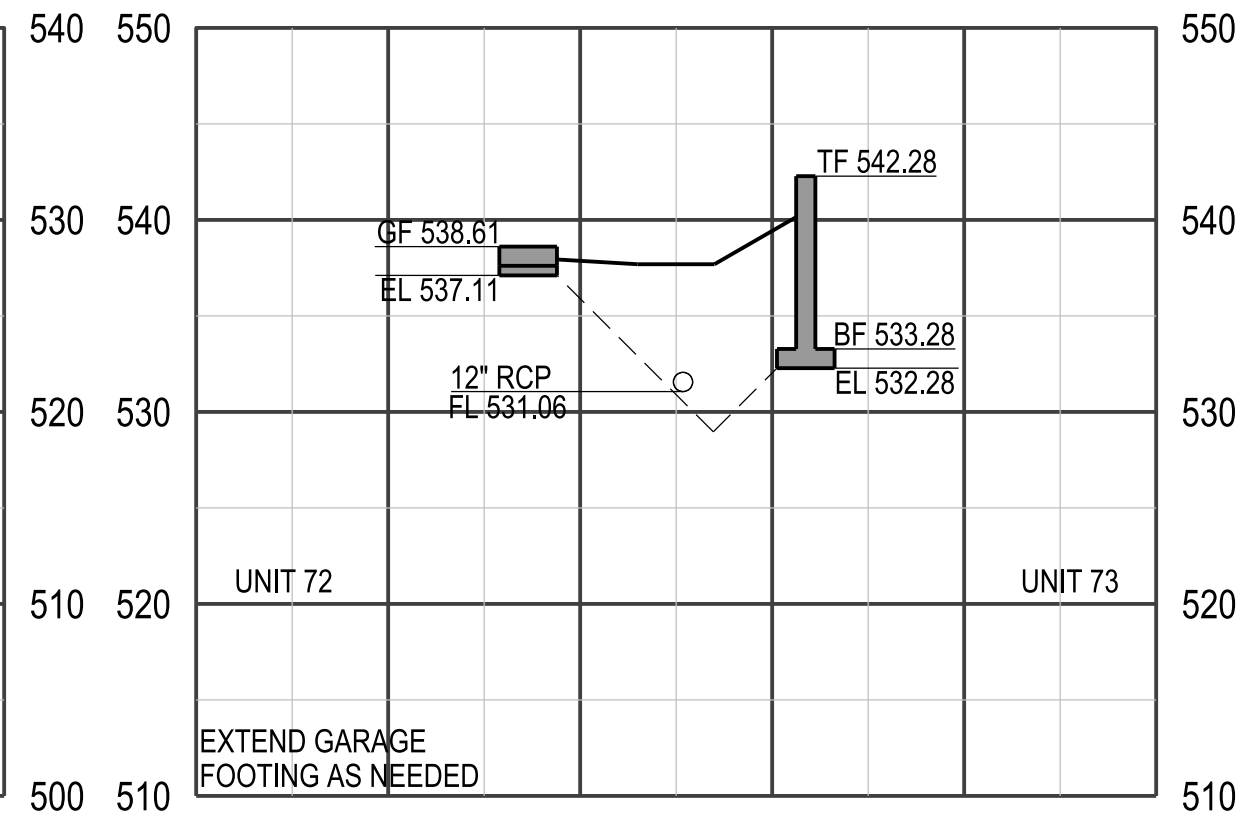
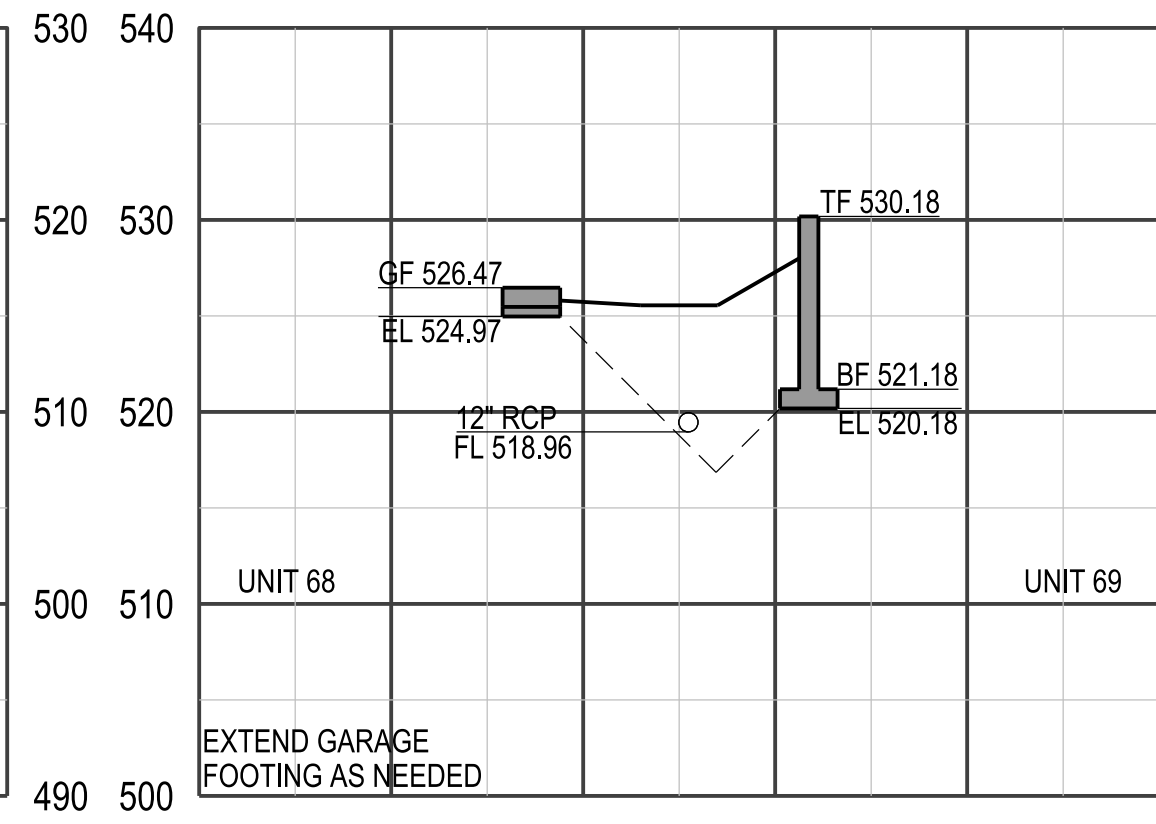
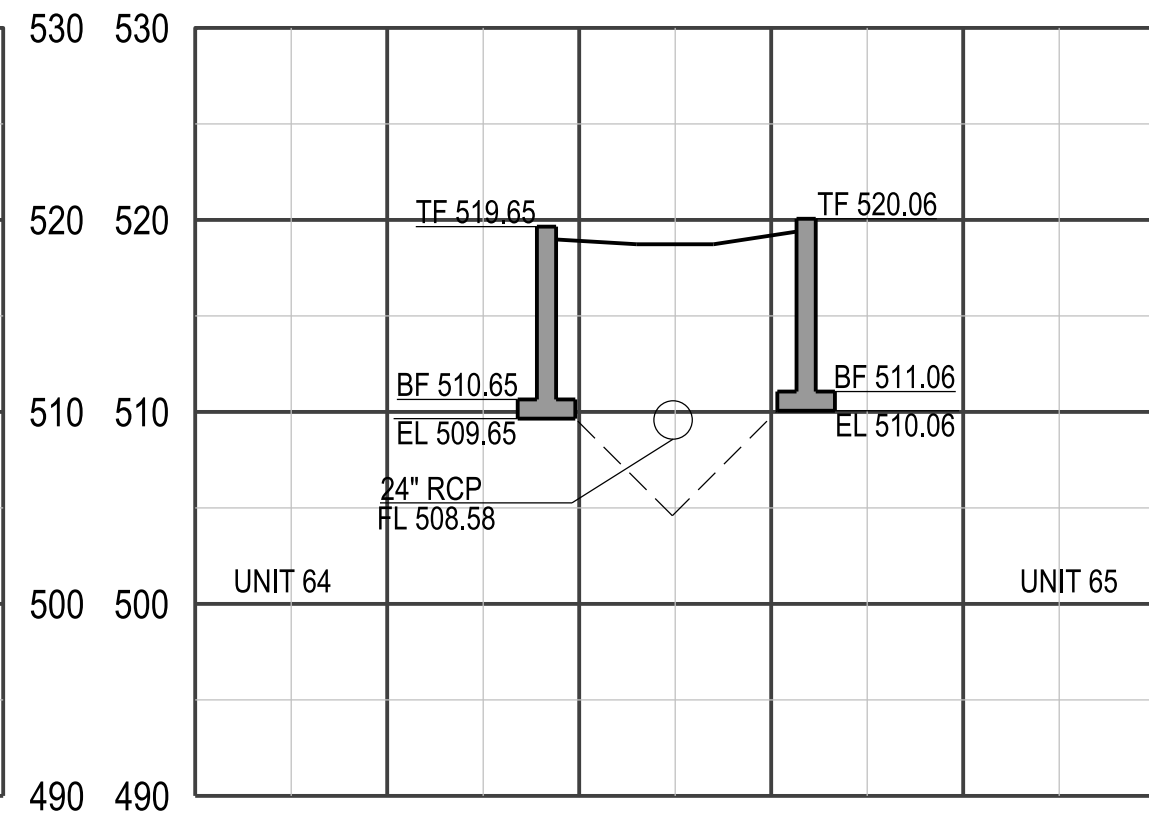
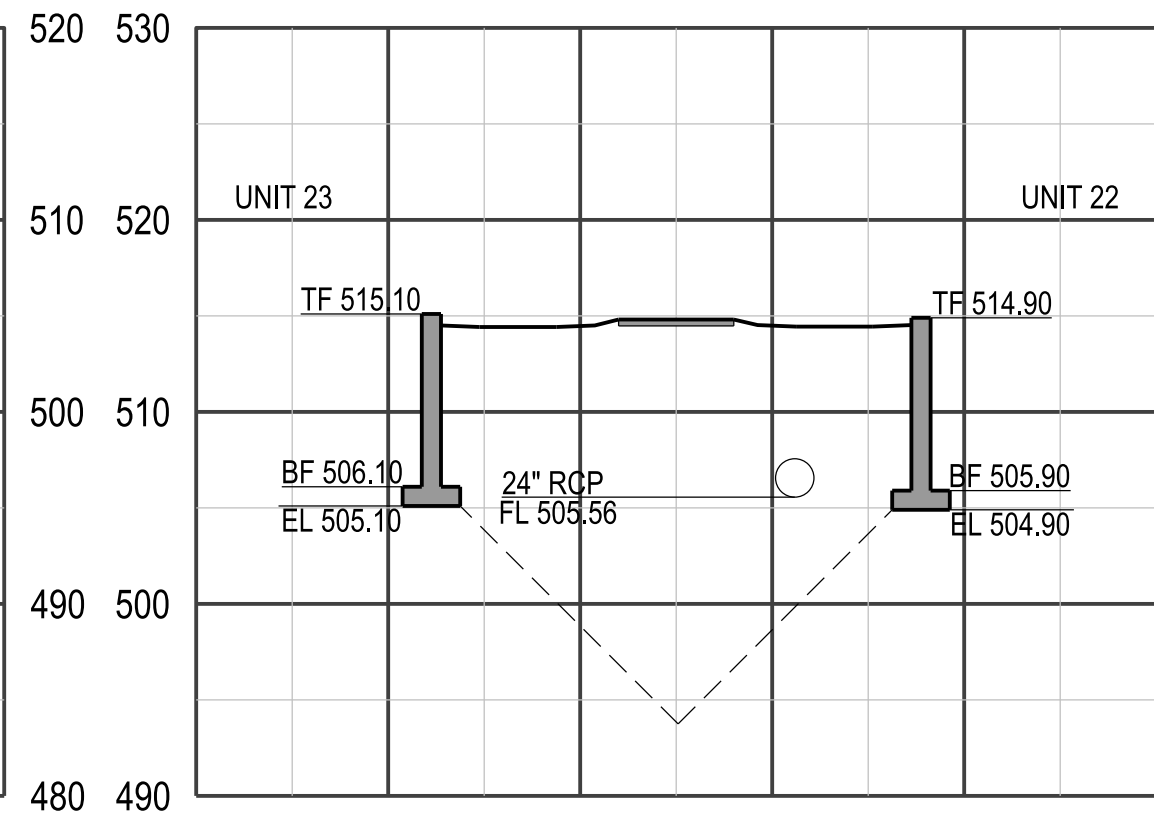
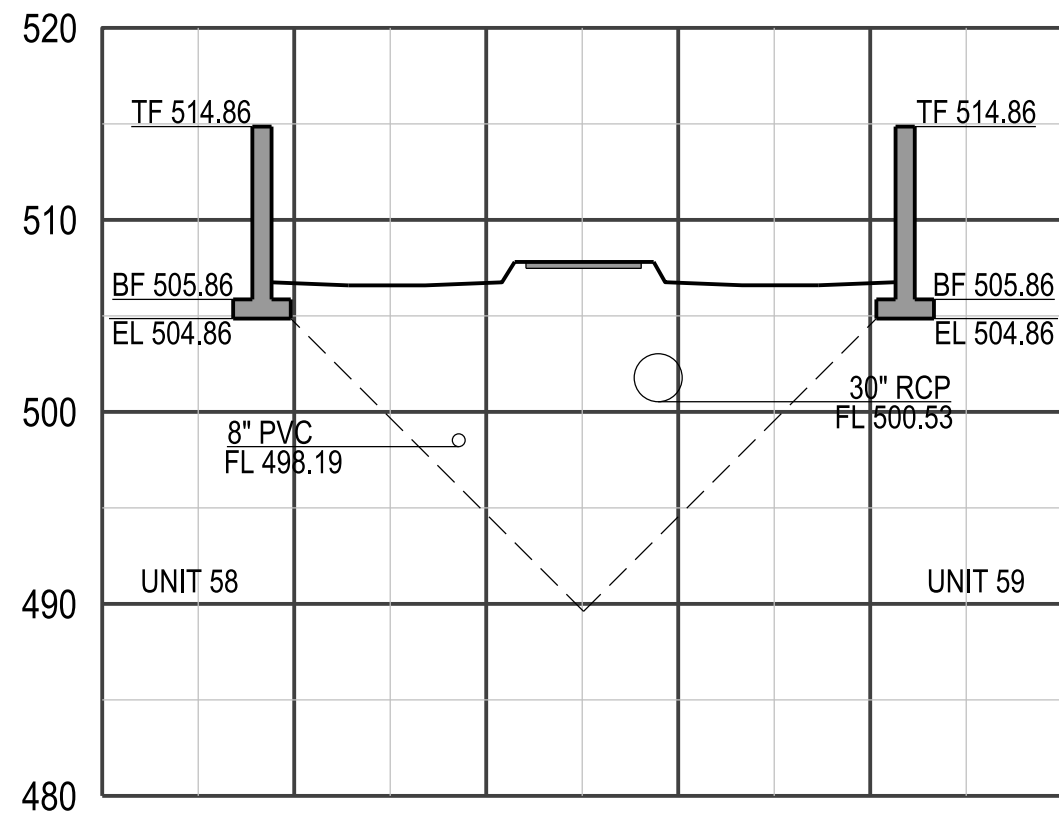
WET RETENTION POND
OUTFALL STRUCTURE

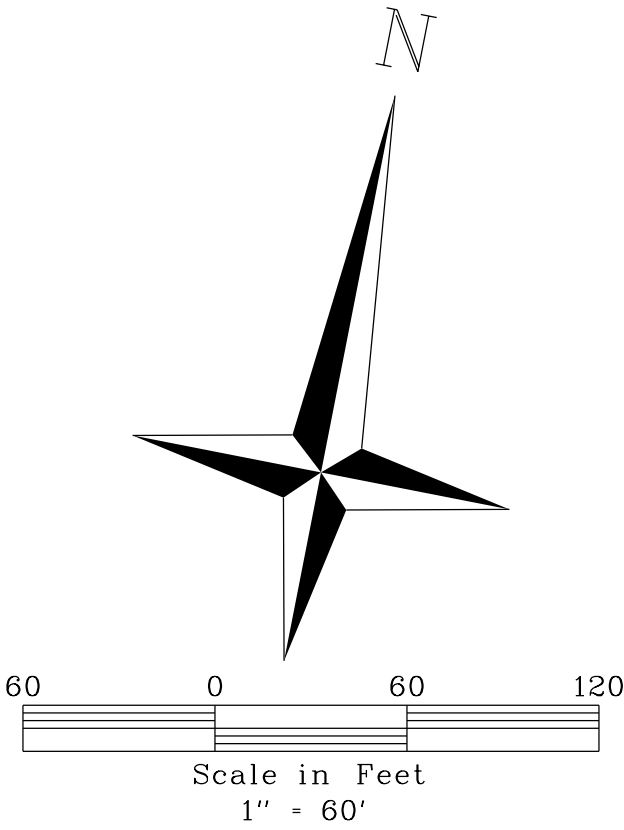
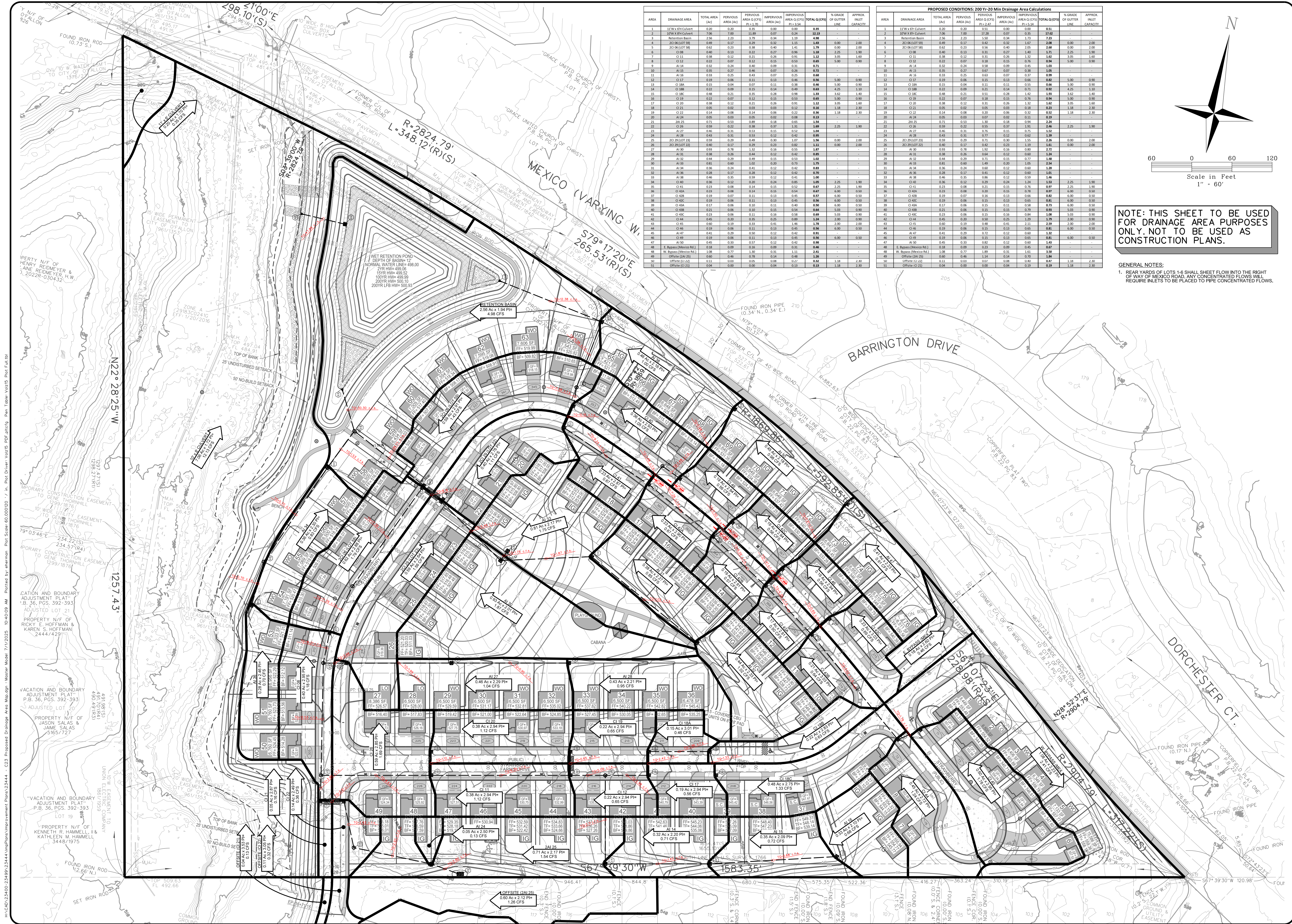
PARCEL ID: 2019-178-00-00245
CITY OF FALLON
PERMIT NO: 65925-00001
PROJECT #23444

8265 MEXICO ROAD,
OFALLON, MO 63376

07-01-25
C19

H:\CAD\23400\23400-23400-23444\img\Plot\Improvement\Plot\23444 - C21 Storm & Sanitary Sewer Influence Profiles.dgn - Master Model 7/1/2025 10:40:06 AM Plotted by sherman Plot Scale: 50,000/100 - / in. Plot Driver: Voids PDF plot5g Pen Table: Voids Pen Full.tbl





NOTE: THIS SHEET TO BE USED FOR DRAINAGE AREA PURPOSES ONLY. NOT TO BE USED AS CONSTRUCTION PLANS.

GENERAL NOTES:
1. REAR YARDS OF LOTS 1-6 SHALL SHEET FLOW INTO THE RIGHT OF WAY OF MEXICO ROAD, ANY CONCENTRATED FLOWS WILL REQUIRE INLETS TO BE PLACED TO PIPE CONCENTRATED FLOWS.

AREA	DRAINAGE AREA	TOTAL AREA (AC)	PERVIOUS AREA (AC)	IMPERVIOUS AREA (AC)	IMPERVIOUS AREA (AC)	IMPERVIOUS AREA (AC)	TOTAL Q (CFS)	% GRADE	APPROX. INLET CAPACITY
1	11'W x 6'W Culvert	0.20	0.20	0.00	0.00	0.00	0.35	-	-
2	10'W x 8'W Culvert	0.20	0.20	0.00	0.00	0.00	0.35	-	-
3	Retention Basin	2.56	2.23	0.34	1.19	4.88	-	-	-
4	20'W x 10'W Culvert	0.40	0.37	0.03	0.03	1.15	1.43	0.00	2.00
5	20'W x 10'W Culvert	0.40	0.37	0.03	0.03	1.15	1.79	0.00	2.00
6	CI 08	0.40	0.13	0.27	0.06	1.18	2.25	1.90	-
7	CI 11	0.38	0.12	0.21	0.06	1.12	3.05	1.90	-
8	CI 12	0.22	0.07	0.12	0.03	0.63	0.65	5.00	0.90
9	AI 14	0.32	0.24	0.40	0.09	0.31	0.71	-	-
10	AI 15	0.18	0.27	0.48	0.07	0.26	0.72	-	-
11	AI 16	0.33	0.25	0.43	0.07	0.25	0.68	-	-
12	CI 17	0.19	0.06	0.11	0.11	0.86	0.96	5.00	0.90
13	CI 18A	0.15	0.04	0.07	0.11	0.38	0.46	5.00	0.90
14	CI 18B	0.22	0.09	0.15	0.14	0.49	0.63	4.25	1.10
15	CI 18C	0.48	0.21	0.25	0.08	1.19	1.62	1.40	-
16	CI 19	0.22	0.07	0.12	0.15	0.53	0.65	5.00	0.90
17	CI 20	0.38	0.12	0.21	0.06	0.91	1.12	3.05	1.40
18	CI 21	0.05	0.00	0.00	0.12	0.16	1.18	2.30	-
19	CI 22	0.14	0.08	0.14	0.06	0.22	0.36	1.18	2.30
20	AI 24	0.06	0.00	0.00	0.00	0.08	0.14	-	-
21	AI 25	0.71	0.53	0.89	0.19	0.08	0.14	-	-
22	CI 26	0.99	0.22	0.38	0.57	1.31	1.69	2.25	1.90
23	AI 17	0.46	0.31	0.13	0.12	0.14	0.15	-	-
24	AI 18	0.43	0.31	0.53	0.12	0.42	0.95	-	-
25	CI 29 (LOT 23)	0.59	0.29	0.49	0.81	1.07	1.50	0.00	2.00
26	CI 29 (LOT 23)	0.40	0.17	0.29	0.81	0.82	1.11	0.00	2.00
27	AI 30	0.93	0.78	1.32	0.16	0.55	1.87	-	-
28	AI 31	0.38	0.26	0.44	0.13	0.43	0.85	-	-
29	AI 32	0.44	0.39	0.49	0.15	0.53	1.02	-	-
30	AI 33	0.81	0.60	1.02	0.20	0.73	1.75	-	-
31	AI 34	0.36	0.24	0.41	0.12	0.42	0.83	-	-
32	AI 36	0.28	0.17	0.28	0.12	0.42	0.70	-	-
33	AI 38	0.35	0.29	0.43	0.12	0.41	1.06	-	-
34	CI 40	0.36	0.12	0.20	0.24	0.85	1.05	2.25	1.90
35	CI 41	0.23	0.08	0.14	0.15	0.52	0.67	2.25	1.90
36	CI 42A	0.08	0.13	0.10	0.14	0.40	0.40	6.00	0.90
37	CI 42B	0.19	0.07	0.11	0.13	0.45	0.57	6.00	0.90
38	CI 42C	0.19	0.06	0.11	0.11	0.40	0.50	6.00	0.90
39	CI 43A	0.17	0.06	0.10	0.11	0.40	0.50	6.00	0.90
40	CI 43B	0.21	0.06	0.10	0.15	0.54	0.64	5.00	0.90
41	CI 43C	0.23	0.06	0.11	0.16	0.59	0.71	5.00	0.90
42	CI 44	0.45	0.30	0.35	0.25	0.89	1.24	2.00	0.90
43	CI 45	0.60	0.19	0.33	0.41	1.46	1.78	2.00	2.00
44	CI 46	0.19	0.06	0.11	0.13	0.45	0.56	6.00	0.90
45	AI 47	0.41	0.29	0.50	0.12	0.41	0.91	-	-
46	AI 48	0.39	0.33	0.11	0.15	0.40	0.56	6.00	0.90
47	AI 50	0.45	0.37	0.52	0.12	0.42	0.98	-	-
48	E Bypass (Mexico Rd.)	0.18	0.09	0.16	0.09	0.31	0.46	-	-
49	W Bypass (Mexico Rd.)	1.08	0.77	1.30	0.53	1.11	2.41	-	-
50	Office (2A/25)	0.60	0.46	0.78	0.14	0.48	1.28	-	-
51	Office (CI 23)	0.11	0.01	0.05	0.08	0.27	0.32	1.18	2.30

PROPOSED CONDITIONS: 200 Yr-20 Min Drainage Area Calculations									
AREA	DRAINAGE AREA	TOTAL AREA (AC)	PERVIOUS AREA (AC)	IMPERVIOUS AREA (AC)	IMPERVIOUS AREA (AC)	IMPERVIOUS AREA (AC)	TOTAL Q (CFS)	% GRADE	APPROX. INLET CAPACITY
1	11'W x 6'W Culvert	0.20	0.20	0.00	0.00	0.00	0.51	-	-
2	10'W x 8'W Culvert	0.20	0.20	0.00	0.00	0.00	0.51	-	-
3	Retention Basin	2.56	2.23	0.34	1.19	7.23	-	-	-
4	20'W x 10'W Culvert	0.40	0.37	0.03	0.03	1.67	2.08	0.00	2.00
5	20'W x 10'W Culvert	0.40	0.37	0.03	0.03	1.67	2.09	0.00	2.00
6	CI 08	0.40	0.13	0.27	0.06	1.40	1.71	2.25	1.90
7	CI 11	0.38	0.12	0.21	0.06	1.32	1.82	1.90	1.40
8	CI 12	0.22	0.07	0.12	0.03	0.76	0.94	5.00	0.90
9	AI 14	0.32	0.24	0.38	0.09	0.45	1.03	-	-
10	AI 15	0.18	0.27	0.47	0.07	0.38	1.05	-	-
11	AI 16	0.33	0.25	0.43	0.07	0.37	0.99	-	-
12	CI 17	0.19	0.06	0.11	0.11	0.66	0.82	5.00	0.90
13	CI 18A	0.15	0.04	0.07	0.11	0.35	0.44	5.00	0.90
14	CI 18B	0.22	0.09	0.21	0.14	0.71	0.92	4.25	1.10
15	CI 18C	0.48	0.21	0.25	0.08	1.42	1.93	1.40	1.40
16	CI 19	0.22	0.07	0.12	0.15	0.76	0.94	5.00	0.90
17	CI 20	0.38	0.12	0.21	0.06	1.12	1.62	3.05	1.40
18	CI 21	0.05	0.00	0.00	0.12	0.16	0.23	1.18	2.30
19	CI 22	0.14	0.08	0.20	0.06	0.32	0.52	1.18	2.30
20	AI 24	0.06	0.00	0.00	0.00	0.08	0.14	-	-
21	AI 25	0.71	0.53	0.89	0.19	0.08	0.14	-	-
22	CI 26	0.99	0.22	0.38	0.57	1.91	2.46	2.25	1.90
23	AI 17	0.46	0.31	0.76	0.15	0.75	1.52	-	-
24	AI 18	0.43	0.31	0.77	0.12	0.62	1.39	-	-
25	CI 29 (LOT 23)	0.59	0.29	0.71	0.80	1.55	2.26	0.00	2.00
26	CI 29 (LOT 23)	0.40	0.17	0.42	0.81	1.19	1.61	0.00	2.00
27	AI 30	0.93	0.78	1.32	0.16	0.80	2.72	-	-
28	AI 31	0.38	0.26	0.44	0.13	0.43	0.86	-	-
29	AI 32	0.44	0.39	0.71	0.15	0.77	1.48	-	-
30	AI 33	0.81	0.60	1.49	0.20	1.05	2.54	-	-
31	AI 34	0.36	0.24	1.00	0.12	0.60	1.29	-	-
32	AI 36	0.28	0.17	0.41	0.12	0.60	1.01	-	-
33	AI 38	0.46	0.39	0.66	0.12	0.59	1.44	-	-
34	CI 40	0.36	0.12	0.20	0.24	1.24	1.53	2.25	1.90
35	CI 41	0.23	0.08	0.21	0.15	0.76	0.97	2.25	1.90
36	CI 42A	0.08	0.13	0.10	0.14	0.40	0.40	6.00	0.90
37	CI 42B	0.19	0.07	0.16	0.13	0.66	0.82	6.00	0.90
38	CI 42C	0.19	0.06	0.15	0.13	0.65	0.81	6.00	0.90
39	CI 43A	0.17	0.06	0.10	0.11	0.48	0.71	6.00	0.90
40	CI 43B	0.21	0.06	0.15	0.15	0.79	0.98	5.00	0.90
41	CI 43C	0.23	0.06	0.15	0.16	0.84	1.00	5.00	0.90
42	CI 44	0.45	0.30	0.50	0.25	1.29	1.79	2.00	0.90
43	CI 45	0.60	0.19	0.48	0.41	2.11	2.59	2.00	2.00
44	CI 46	0.19	0.06	0.13	0.13	0.65	0.81	6.00	0.90
45	AI 47	0.41	0.29	0.72	0.12	0.60	1.32	-	-
46	AI 48	0.39	0.33	0.55	0.13	0.40	0.91	6.00	0.90
47	AI 50	0.45	0.37	0.55	0.12	0.60	1.43	-	-
48	E Bypass (Mexico Rd.)	0.18	0.09	0.23	0.09	0.45	0.67	-	-
49	W Bypass (Mexico Rd.)	1.08	0.77	1.89	0.11	1.11	3.59	-	-
50	Office (2A/25)	0.60	0.46	1.14	0.14	0.70	1.84	-	-
51	Office (CI 23)	0.11	0.01	0.07	0.08	0.40	0.47	1.18	2.30

McKelvey
H O M E S
Building A Tradition Of Excellence Since 1888

421 S. WOODS MILL RD., SUITE 110
TOWN & COUNTRY, MISSOURI 63071

VOLZ
Incorporated
10849 Indiana Road, Suite 100, St. Louis, MO 63122
Ph: 314.426.6212 Main
314.890.1250 Fax
www.volzinc.com
Authority #203

TIMOTHY J MEYER
Professional Engineer
MO E-24665

PROPOSED DRAINAGE AREA MAP

PARCEL ID: 24019-178-00-00245
CITY OF OFALLON
PERMIT NO: 8592-00001
PROJECT #23444

07-01-23
C23

STRAWBERRY FARMS

8265 MEXICO ROAD,
OFALLON, MO 63376



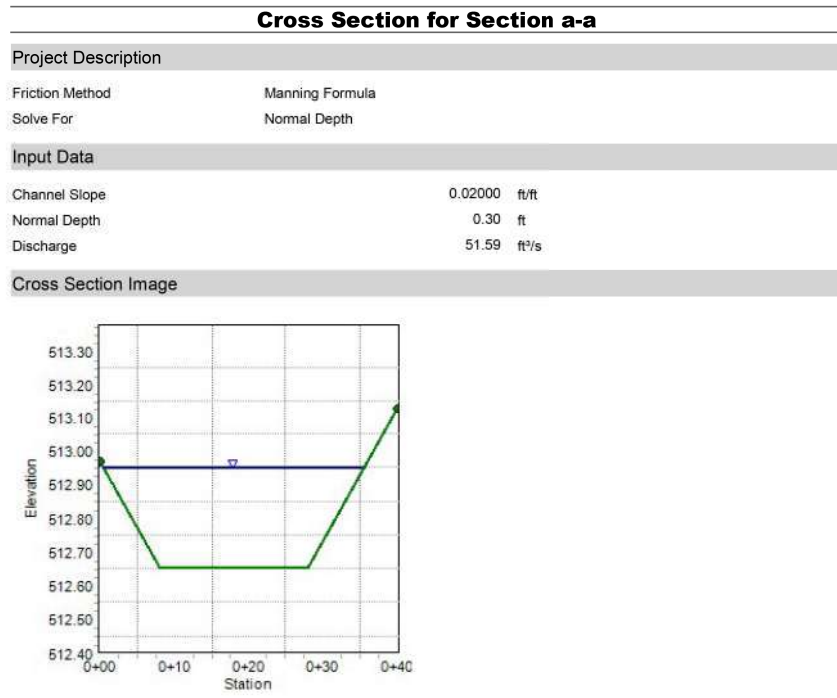
CALL MISSOURI ONE CALL SYSTEMS INC.
TWO FULL WORKING DAYS IN ADVANCE
OF STARTING WORK.



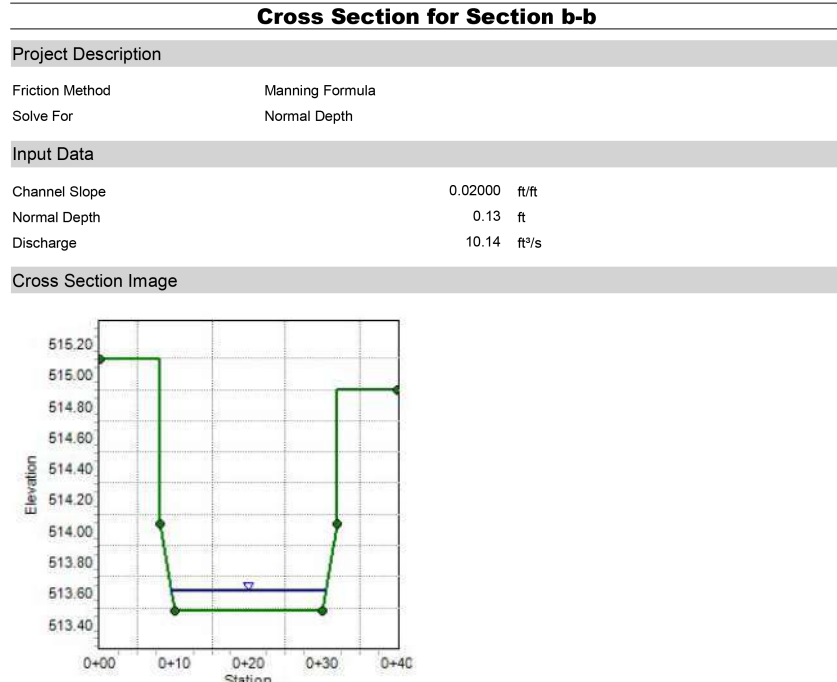
MISSOURI ONE-CALL 1-800-344-7483

THE UNDERGROUND UTILITIES SHOWN HEREIN WERE PLOTTED FROM AVAILABLE INFORMATION AND FIELD MARKINGS PROVIDED BY THE MEMBER UTILITIES OF THE MISSOURI ONE CALL SYSTEM (1-800-8DIGIT), AND DO NOT NECESSARILY REFLECT THE ACTUAL EXISTENCE, NONEXISTENCE, SIZE, TYPE, CAPACITY, NUMBER, OR LOCATION OF THESE OR OTHER UTILITIES, NOR THE ABILITY TO SERVE THE EXISTING OR INTENDED USES OF THIS OR ADJACENT SITES. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THE ACTUAL LOCATION OF ALL UNDERGROUND UTILITIES PRIOR TO THE START, SHOWING, OR PRIOR TO ANY GRAVING, EXCAVATION, OR CONSTRUCTION OF IMPROVEMENTS. THESE PROVISIONS SHALL IN NO WAY ABSOLVE ANY PARTY FROM COMPLYING WITH THE UNDERGROUND FACILITY SAFETY AND DAMAGE PREVENTION ACT, CHAPTER 319, RSMO.

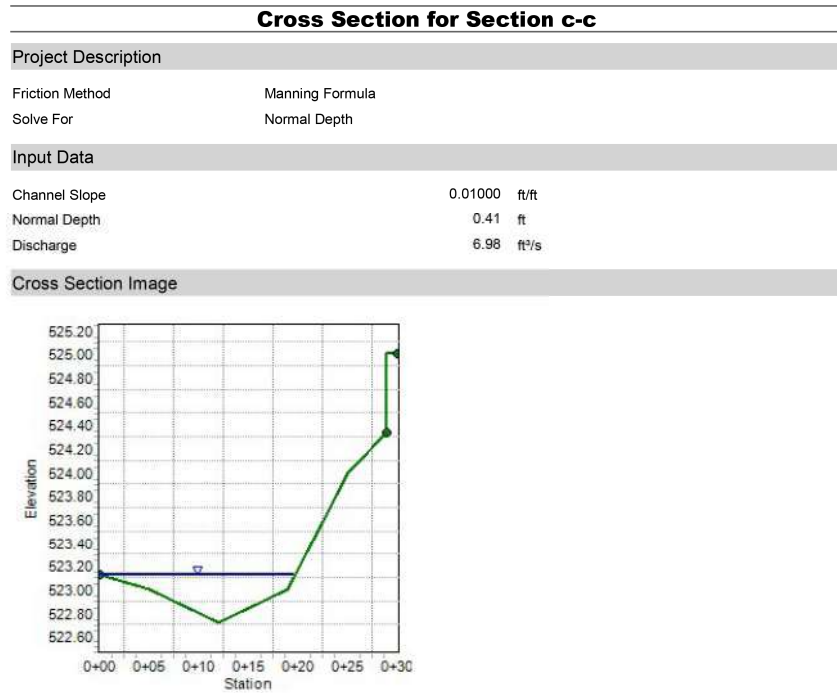
H:\CAD\234500-234499-234443\img\pos\improvement\Plan\23444 - C26 Emergency Relief Path Cross-Sections.dgn - Water Model 7/1/2025 10:40:20 AM Plotted by emerson Plot Scale: 50.000000 - / in Plot Driver: Vortis PDF plotter Pen Table: Vortis5 Plot Font: tcl



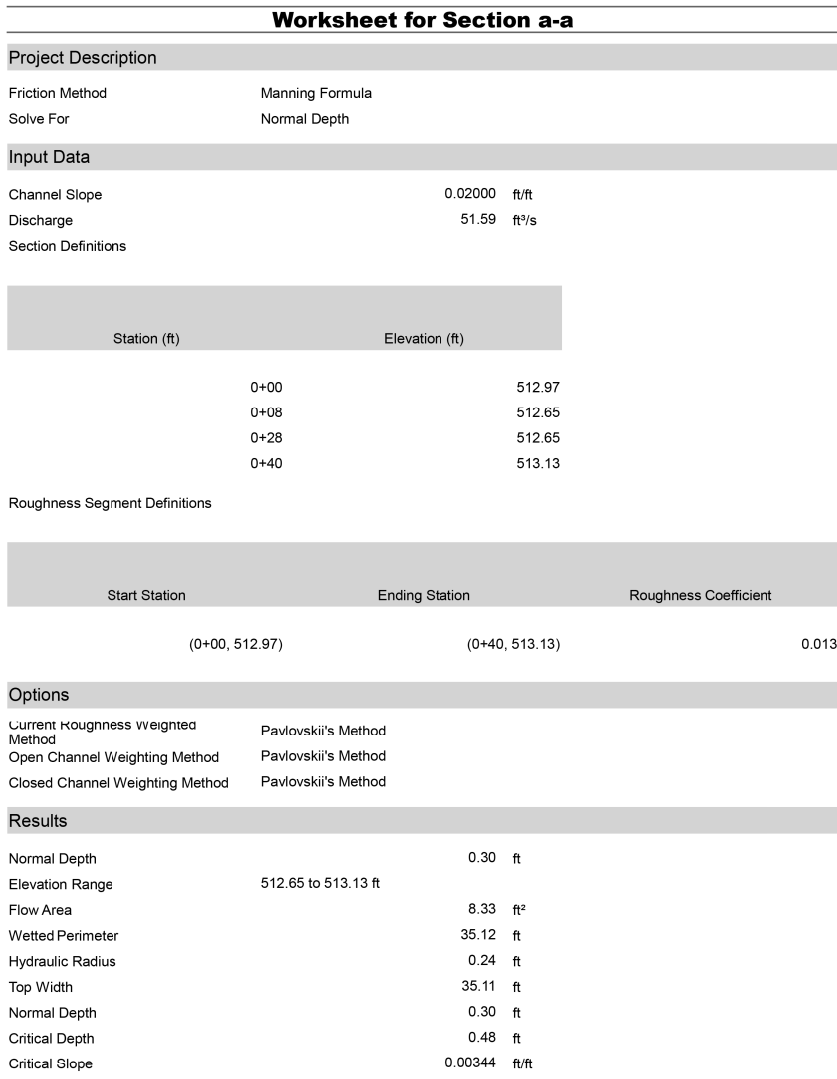
Bentley Systems, Inc. Hydraulic Methods Software/HydroMaster V8i (SELECTSeries 1) [08.11.01.03]
4/30/2025 6:34:48 AM 27 Siemens Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Page 1 of 1



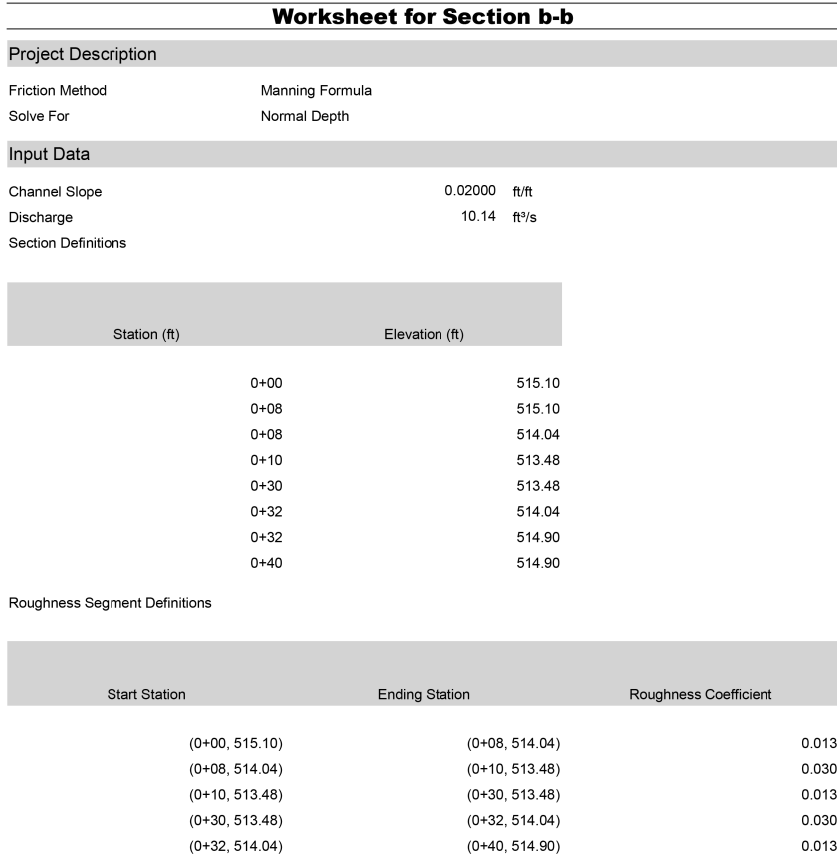
Bentley Systems, Inc. Hydraulic Methods Software/HydroMaster V8i (SELECTSeries 1) [08.11.01.03]
4/30/2025 6:37:48 AM 27 Siemens Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Page 1 of 1



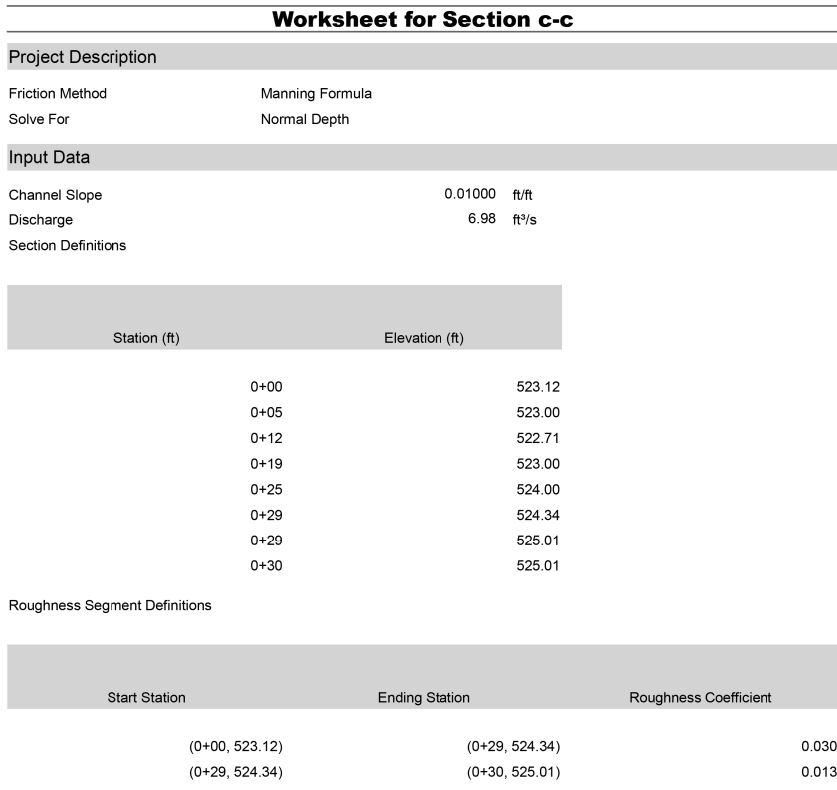
Bentley Systems, Inc. Hydraulic Methods Software/HydroMaster V8i (SELECTSeries 1) [08.11.01.03]
1/28/2025 10:37:59 AM 27 Siemens Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Page 1 of 1



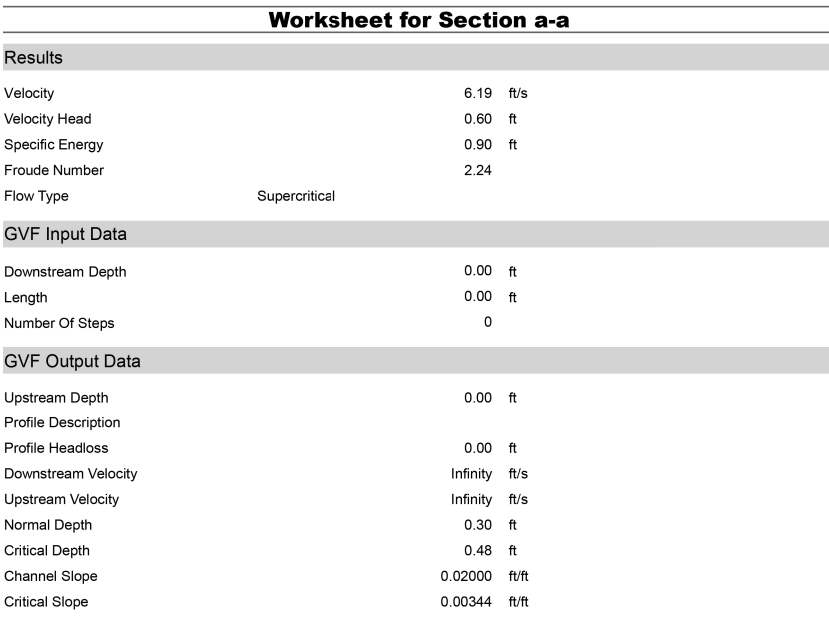
Bentley Systems, Inc. Hydraulic Methods Software/HydroMaster V8i (SELECTSeries 1) [08.11.01.03]
4/30/2025 6:34:28 AM 27 Siemens Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Page 2 of 2



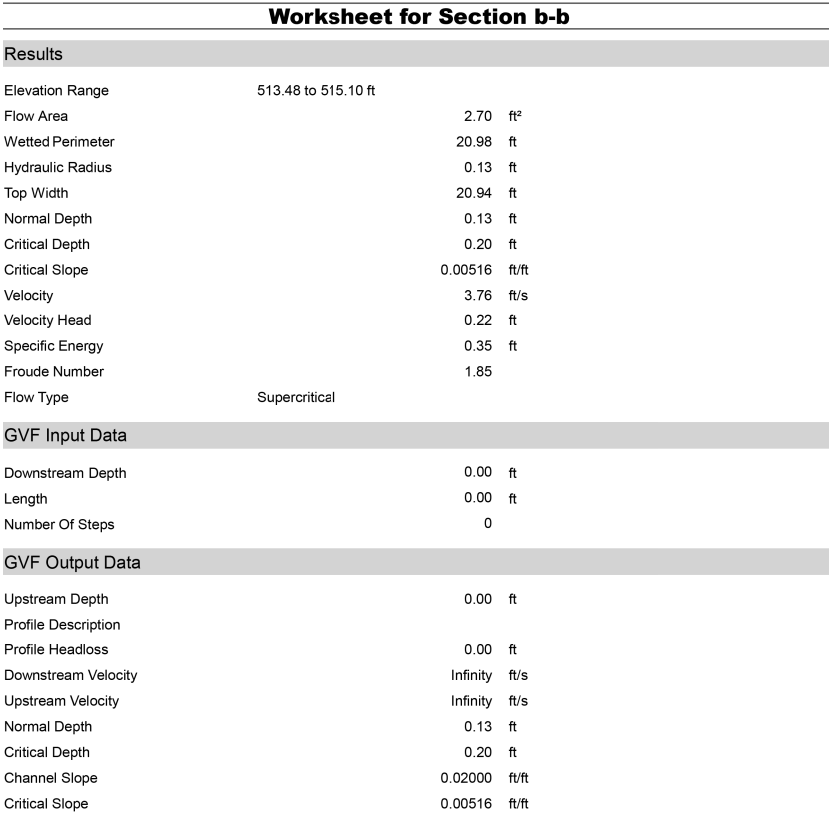
Bentley Systems, Inc. Hydraulic Methods Software/HydroMaster V8i (SELECTSeries 1) [08.11.01.03]
4/30/2025 6:37:23 AM 27 Siemens Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Page 1 of 2



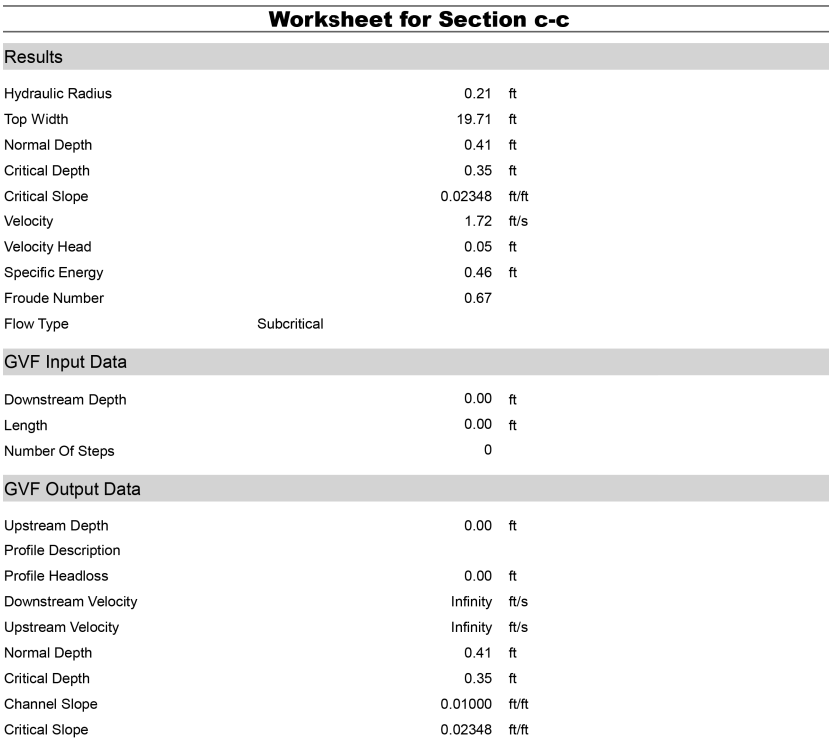
Bentley Systems, Inc. Hydraulic Methods Software/HydroMaster V8i (SELECTSeries 1) [08.11.01.03]
1/28/2025 10:37:42 AM 27 Siemens Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Page 2 of 2



Bentley Systems, Inc. Hydraulic Methods Software/HydroMaster V8i (SELECTSeries 1) [08.11.01.03]
4/30/2025 6:34:28 AM 27 Siemens Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Page 2 of 2



Bentley Systems, Inc. Hydraulic Methods Software/HydroMaster V8i (SELECTSeries 1) [08.11.01.03]
4/30/2025 6:37:23 AM 27 Siemens Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Page 2 of 2



Bentley Systems, Inc. Hydraulic Methods Software/HydroMaster V8i (SELECTSeries 1) [08.11.01.03]
1/28/2025 10:37:42 AM 27 Siemens Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Page 2 of 2

EMERGENCY RELIEF PATH
CROSS-SECTIONS

PARCEL ID: 23175-1756-00-00-45
CITY OF OFFALLON
PERMIT No: ESF25-00001
PROJECT #23444

8265 MEXICO ROAD,
OFFALLON, MO 63376

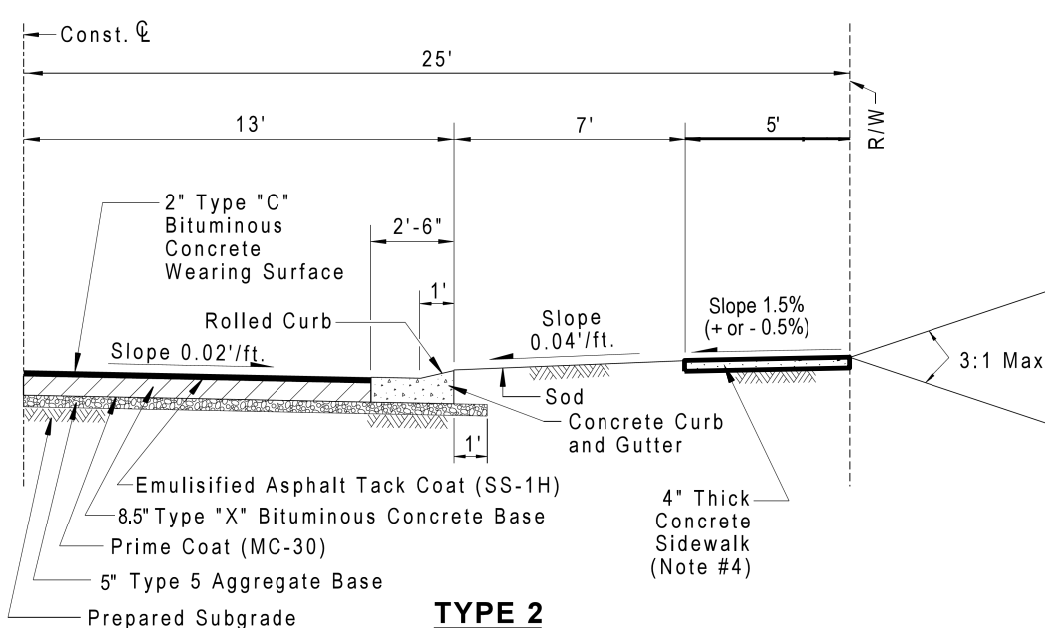
07-01-23
C26

STRAWBERRY FARMS

7/1/25
TIMOTHY JOHN MEYER
NUMBER
E-24665
TIMOTHY J MEYER
Professional Engineer
MO E-24665

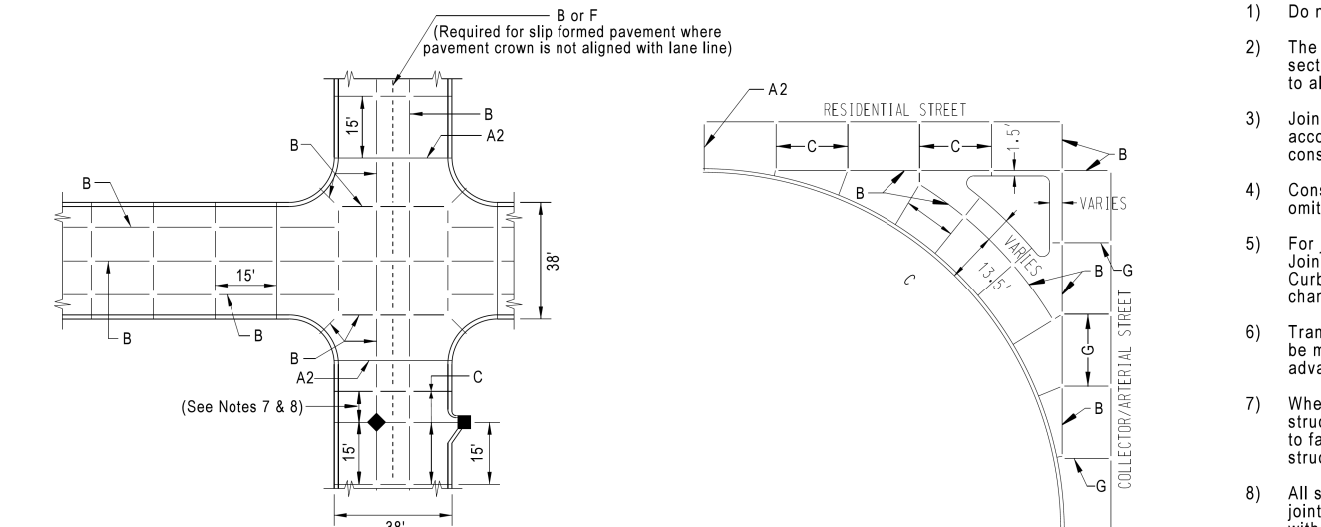
VOLZ
Incorporated
10849 Indiana Road, Independence, MO 64055
St. 314-426-6212 Main
314-890-1250 Fax
www.volzinc.com
Authority #203

McKelvey
HOMES
Building A Tradition Of Excellence Since 1888
424 S. WOODS MILL RD., SUITE 110
TOWN & COUNTRY, MISSOURI 63017

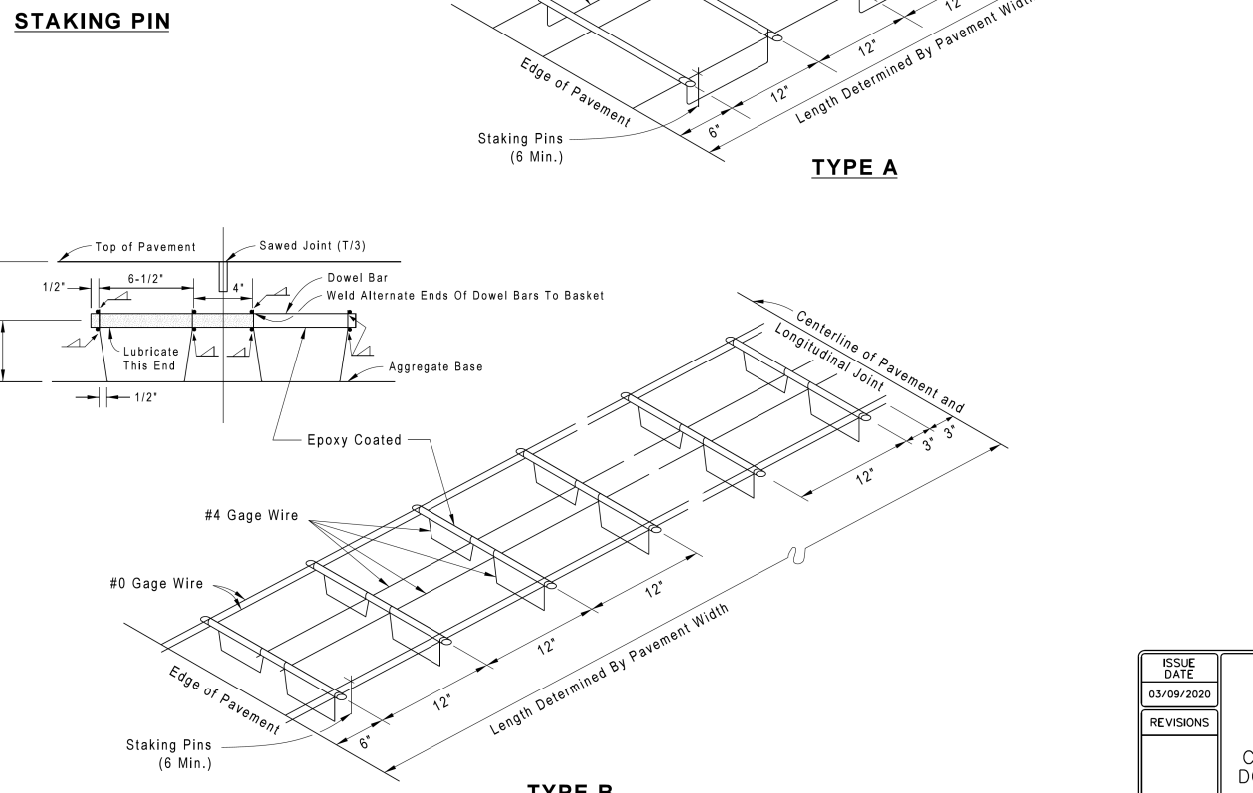


- 1) Do not scale drawings, follow dimensions.
- 2) Sections shown are symmetrical about construction line
- 3) All necessary pavement markings shall be as required by St. Louis County. For details refer to "Guide Lines for Pavement Markings and Lane Striping", Standard Drawing G902.10.
- 4) Sidewalk thickness increases to 6" at through driveway entrances and for the first section of sidewalk on each side of the driveway entrance.
- 5) Easements, when required, shall be established as a "Normal Roadway Improvement, Maintenance, Utility Sewer and Sidewalk Easement," and dedicated to St. Louis County.

ISSUE DATE 03/09/2018 REVISIONS	THIS IS NOT A CERTIFIED DOCUMENT	Saint Louis COUNTY TRANSPORTATION ONE SOUTH OXFORD ST. LOUIS, MISSOURI 63102	STANDARD DRAWING RESIDENTIAL STREET (50' R/W - 26' PVMT.) (TYPE 1 & TYPE 2)	EFFECTIVE 05/01/2020 SHEET 1 OF 1	DRAWING C203.82
--	---	---	---	--	----------------------------------



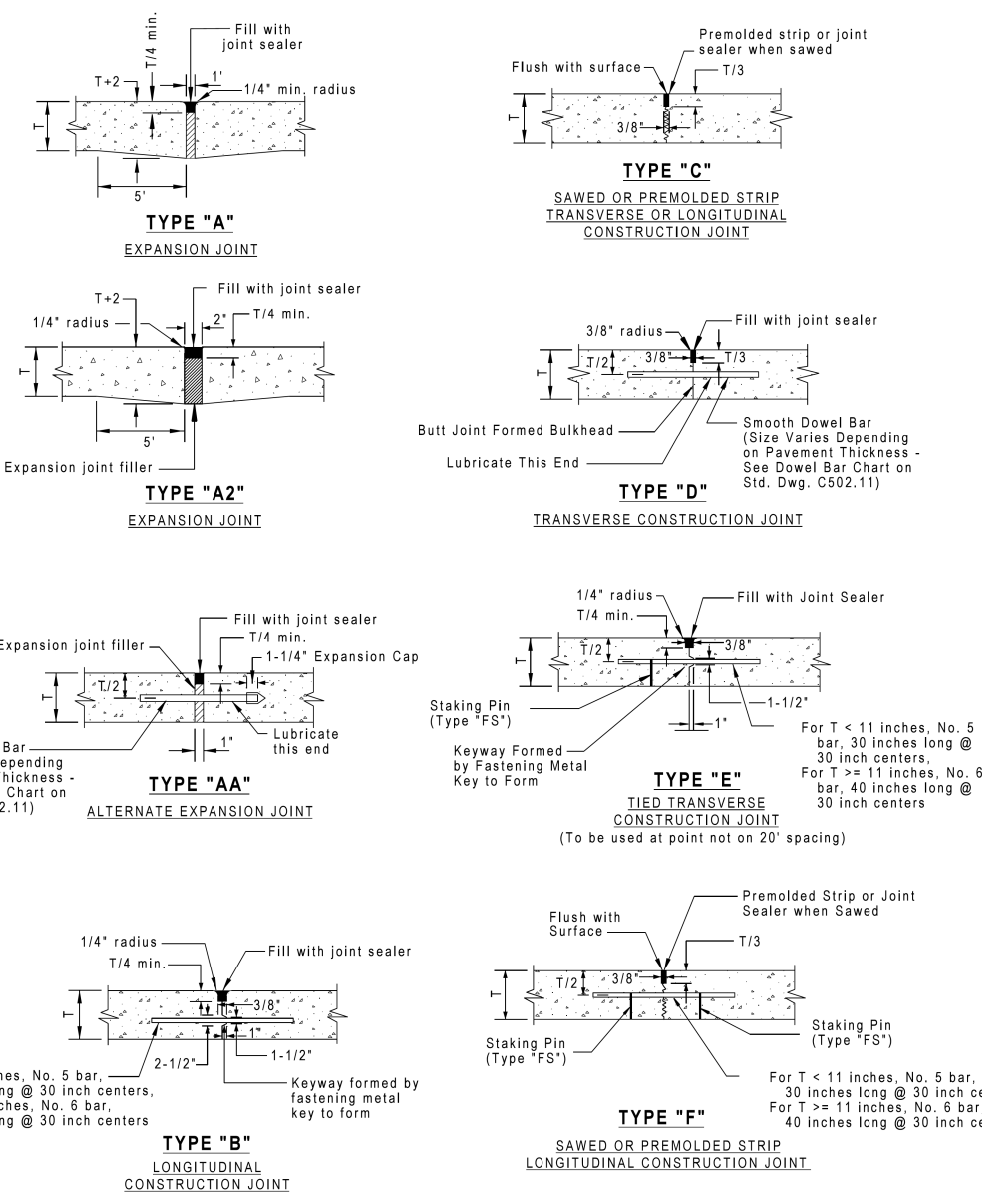
- 2) Do not scale drawing, follow dimensions.
- 3) The joint layouts should show correspondence with the Department's standard typical sections. The actual locations of longitudinal construction joints shall be set out on the plan. The center line of the road shall be shown.
- 4) Joint layout within intersections shall be adjusted as necessary to accommodate the intersection channelization details in the approved construction plan.
- 5) Construction joints and tie bars associated with curb construction may be omitted when curb is required monolithic with pavement.
- 6) For joint and tie bar requirements refer to the "Pavement Construction Details - Joints" (Standard Drawing C202.13) and "Pavement Construction Details - Tie Bars" (Standard Drawing C202.14). Note that the width and location may change the type of joint required.
- 7) Transverse or longitudinal construction joints in slig formed pavements may be omitted when a groove or tie bar, if such device has been approved in advance by the Department.
- 8) When a joint falls within 5ft. of or contacts inlets, manholes, or other structures, the contractor shall be required to either side of opening to construct joint to fall on round structures and tie or bar between corners of rectangular.
- 9) All street inlets shall be separated from the pavement and curb by expansion joint material extending completely through curb and slab. Manhole casting



- 2) Do not scale drawing. Follow dimensions.
- 3) Approved for use with transverse joints.
- 4) The cover supporting equipment shall be factory assembled and capable of holding the dowsels in their required positions. In the completed joint installation, no dowsels shall vary from the required spacing more than 1/8 inch (3 mm).
- 5) The cover spacing shall be 12 inches on centers, beginning with the first dowel.
- 6) The dowel spacing shall be 12 inches on centers, beginning with the first dowel.
- 7) Basket skaling pins shall be fabricated from #10 gauge (0.308 inch) wire with suitably welded heads. Skaling pins shall be of the minimum length required to hold the Type-A or Type-B assemblies shall be provided unless approved otherwise. The skaling pins shall be placed in the mid-point of each side of the basket assembly.
- 8) Wires, bars, or clips shall be used as necessary to strengthen joints.
- 9) Minor variations in the configuration of the support units will be allowed.
- 10) The final length of the dowel for a length of at least 11 inches shall be specified with an approved grouting grade concrete or grout. The dowels shall be installed in accordance with Standard Specifications or may substitute composite basket units with an approved tested bracket solution conforming to Section 1057.3.2.

DOWEL BARS - EPOXY COATED		
PAVEMENT THICKNESS*	BAR SIZE	
	DIAMETER	LENGTH
7 inches	1 inch	18 inches
8 inches	1 inch	18 inches
9 inches	1-1/4 inches	18 inches
10 inches	1-1/4 inches	18 inches
11 inches	1-1/2 inches	18 inches

ISSUE DATE 03/09/2020 REVISIONS	THIS IS NOT A CERTIFIED DOCUMENT	Saint Louis COUNTY TRANSPORTATION 1050 NORTH LINCOLN BLVD. ST. LOUIS, MISSOURI 63102	PAVEMENT CONSTRUCTION DETAILS DOWEL SUPPORTING UNITS	
		EFFECTIVE 05/01/2020	SHEET 1 OF 1	DRAWING C502.11



TYPE "FS" STAIR
PIN DETAIL

JOINT FILLING AND SEALING MATERIAL SHALL MEET THE FOLLOWING:	
Material	Standard Specification
Hot Poured Elastic Joint Sealant	Section 1057.5.2
Backer Rod Material for Hot Poured Elastic Joint Sealant ¹	Section 1057.5.3
Polysulfide Elastic Joint Sealant	Section 1057.6
Pure Polyurethane Expansion Joint Filler ²	Section 1057.5.2
Pure Polyurethane Expansion Joint Filler ²	Section 1057.5.4
Backer Rod Material for Saw Cut and Formed Joints	Section 1057.11
Backer Rod Material for Silicone Joint Sealant	Section 1057.12

¹ Backer rod required for material with 2 to 6 inch size.

² The joint filler material thickness shall be 2 inches across for concrete pavement Type A expansion joints, 1 inch across the intersection of concrete pavement and concrete structures and 1/2 inch for all expansion joints. 3/4-inch across sawed street spans. 1/2-inch across concrete expansion joints. The joint filler material shall be applied in a continuous layer of a width equal to the full depth of the pavement less the depth of the reinforcement and shall be applied to the full depth of the joint on the face of the inlet, shoulder or less than the pavement surface width.

- 1) Do not scale drawing, follow dimensions.
- 2) All deflected bars for joints and curls shall be in accordance with section 1057.4 of the "St. Louis County Standard Specifications" manual.
- 3) Transverse or longitudinal construction joints in slip form pavements may be made with a groover or tool, if such device has been approved in advance by the Department.

SAW CUTTING

When a standard (water cooled diamond bladed) concrete saw is used to cut the transverse or longitudinal joint when the concrete has reached 95% of its design compressive strength (before final seal) the following applies:

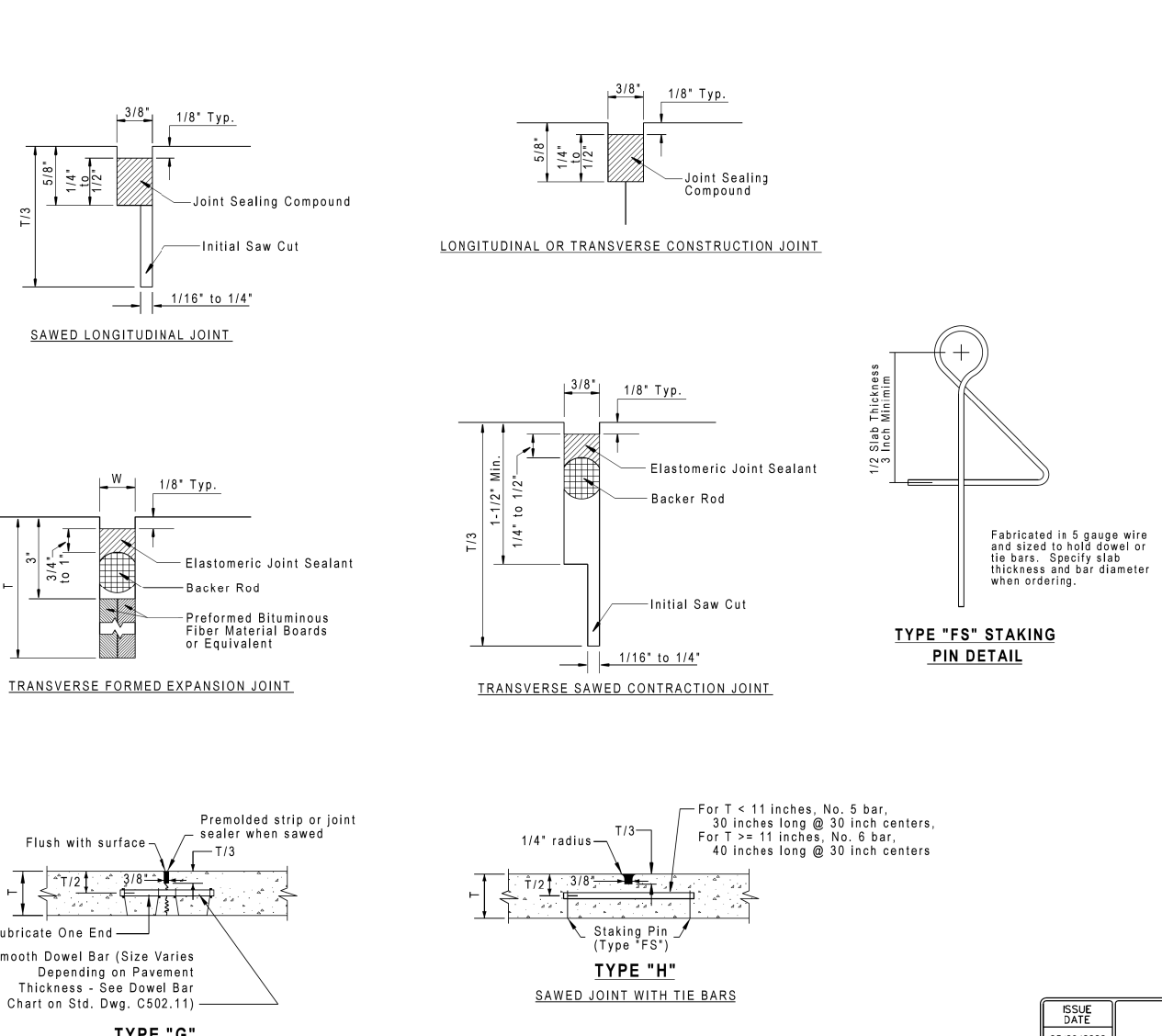
- For pavement < 7 inches thick, saw the joint to a minimum depth of one-fourth (1/4) the specified pavement thickness.
- For pavements > 7 inches thick, saw the joint to a minimum depth of one third (1/3) the specified pavement thickness.
- Saw joints 3/8 inch = 1/16 inch wide as measured at the time of sawing.

- When a standard (water cooled diamond bladed) concrete saw is used to cut the transverse or longitudinal joint when the pavement reaches 950 psi compressive strength [before final set] the following applies:
 - For pavements > 7 inches thick, saw the joint to a minimum depth of one-fourth (1/4) the specified pavement thickness.
 - For pavements > 7 inches thick, saw the joint to a minimum depth of one-third (1/3) the specified pavement thickness.
- Saw joints 3/8 inch - 1/6 inch wide as measured at the time of sawing.

When using early-entry (dry cut, light weight) saws, only use saw blades and sled plates as recommended by the manufacturer. Perform the early entry sawing reaches [before final set] to achieve the necessary strength (after initial set and before final set) as follows:

- Saw the joint 2-1/4 to 2-1/2 inches deep.
- Saw joints approximately 1/8 inch - 1/16 inch wide as measured at the time of sawing.

ISSUE DATE 03/09/2020 REVISIONS	THIS IS NOT A CERTIFIED DOCUMENT	 4850 NORTH LINCOLN BLVD. ST. LOUIS, MISSOURI 63122	PAVEMENT CONSTRUCTION DETAILS CONCRETE JOINTS		
			EFFECTIVE 05/01/2020	SHEET 1 OF 2	DRAWING C502.13



TYPE "FS" STAKING
BIN DETAIL

JOINT FILLER AND SEALING MATERIAL		
SHALL MEET THE FOLLOWING:		
Materials	Standard	Specification
Hot-Poured Elastic Joint Sealant	Section 0575	Section 0575.2.1
Backer Rod for Hot-Poured Elastic Joint Sealant ¹	Section 0575	Section 0575.2.1
Polyurethane Elastomeric Joint Sealant	Section 0575	Section 0575.2.6
Preformed Fiber Expansion Joint Filler ²	Section 0575	Section 0575.2.2
Preformed Spigongee Rubber Expansion Joint Filler	Section 0575	Section 0575.2.3
Silicone Joint Sealant for Set Cast and Formed Joints	Section 0575	Section 0575.11.1
Backer Rod Material for Silicone Joint Sealant ³	Section 0575	Section 0575.12.1

¹ Backer rod not required for joint width = 0.25 inch.

² The joint filler material thickness shall be 2 inches across for concrete pavement joints. Expansion joints: 1 inch across the thickness of concrete sidewalk and concrete entrances and 2 inches for A or AA expansion joints. 3/4 inch across the thickness of concrete sidewalk, concrete sidewalk or of the joint thickness specified. The joint filler material shall be of a width equal to the full depth of the pavement less the depth of the joint sealer and 1/2 inch less than the following:

- 2) Do not cast concrete during freezing dimensions.
- 3) All delivered forms for joints and curls shall be in accordance with ASTM 1058 and ACI 308.1 (Joint Curing Standard Specifications) manual.
- 4) Temperature of concrete shall be maintained in site formed pavements "as may be made with a prover or hot air," if such device has been approved in advance by the Department.

SAW CUTTING

When a standard (water cooled diamond blade) concrete saw is used to cut the joints, the following procedure when pavement thickness 150 psi compressive strength (1000 psi flexural strength) shall be followed:

- For pavements 7" joints thick, saw the joint to a minimum depth of one-fourth (1/4") the specified pavement thickness.
- For pavements 7" joints thick, saw the joint to a maximum depth of one-third (1/3") the specified pavement thickness.
- For joints 3/8 inch - 1/8 inch wide use as measured at the time of sawing.

Use early-entry (dry cut, light weight saws, only use manufacturer's rated RPM) or early entry sawing saws. Perform the early entry sawing saws in the following sequence (initial set and before final set) as follows:

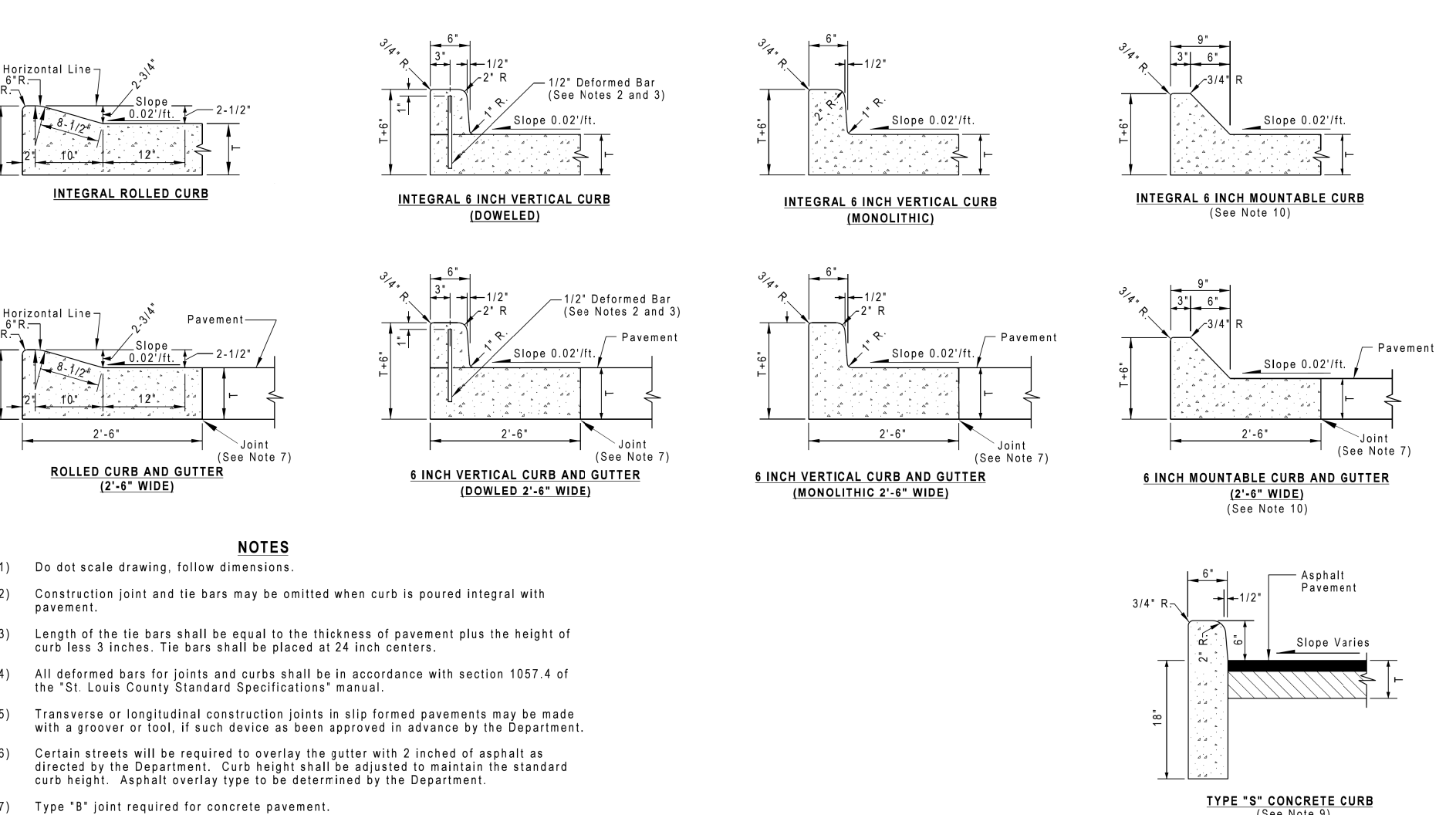
- Saw the joint 2'-1/4 to 2'-1/2 inches deep.

- When a standard (water cooled diamond bladed) concrete saw is used to cut the transverse or longitudinal joint the pavement reacts to the sawing process and gains strength before final seal the following applies:
 - For pavement < 7 inches thick, saw the joint to a minimum depth of one-fourth (1/4) the total pavement thickness.
 - For pavements > 7 inches thick, saw the joint to a minimum depth of one-third (1/3) the specified pavement thickness.
- Saw joints 3/8 inch (1/8) inch wide as measured at the time of sawing.

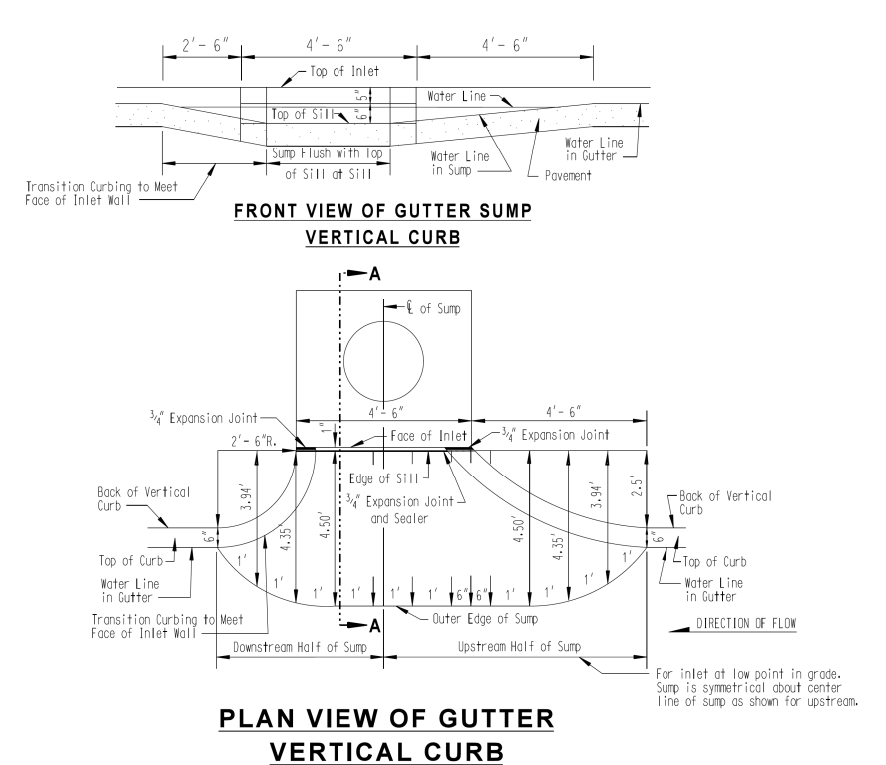
When using water-curable (dry cut, light weight) saws, only use saw blades and steel plates as recommended by the manufacturer. Perform the water-curable sawing reaction cure process (after initial seal and before final seal) as follows:

- Saw the joint 2-1/4 to 2-1/2 inches deep.

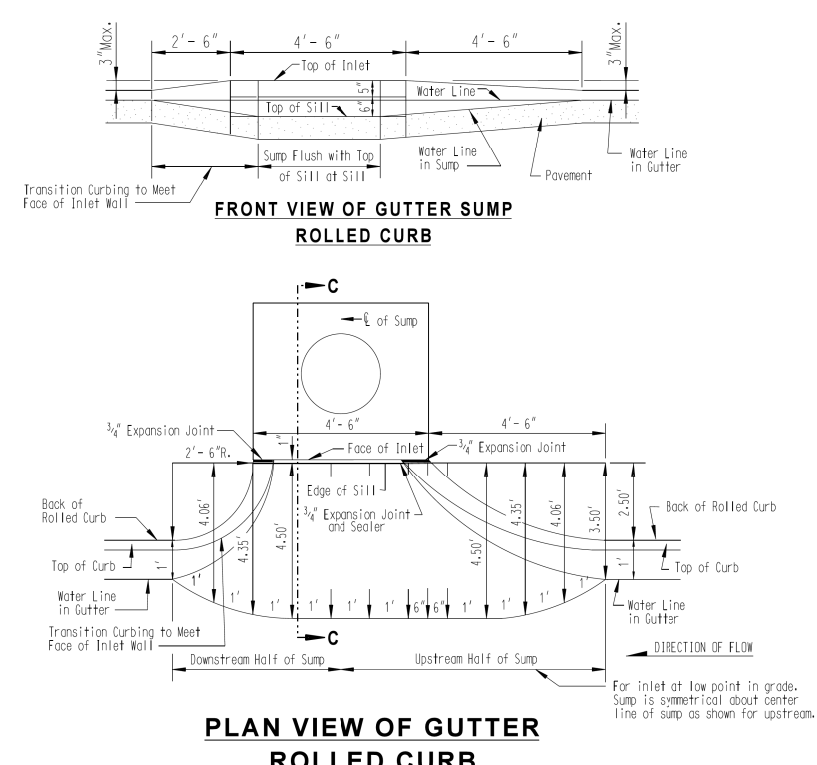
ISSUE DATE	THIS IS NOT A CERTIFIED DOCUMENT	 800 N. KENNEDY BLVD., SUITE 100 ST. LOUIS, MISSOURI 63102	PAVEMENT CONSTRUCTION DETAILS		
03/08/2025			CONCRETE JOINTS		
REVIEWS			EFFECTIVE	SHEET	DRAWING
			05/01/2020	2 OF 2	C502.13



ISSUE DATE 03/09/2020 REVISIONS	THIS IS NOT A CERTIFIED DOCUMENT	Saint Louis COUNTY TRANSPORTATION 1055 NORTH LINDSEY BLVD ST. LOUIS, MISSOURI 63102	STANDARD DRAWING CONCRETE CURB DETAILS	
			EFFECTIVE 05/01/2020	SHEET 1 OF 1 DRAWING CS02.14



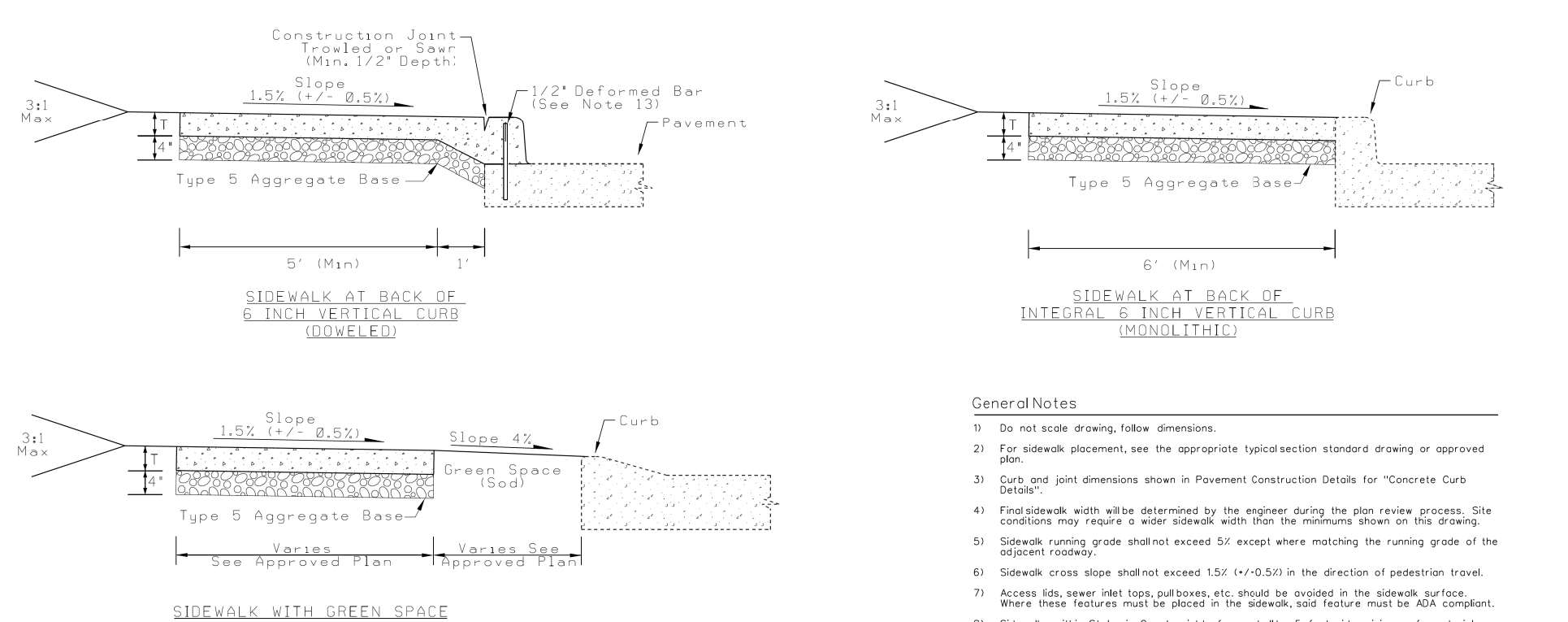
PLAN VIEW OF GUTT VERTICAL CURB



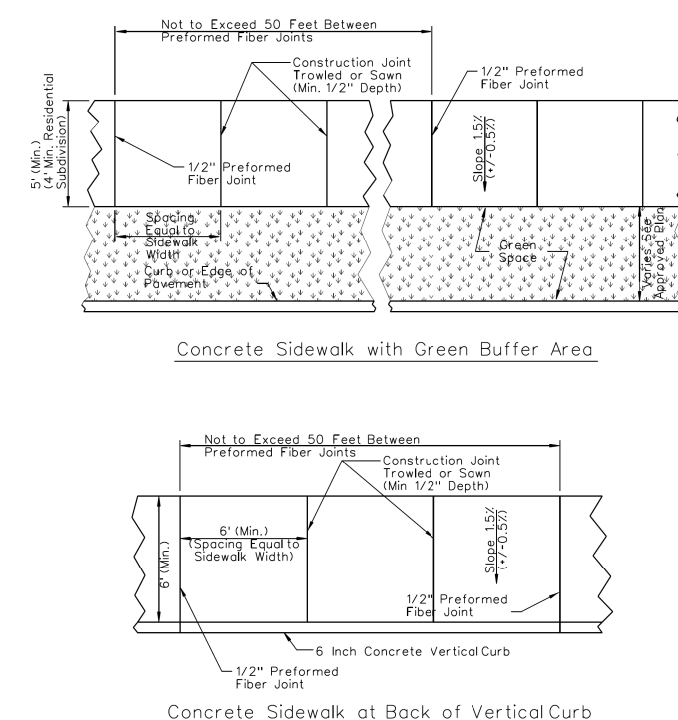
PLAN VIEW OF GUTTER ROLLED CURB

- 1) Do not scale drawing, follow dimensions.
- 4) Curb and inlet sumps may be modified due to field conditions.
- 3) All expansion joints shall be sealed.
- 4) Inlet structure shall be constructed in accordance with the current St. Louis Metropolitan Sewer District 'Standard Construction Specifications for Sewer and Drainage Facilities' manual.

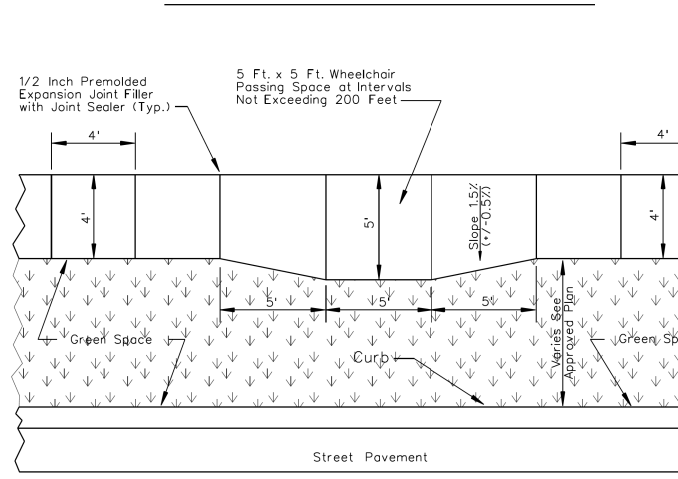
SECTION C - C			
ISSUE DATE 03/09/2020 REVISIONS	THIS IS NOT A CERTIFIED DOCUMENT	Saint Louis COUNTY TRANSPORTATION 650 NORTH KENNEDY BLVD ST. LOUIS, MISSOURI 63102	STANDARD DRAWING SEWER CONSTRUCTION DETAILS - STREET INLET SUMPS EFFECTIVE 05/01/2020 SHEET 1 OF 2 DRAWING C604.36



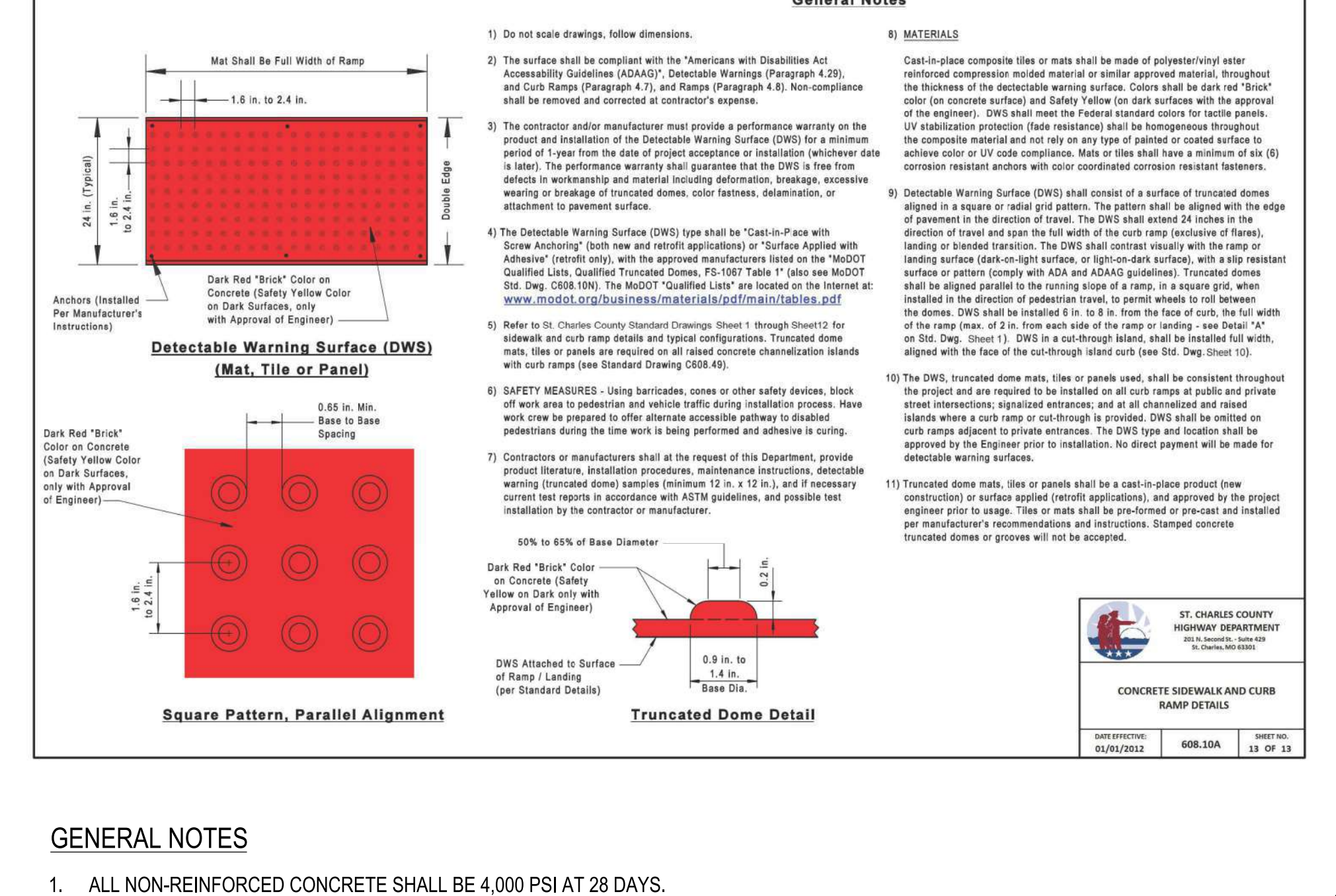
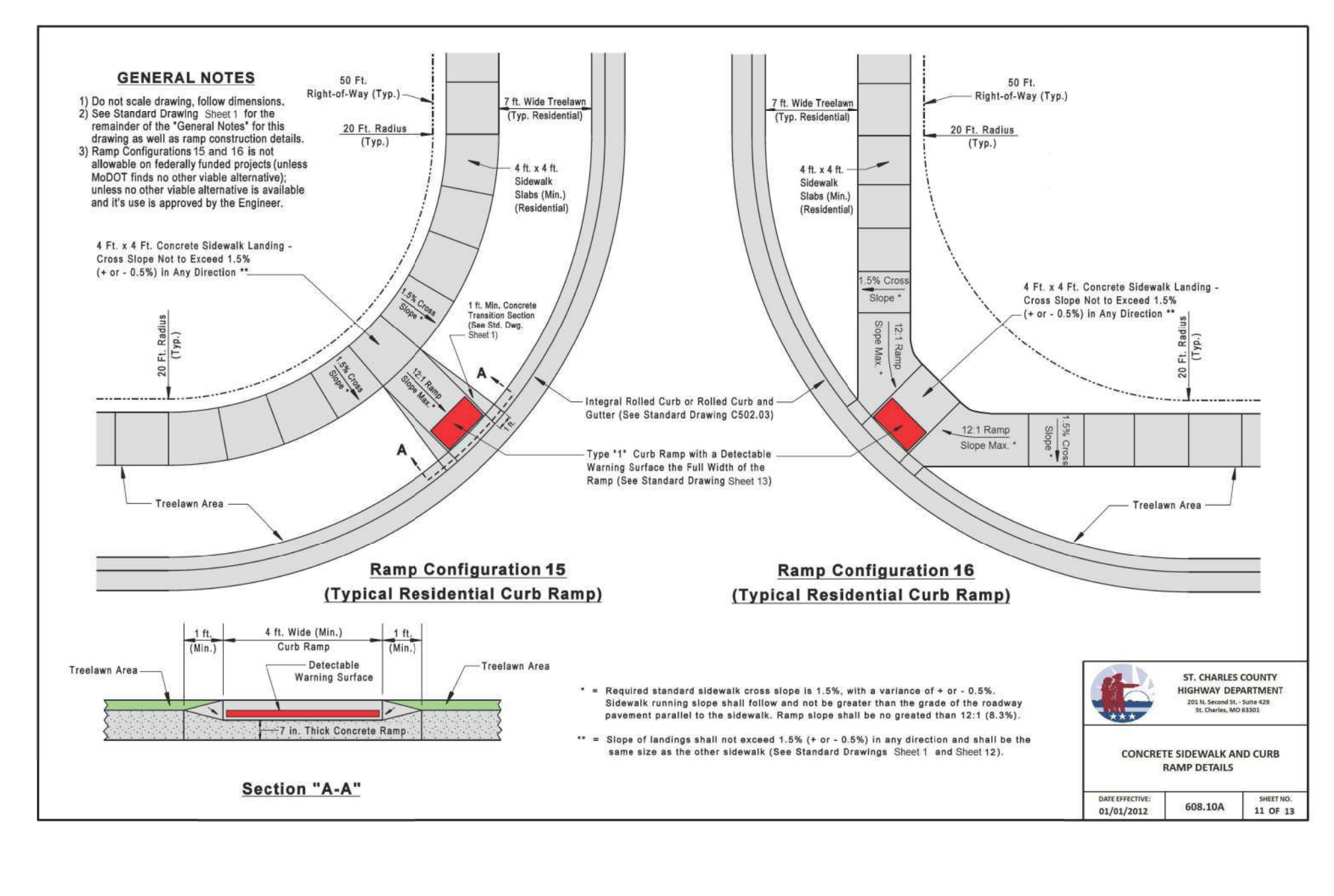
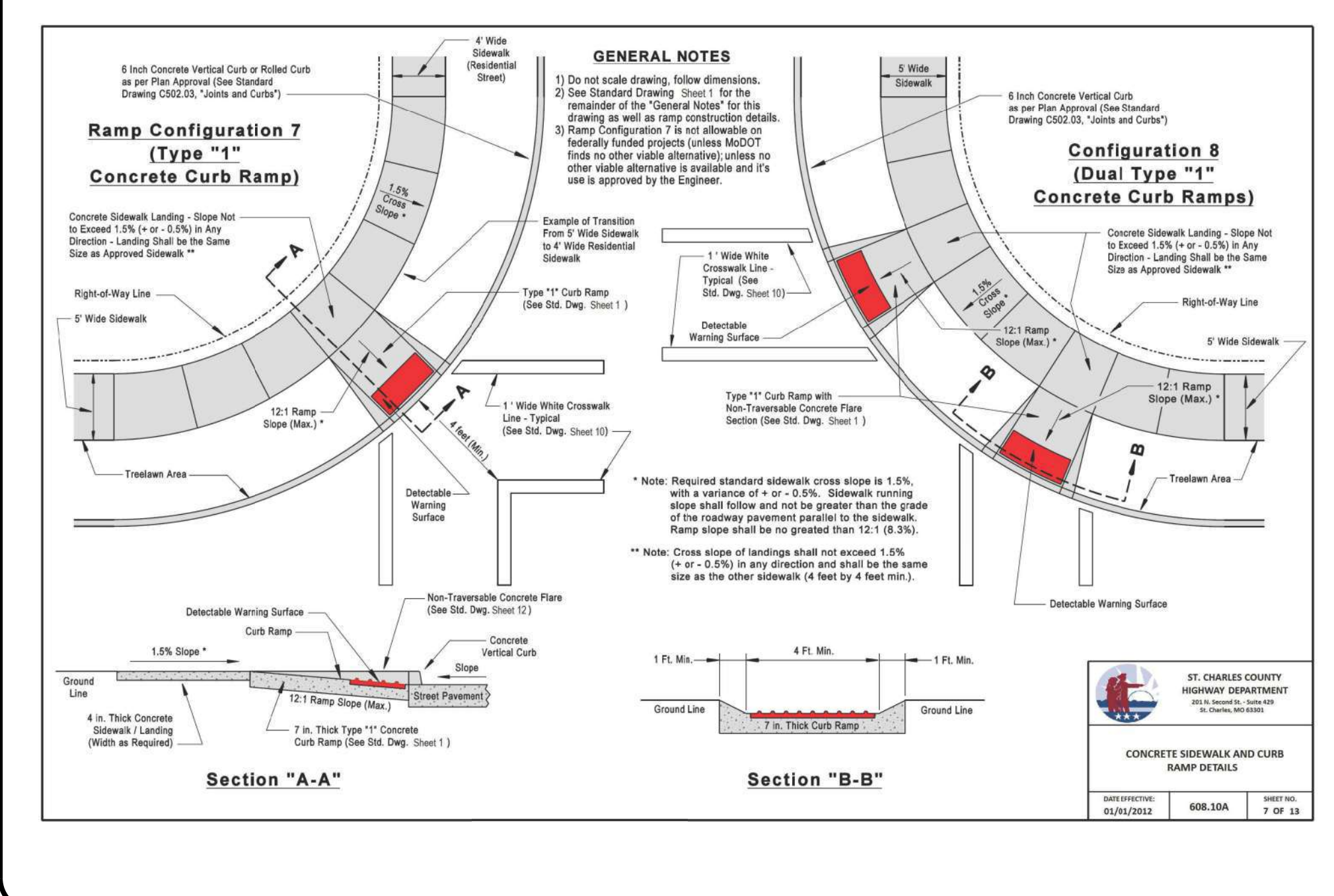
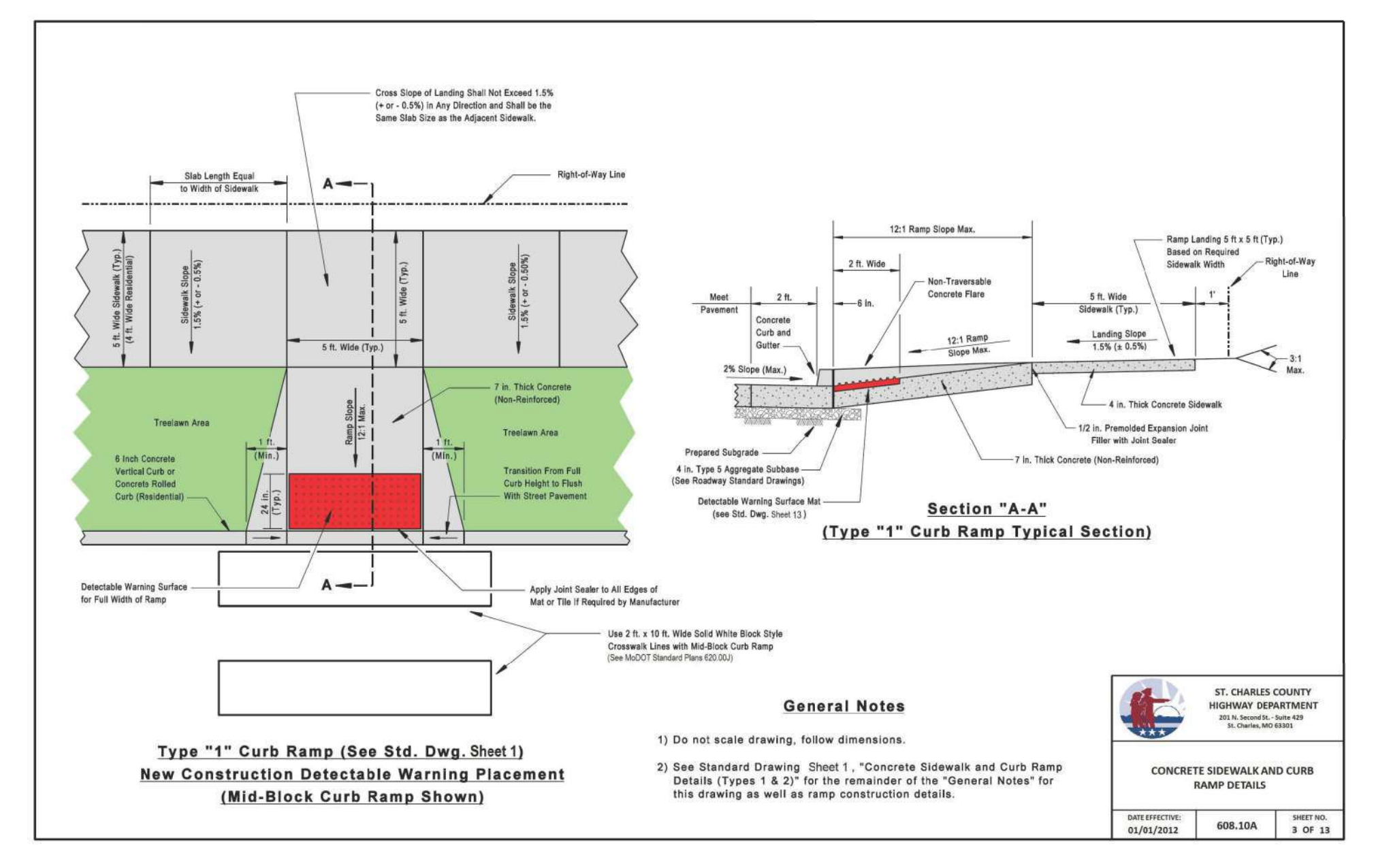
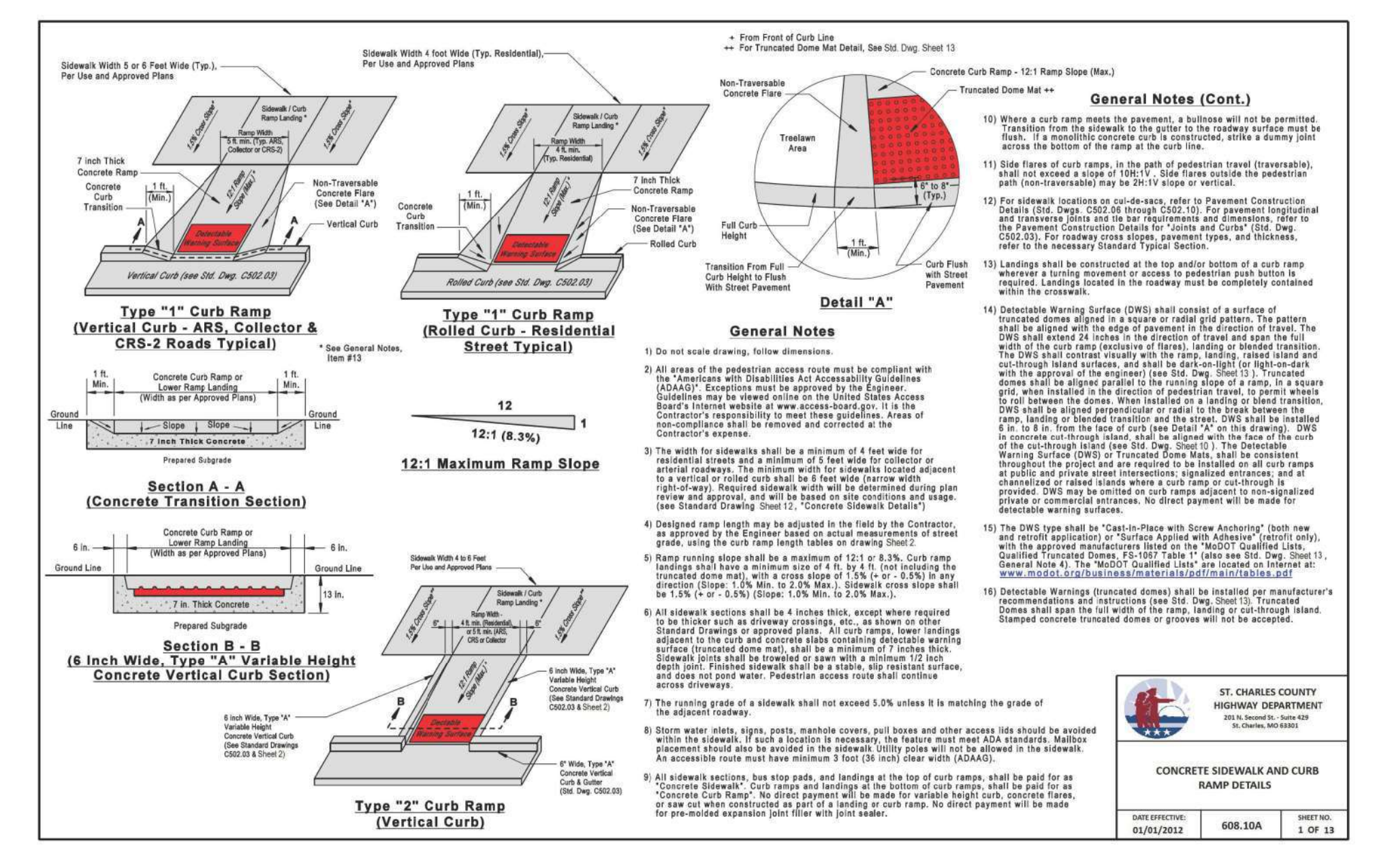
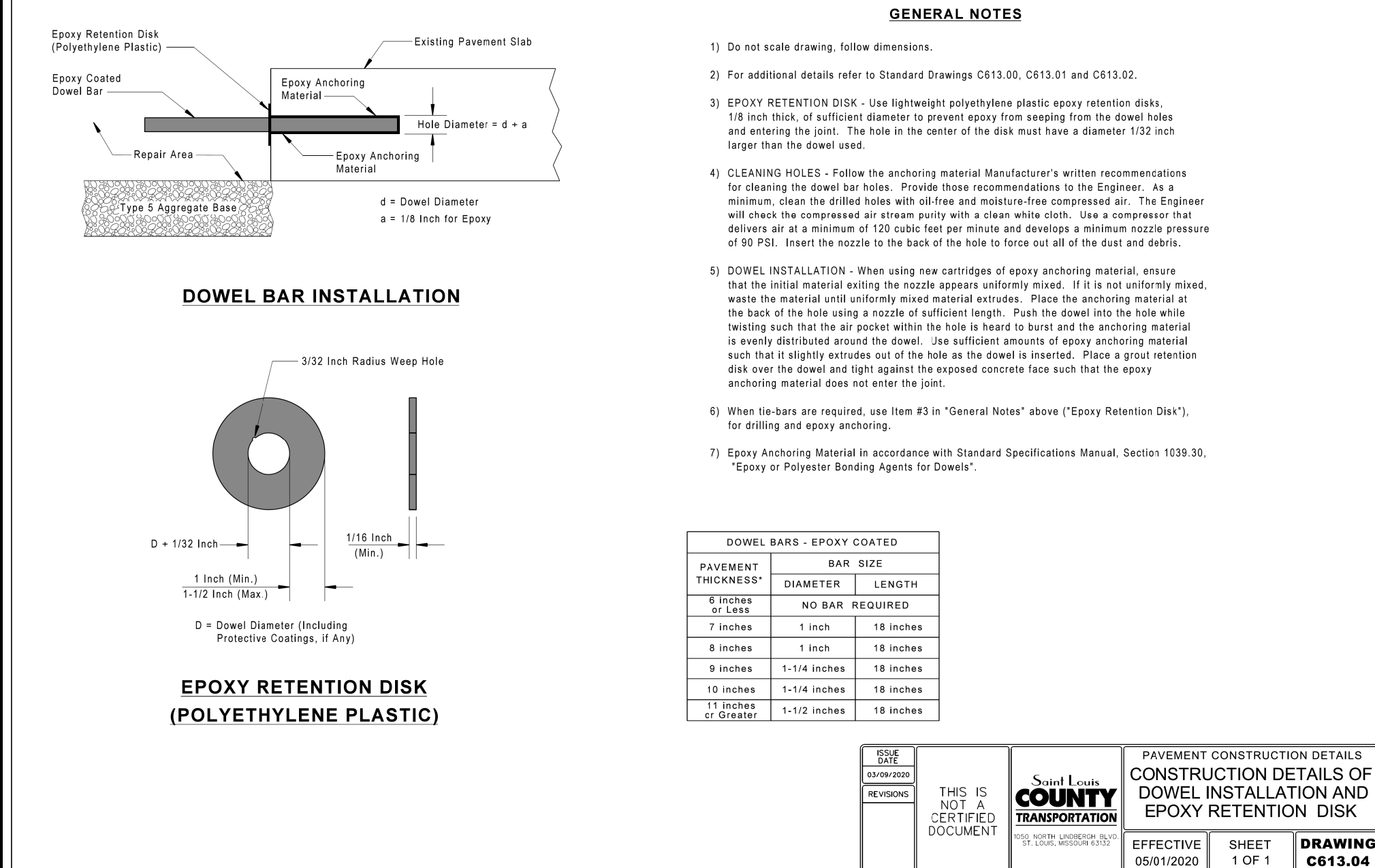
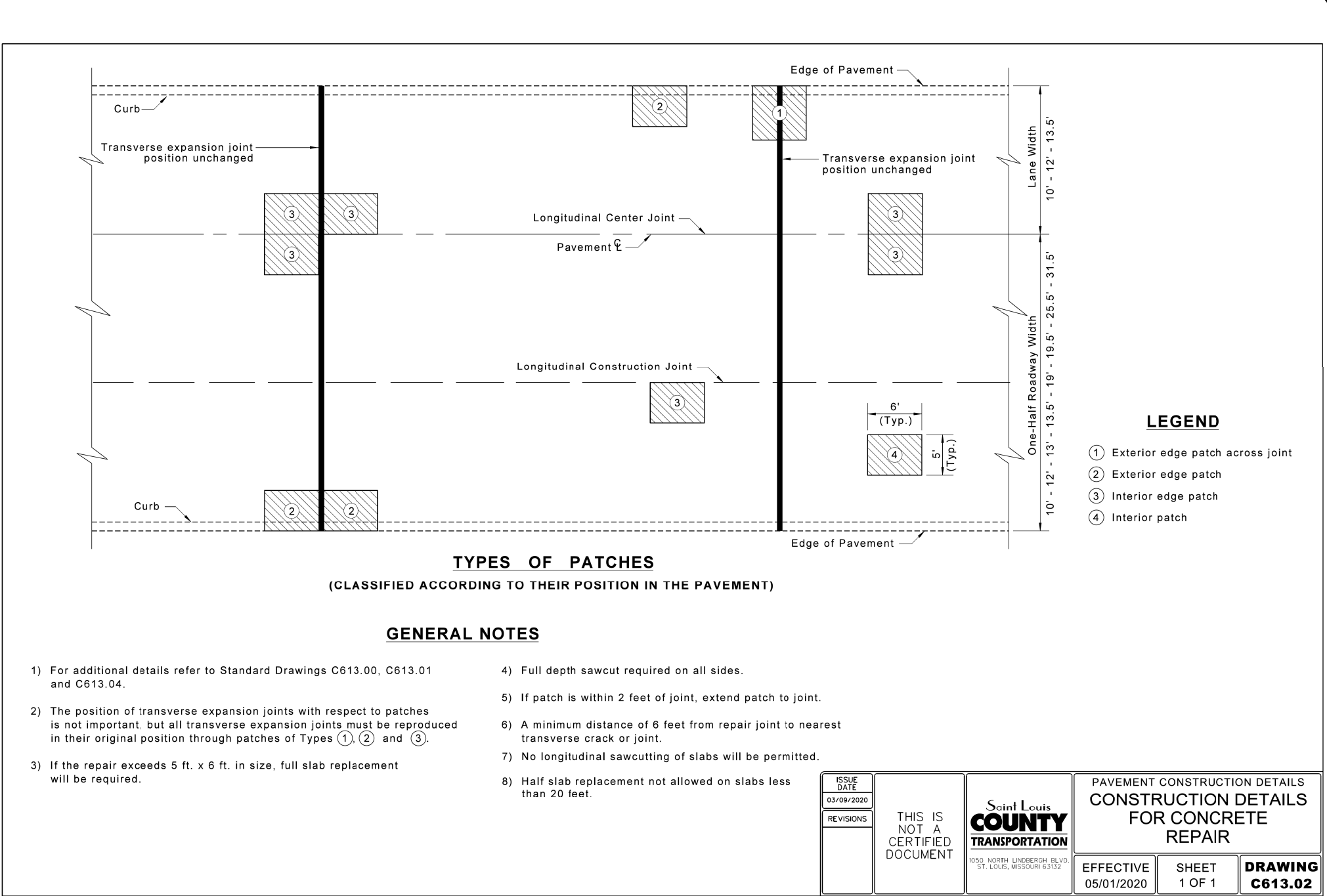
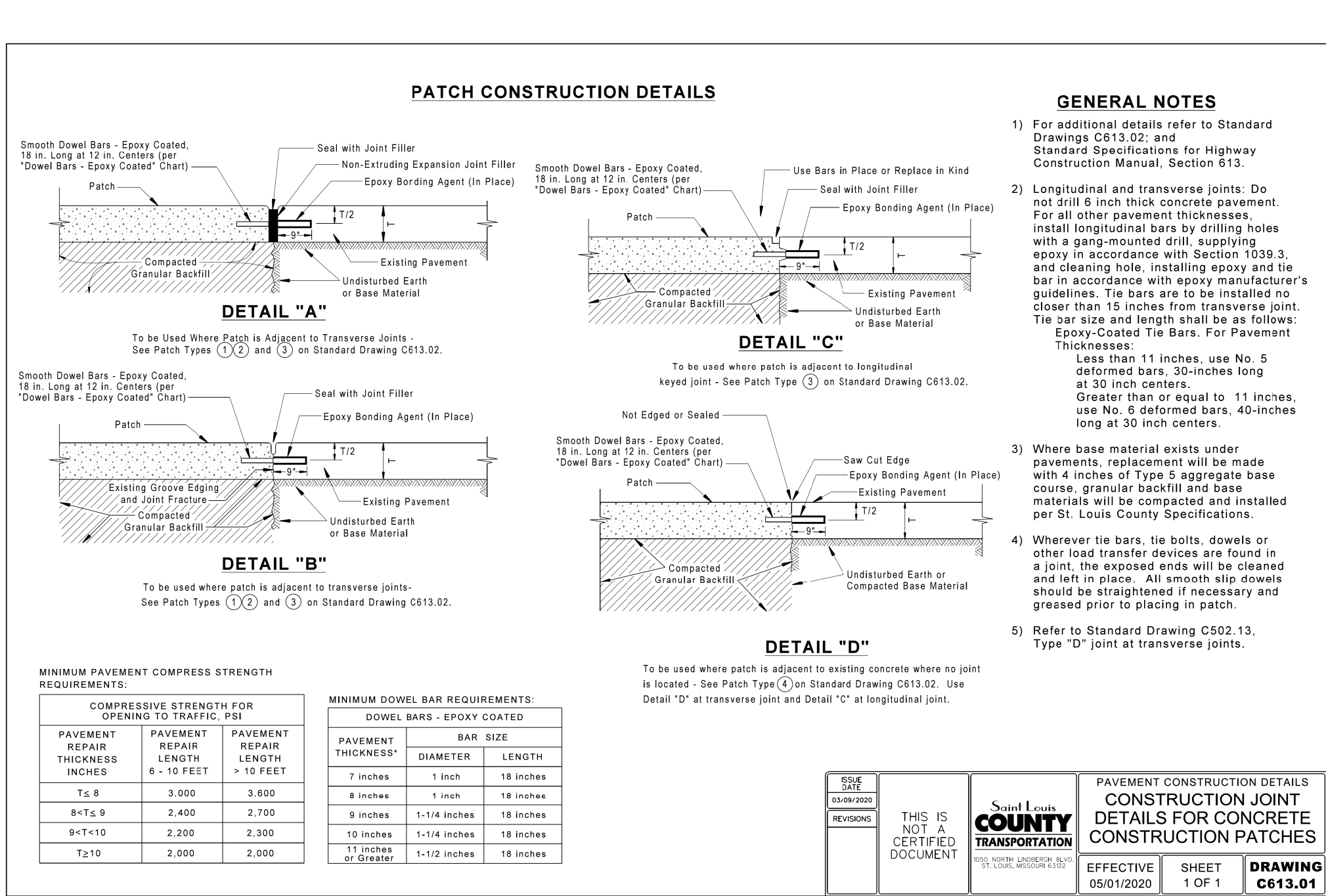
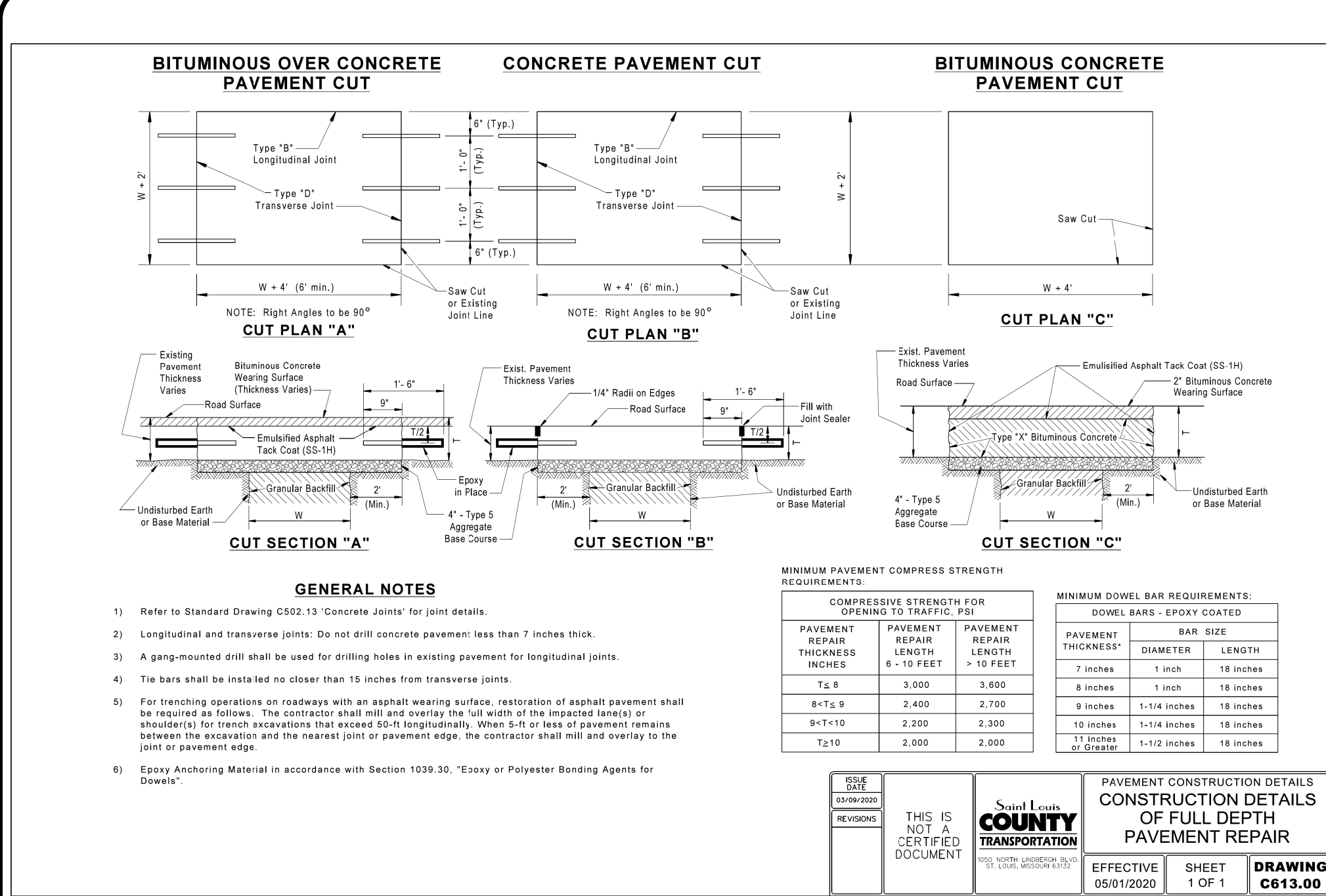
1) Sidewalk shall be a continuous and unobstructed clear width of 4 feet wide minimum and shall be constructed of concrete (ACI 308.4R) or asphalt (ACI 308.4R) with a minimum thickness of 4 inches. A 1/2" aggregate (D50-2) shall be used for the concrete. The clear width may be reduced to 4 feet only in order to accommodate a building entrance or other obstruction. 2) Where 4' sidewalk is provided, 5' of 1/2" passing stone should be provided at the building entrance. The 5' of 1/2" passing stone may be used to meet the passing stone requirement. 3) Sidewalk shall be constructed with 2" of 1/2" of the back of curb or on the back of curb for 2' minimum. 4) Where sidewalk shall be constructed with no pedestrian utility, turning stone shall be used to provide a 4' minimum clear width as defined by the 1/2" passing stone. The turning stone shall be 1/2" of 1/2" passing stone. The Back Slope Extension is not required. 5) Length of the curb shall be equal to the thickness of pavement plus the height of curb minus 2 inches. The curb shall slope at 2" or 24" minimum.	STANDARD DRAWING CITY OF CHANDLER DEPARTMENT OF PUBLIC WORKS THIS IS NOT A PERMIT DOCUMENT CHANDLER COUNTY TRANSPORTATION www.chandlercountytransportation.com EFFECTIVE 05/01/2020 SHEET 1 OF 3 DRAWN BY CE08.43
--	---



Concrete Sidewalk at Back of Vertical Curb



1. ALL NON-REINFORCED CONCRETE SHALL BE 4,000 PSI AT 28 DAYS.



GENERAL NOTES

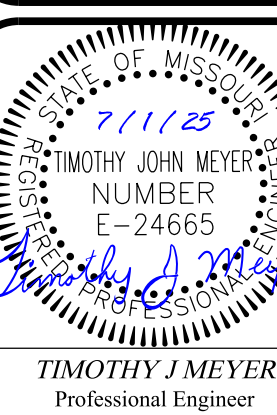
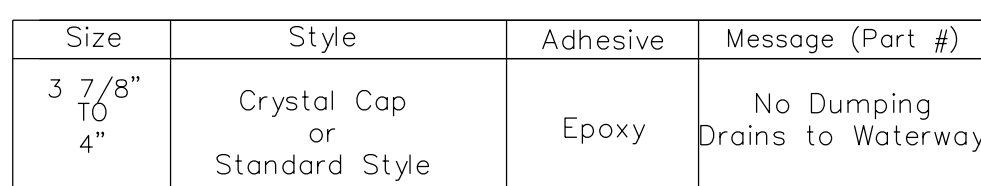
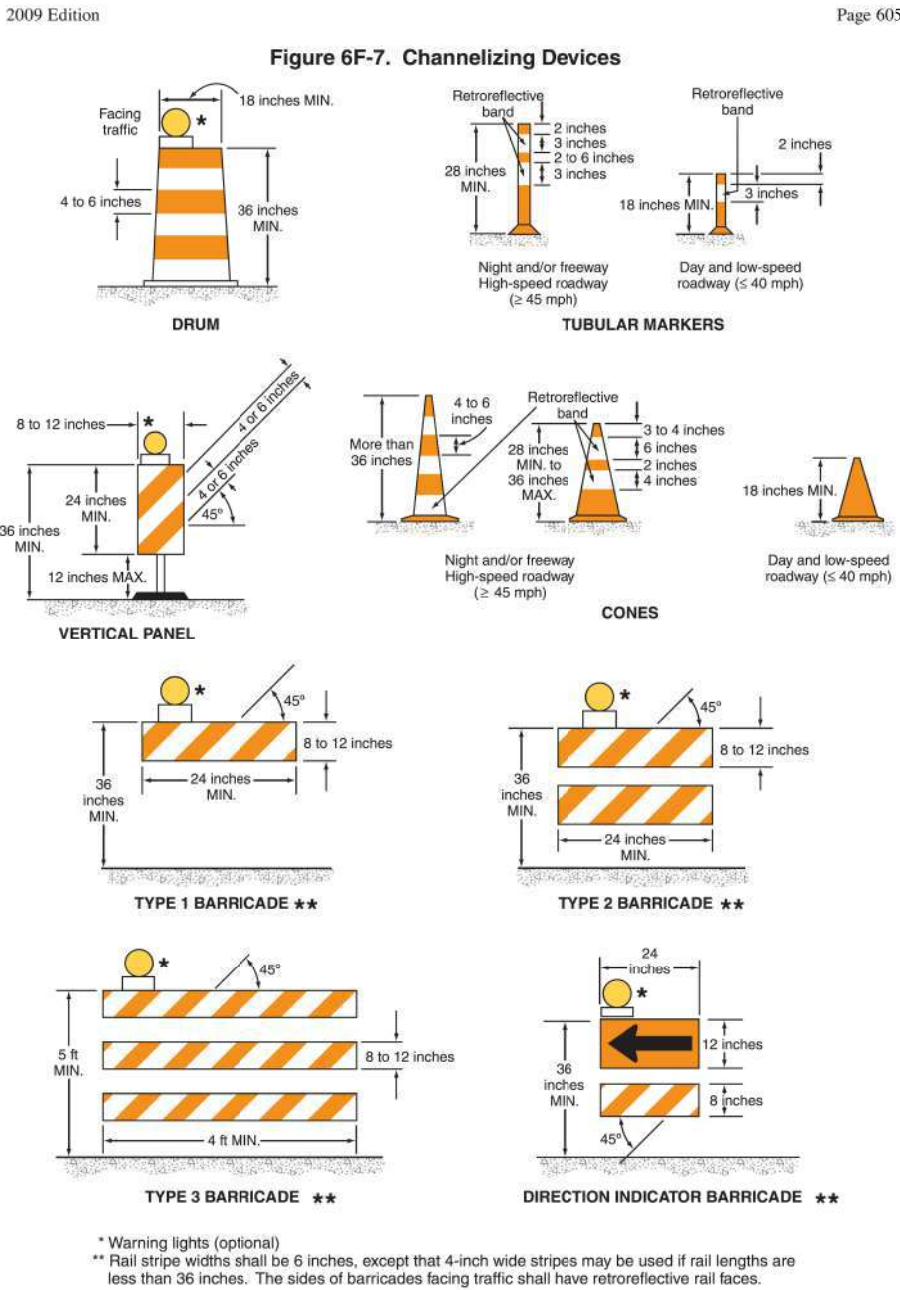
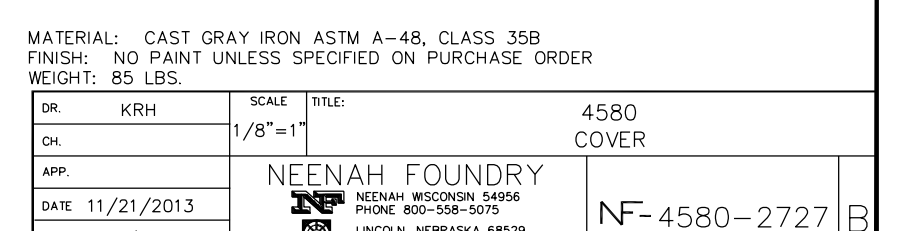
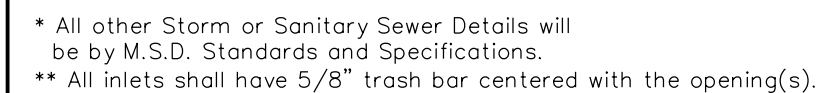
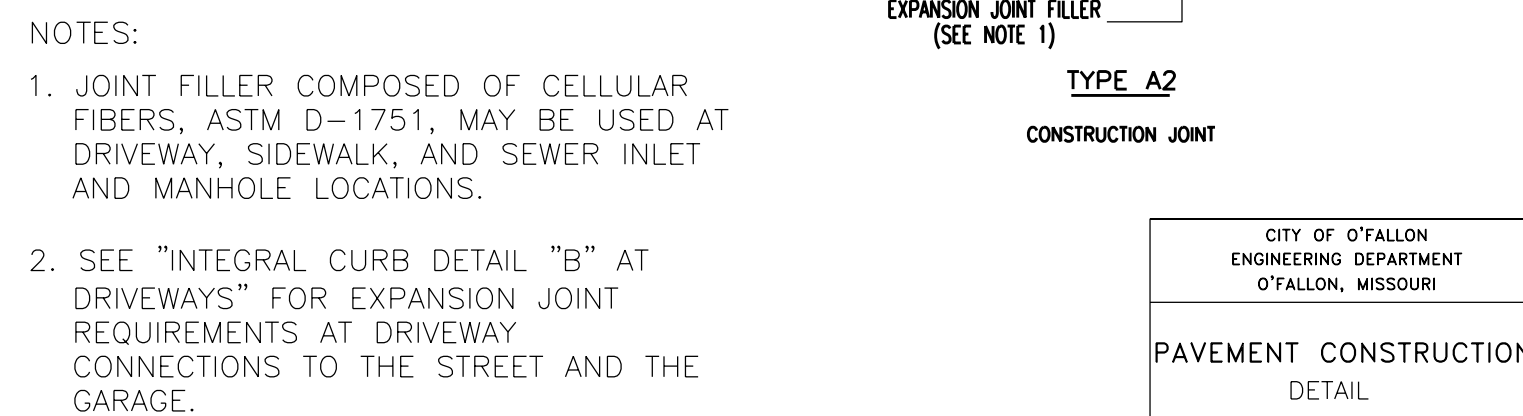
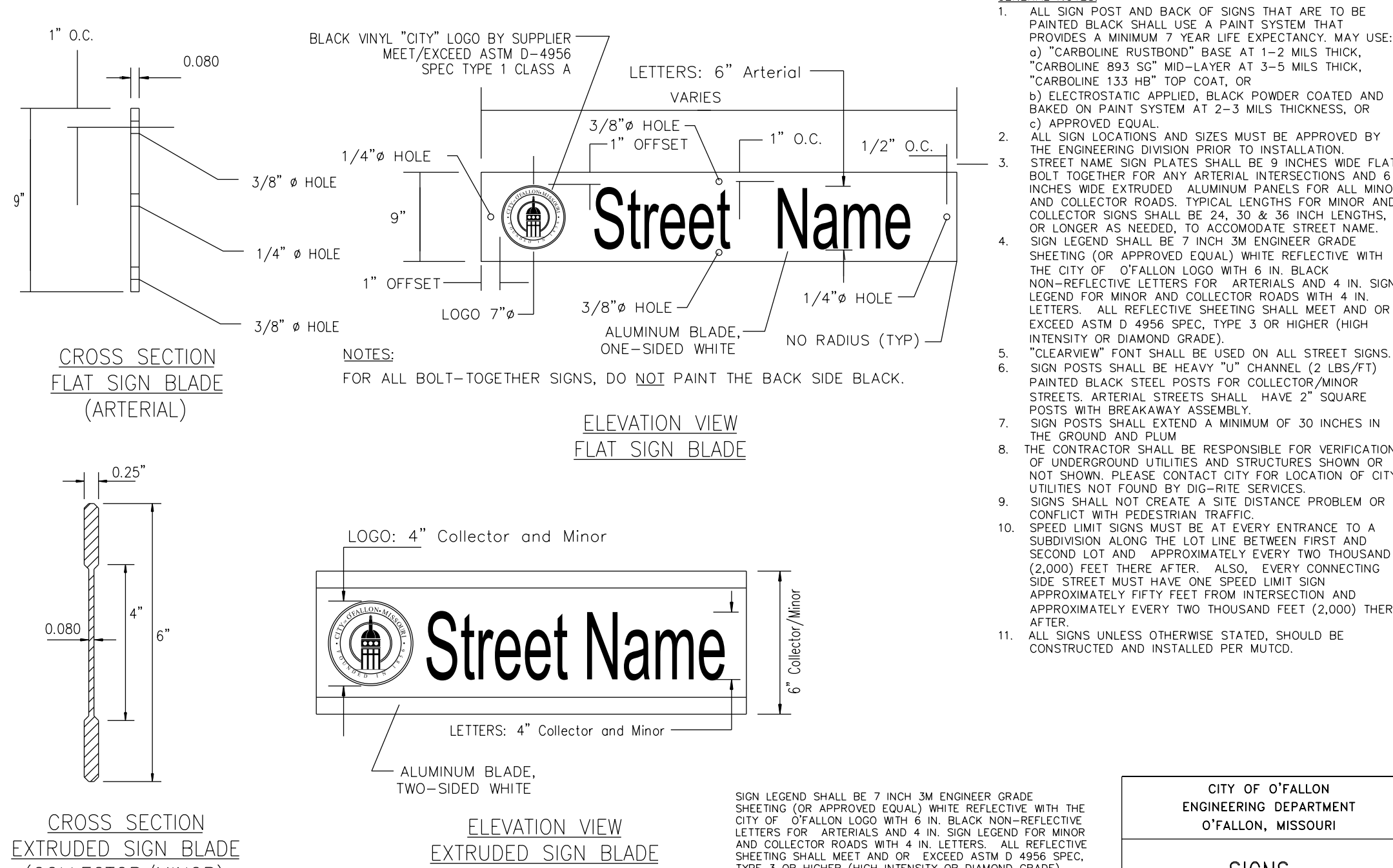
- ALL NON-REINFORCED CONCRETE SHALL BE 4,000 PSI AT 28 DAYS.

McKelvey Incorporated, 424 S. WOODS MILL RD., SUITE 110, TOWN & COUNTRY, MISSOURI 63017. Includes logo and contact information.

Professional Engineer, TIMOTHY J MEYER, Professional Engineer, MO E-24665. Includes logo and contact information.

STRAWBERRY FARMS

CONSTRUCTION DETAILS: SHEET 2, 8265 MEXICO ROAD, OF FALLON, MO 63376. Includes logo and contact information.



STRAWBERRY FARMS

CONSTRUCTION DETAILS:
SHEET 3

PARCEL ID: 2-0119-1766-00-0024.5
CITY OF O'FALLON
PERMIT No: RSP25-000001

07-01-25