

2005



ENGINEERING

PLANNING

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**Diehr Road Tract
Clearing Plan
Interim Sediment Basin Storage Calculations**

**Prepared For:
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March 28, 2005
Bax Project No.: 04-12901**



BAX ENGINEERING ►

Sediment Storage Calculations

Prepared By: Bax Engineering Company, Inc.

Diehr Road Tract – O’Fallon, MO

Bax Project No.: 04-12901

March 28, 2005

Temporary Sediment Basin A:

Basin Inflow Data

Basin design to accommodate the 25 year, 20 minute design storm and allowing for 1 foot minimum of freeboard above 25 year high-water elevation.

Disturbed area to the basin = 1.41 Acres

25-Year, 20 minute P.I. – 2.50; Discharge $Q = 1.41 * 2.50 = 3.52$ cfs

Total drainage area to basin 1.41 Acres

Total discharge to basin 3.52 cfs

Annual sediment storage volume required per City of O’Fallon requirements is 125 ft³ of sediment storage/Acre of disturbed area to sediment basin

A. Sediment storage volume required

2 years of sediment storage = 1.41 Acres (125 ft³/Acre/year)(2 years)

2 years of sediment storage = **353 ft³**

Basin Volume:

Elev.	Area (ft ²)	Incr. Vol. (ft ³)	Total Vol. (ft ³)
Btm. Of Basin 619	706	0	0
620	967	833	833
622	1,569	2,512	3,345
624	2,274	3,821	7,166

Sediment storage volume achieved at elevation : **619.42**

B. “Wet” storage volume required

Note: “Wet” storage volume to be provided above sediment storage volume elevation

$67 \text{ yd}^3/\text{Ac} \times 27 \text{ ft}^3/\text{yd}^3 \times 1.41 \text{ Ac} = \mathbf{2,551 \text{ ft}^3}$

“Wet” storage volume achieved at elevation: **621.65**

C. “Dry” storage volume required

Note: “Dry” storage volume to be provided above “wet” storage volume elevation

$67 \text{ yd}^3/\text{Ac} \times 27 \text{ ft}^3/\text{yd}^3 \times 1.41 \text{ Ac} = \mathbf{2,551 \text{ ft}^3}$

“Dry” storage volume achieved at elevation: **623.10**

E. **Temporary Basin A outfall structure**

Structure to consist of a temporary 24" C.M.P. riser section connected to a temporary 18" C.M.P. with a dewatering device composed of 8" perforated polyethylene drainage tubing connected to the C.M.P. riser at the "wet" storage volume elevation. Top of riser is set at a 623.10 elevation. See Figure 1 for details.

F. **25-Year routing results**

See attached calculations for details. Calculations ran assuming a starting water surface elevation at the "dry" sediment storage volume: 623.10 and a time of concentration duration of 5 minutes.

25-Year H.W. – 623.43

Top of Berm – 624.50

Freeboard – 1.07'

Temporary Sediment Basin B:

Basin Inflow Data

Basin design to accommodate the 25 year, 20 minute design storm and allowing for 1 foot minimum of freeboard above 25 year high-water elevation.

Disturbed area to the basin = 1.30 Acres

25-Year, 20 minute P.I. – 2.50; Discharge Q = 1.30 * 2.50 = 3.25 cfs

Total drainage area to basin 1.30 Acres

Total discharge to basin 3.25 cfs

Annual sediment storage volume required per City of O'Fallon requirements is 125 ft³ of sediment storage/Acre of disturbed area to sediment basin

A. **Sediment storage volume required**

2 years of sediment storage = 1.30 Acres (125 ft³/Acre/year)(2 years)

2 years of sediment storage = **325 ft³**

Basin Volume:

Elev.	Area (ft ²)	Incr. Vol. (ft ³)	Total Vol. (ft ³)
Btm. Of Basin 621	631	0	0
622	975	797	797
624	1,514	2,469	3,266
626	2,169	3,663	6,930

Sediment storage volume achieved at elevation : **621.41**

B. **"Wet" storage volume required**

Note: "Wet" storage volume to be provided above sediment storage volume elevation
67 yd³/Ac x 27 ft³/yd³ x 1.30 Ac = **2,352 ft³**

"Wet" storage volume achieved at elevation: **623.52**

C. **“Dry” storage volume required**

Note: “Dry” storage volume to be provided above “wet” storage volume elevation
 $67 \text{ yd}^3/\text{Ac} \times 27 \text{ ft}^3/\text{yd}^3 \times 1.30 \text{ Ac} = 2,352 \text{ ft}^3$

“Dry” storage volume achieved at elevation: **624.96**

E. **Temporary Basin A outfall structure**

Structure to consist of a temporary 24” C.M.P. riser section connected to a temporary 18” C.M.P. with a dewatering device composed of 8” perforated polyethylene drainage tubing connected to the C.M.P. riser at the “wet” storage volume elevation. Top of riser is set at a 624.96 elevation. See Figure 1 for details.

F. **25-Year routing results**

See attached calculations for details. Calculations ran assuming a starting water surface elevation at the “dry” sediment storage volume: 624.96 and a time of concentration duration of 5 minutes.

25-Year H.W. – 625.27

Top of Berm – 626.30

Freeboard – 1.03’

Temporary Sediment Basin C:

Basin Inflow Data

Basin design to accommodate the 25 year, 20 minute design storm and allowing for 1 foot minimum of freeboard above 25 year high-water elevation.

Disturbed area to the basin = 3.05 Acres

25-Year, 20 minute P.I. – 2.50; Discharge Q = 3.05 * 2.50 = 7.63 cfs

Total drainage area to basin 3.05 Acres

Total discharge to basin 7.63 cfs

Annual sediment storage volume required per City of O’Fallon requirements is 125 ft³ of sediment storage/Acre of disturbed area to sediment basin

A. **Sediment storage volume required**

2 years of sediment storage = 3.05 Acres (125 ft³/Acre/year)(2 years)

2 years of sediment storage = **763 ft³**

Basin Volume:

Elev.	Area (ft ²)	Incr. Vol. (ft ³)	Total Vol. (ft ³)
Btm. Of Basin 600	2,340	0	0
602	3,195	5,513	5,513
604	4,153	7,327	12,840
606	5,438	9,562	22,402

Sediment storage volume achieved at elevation : **600.28**

B. "Wet" storage volume required

Note: "Wet" storage volume to be provided above sediment storage volume elevation
 $67 \text{ yd}^3/\text{Ac} \times 27 \text{ ft}^3/\text{yd}^3 \times 3.05 \text{ Ac} = 5,517 \text{ ft}^3$

"Wet" storage volume achieved at elevation: **602.21**

C. "Dry" storage volume required

Note: "Dry" storage volume to be provided above "wet" storage volume elevation
 $67 \text{ yd}^3/\text{Ac} \times 27 \text{ ft}^3/\text{yd}^3 \times 3.05 \text{ Ac} = 5,517 \text{ ft}^3$

"Dry" storage volume achieved at elevation: **603.71**

E. Temporary Basin A outfall structure

Structure to consist of a temporary 24" C.M.P. riser section connected to a temporary 18" C.M.P. with a dewatering device composed of 8" perforated polyethylene drainage tubing connected to the C.M.P. riser at the "wet" storage volume elevation. Top of riser is set at a 604.00 elevation. See Figure 1 for details.

F. 25-Year routing results

See attached calculations for details. Calculations ran assuming a starting water surface elevation at the "dry" sediment storage volume: 603.71 and a time of concentration duration of 5 minutes.

25-Year H.W. – 604.54

Top of Berm – 606.00

Freeboard – 1.46'

Temporary Sediment Basin D:

Basin Inflow Data

Basin design to accommodate the 25 year, 20 minute design storm and allowing for 1 foot minimum of freeboard above 25 year high-water elevation.

Disturbed area to the basin = 2.27 Acres

25-Year, 20 minute P.I. – 2.50; Discharge $Q = 2.27 \times 2.50 = 5.68 \text{ cfs}$

Total drainage area to basin 2.27 Acres

Total discharge to basin 5.68 cfs

Annual sediment storage volume required per City of O'Fallon requirements is
 125 ft^3 of sediment storage/Acre of disturbed area to sediment basin.

A. Sediment storage volume required

2 years of sediment storage = 2.27 Acres (125 ft³/Acre/year)(2 years)

2 years of sediment storage = **568 ft³**

Basin Volume:

Elev.	Area (ft ²)	Incr. Vol. (ft ³)	Total Vol. (ft ³)
Btm. Of Basin 612	1,961	0	0
614	2,845	4,779	4,779
616	3,851	6,671	11,449
618	4,975	8,802	20,251
620	6,203	11,155	31,407

Sediment storage volume achieved at elevation : **612.24**

B. "Wet" storage volume required

Note: "Wet" storage volume to be provided above sediment storage volume elevation

67 yd³/Ac x 27 ft³/yd³ x 2.27 Ac = **4,107 ft³**

"Wet" storage volume achieved at elevation: **613.96**

C. "Dry" storage volume required

Note: "Dry" storage volume to be provided above "wet" storage volume elevation

67 yd³/Ac x 27 ft³/yd³ x 2.27 Ac = **4,107 ft³**

"Dry" storage volume achieved at elevation: **615.20**

E. Temporary Basin A outfall structure

Structure to consist of a temporary 24" C.M.P. riser section connected to a temporary 18" C.M.P. with a dewatering device composed of 8" perforated polyethylene drainage tubing connected to the C.M.P. riser at the "wet" storage volume elevation. Top of riser is set at a 617.00 elevation. See Figure 1 for details.

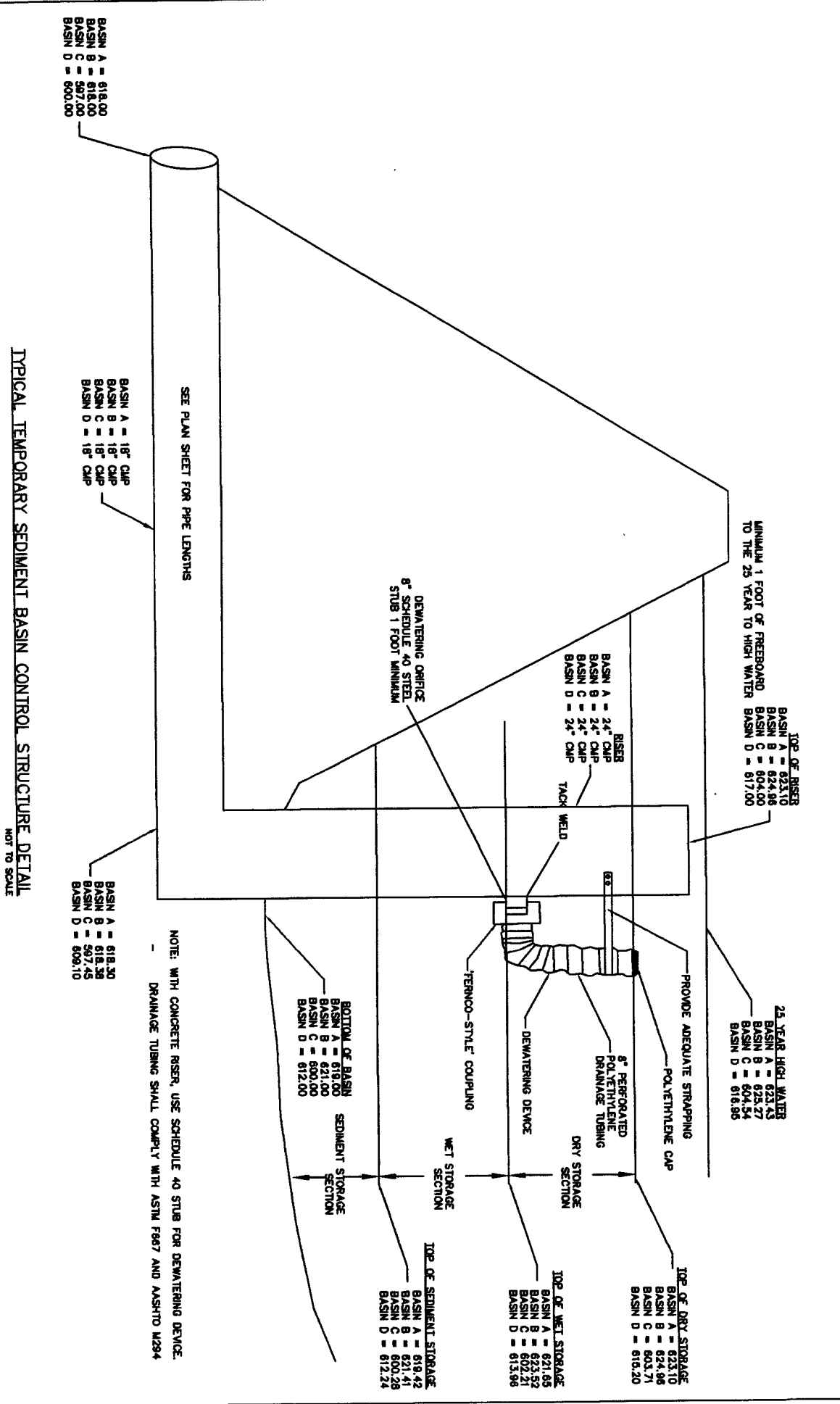
F. 25-Year routing results

See attached calculations for details. Calculations ran assuming a starting water surface elevation at the "dry" sediment storage volume: 615.20 and a time of concentration duration of 5 minutes.

25-Year H.W. – 616.95

Top of Berm – 620.00

Freeboard – 3.05'



TYPICAL TEMPORARY SEDIMENT BASIN CONTROL STRUCTURE DETAIL
NOT TO SCALE

FIGURE 1

BASIN A = 616.00
BASIN B = 616.00
BASIN C = 597.00
BASIN D = 600.00

BASIN A = 18" CAP
BASIN B = 18" CAP
BASIN C = 18" CAP
BASIN D = 18" CAP

BASIN A = 618.30
BASIN B = 618.38
BASIN C = 597.45
BASIN D = 609.10

NOTE: WITH CONCRETE RISER, USE SCHEDULE 40 STUB FOR DEWATERING DEVICE.
DRAINAGE TUBING SHALL COMPLY WITH ASTM F987 AND AASHTO M294

TOP OF RISER
BASIN A = 623.10
BASIN B = 624.86
BASIN C = 604.00
BASIN D = 617.00

25 YEAR HIGH WATER
BASIN A = 623.43
BASIN B = 625.27
BASIN C = 604.54
BASIN D = 618.95

RISER
BASIN A = 24" CAP
BASIN B = 24" CAP
BASIN C = 24" CAP
BASIN D = 24" CAP

TACK
WELD

DEWATERING ORIFICE
8" SCHEDULE 40 STEEL
STUB 1 FOOT MINIMUM

PROVIDE ADEQUATE STRAPPING
POLYETHYLENE CAP

8" PERFORATED
POLYETHYLENE
DRAINAGE TUBING

DEWATERING DEVICE

'TERRCO'-STYLE' COUPLING

DRY STORAGE
SECTION

WET STORAGE
SECTION

SEDIMENT
STORAGE
SECTION

BOTTOM OF BASIN
BASIN A = 618.00
BASIN B = 621.00
BASIN C = 600.00
BASIN D = 612.00

TOP OF DRY STORAGE
BASIN A = 623.10
BASIN B = 624.86
BASIN C = 603.71
BASIN D = 616.20

TOP OF WET STORAGE
BASIN A = 621.65
BASIN B = 623.22
BASIN C = 603.71
BASIN D = 613.96

TOP OF SEDIMENT STORAGE
BASIN A = 619.42
BASIN B = 621.41
BASIN C = 600.28
BASIN D = 612.24

SEE PLAN SHEET FOR PIPE LENGTHS

**25 YEAR, 20 MINUTE
POND 9
Sediment Storage
Routing Calculations
For Temporary Sediment Basin A**

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***** POND ROUTING *****

SED BASIN A IN 25	
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SED BASIN A OUT 25	
Pond Routing Summary	4.03

HYG file =
 HYG ID = Basin A 25 in
 HYG Tag = 25 Yr

 Peak Discharge = 3.52 cfs
 Time to Peak = 5.00 min
 HYG Volume = 4219 cu.ft

HYDROGRAPH ORDINATES (cfs)
 Output Time increment = 1.00 min
 Time on left represents time for first value in each row.

Time min					
.00	.00	.70	1.40	2.10	2.80
5.00	3.52	3.52	3.52	3.52	3.52
10.00	3.52	3.52	3.52	3.52	3.52
15.00	3.52	3.52	3.52	3.52	3.52
20.00	3.52	2.80	2.10	1.40	.70
25.00	.00	.00	.00	.00	.00
30.00	.00	.00	.00	.00	.00
35.00	.00	.00	.00	.00	.00
40.00	.00	.00	.00	.00	.00
45.00	.00	.00	.00	.00	.00
50.00	.00	.00	.00	.00	.00
55.00	.00	.00	.00	.00	.00
60.00	.00	.00	.00	.00	.00
65.00	.00	.00	.00	.00	.00
70.00	.00	.00	.00	.00	.00
75.00	.00	.00	.00	.00	.00
80.00	.00	.00	.00	.00	.00
85.00	.00	.00	.00	.00	.00
90.00	.00	.00	.00	.00	.00
95.00	.00	.00	.00	.00	.00
100.00	.00	.00	.00	.00	.00
105.00	.00	.00	.00	.00	.00
110.00	.00	.00	.00	.00	.00
115.00	.00	.00	.00	.00	.00
120.00	.00	.00	.00	.00	.00
125.00	.00	.00	.00	.00	.00
130.00	.00	.00	.00	.00	.00
135.00	.00	.00	.00	.00	.00
140.00	.00	.00	.00	.00	.00
145.00	.00	.00	.00	.00	.00
150.00	.00	.00	.00	.00	.00

HYDROGRAPH ORDINATES (cfs)
 Output Time increment = 1.00 min
 Time on left represents time for first value in each row.

Time min						
155.00	.00	.00	.00	.00	.00	.00
160.00	.00	.00	.00	.00	.00	.00
165.00	.00	.00	.00	.00	.00	.00
170.00	.00	.00	.00	.00	.00	.00
175.00	.00	.00	.00	.00	.00	.00
180.00	.00	.00	.00	.00	.00	.00
185.00	.00	.00	.00	.00	.00	.00
190.00	.00	.00	.00	.00	.00	.00
195.00	.00	.00	.00	.00	.00	.00
200.00	.00	.00	.00	.00	.00	.00
205.00	.00	.00	.00	.00	.00	.00
210.00	.00	.00	.00	.00	.00	.00
215.00	.00	.00	.00	.00	.00	.00
220.00	.00	.00	.00	.00	.00	.00
225.00	.00	.00	.00	.00	.00	.00
230.00	.00	.00	.00	.00	.00	.00
235.00	.00	.00	.00	.00	.00	.00
240.00	.00	.00	.00	.00	.00	.00
245.00	.00	.00	.00	.00	.00	.00
250.00	.00					

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POND VOLUME CALCULATIONS

Planimeter scale: 1.00 ft/in

Elevation (ft)	Planimeter (sq.in)	Area (acres)	A1+A2+sqrt(A1*A2) (acres)	Volume (cu.ft)	Volume Sum (cu.ft)
619.00	706.000	.0162	.0000	0	0
620.00	967.000	.0222	.0574	833	833
622.00	1569.000	.0360	.0865	2512	3345
624.00	2274.000	.0522	.1316	3821	7166

POND VOLUME EQUATIONS

* Incremental volume computed by the Conic Method for Reservoir Volumes.

$$\text{Volume} = (1/3) * (\text{EL2}-\text{EL1}) * (\text{Areal} + \text{Area2} + \text{sq.rt.}(\text{Areal}*\text{Area2}))$$

where: EL1, EL2 = Lower and upper elevations of the increment
Areal, Area2 = Areas computed for EL1, EL2, respectively
Volume = Incremental volume between EL1 and EL2

REQUESTED POND WS ELEVATIONS:

Min. Elev.= 619.00 ft
Increment = .10 ft
Max. Elev.= 624.00 ft

OUTLET CONNECTIVITY

---> Forward Flow Only (UpStream to DnStream)
<--- Reverse Flow Only (DnStream to UpStream)
<---> Forward and Reverse Both Allowed

Structure	No.	Outfall	E1, ft	E2, ft
Stand Pipe	SP	---> CV	623.100	624.000
Culvert-Circular	CV	---> TW	618.300	624.000
TW SETUP, DS Channel				

OUTLET STRUCTURE INPUT DATA

Structure ID	=	SP
Structure Type	=	Stand Pipe

# of Openings	=	1
Invert Elev.	=	623.10 ft
Diameter	=	2.0000 ft
Orifice Area	=	3.1416 sq.ft
Orifice Coeff.	=	.600
Weir Length	=	6.28 ft
Weir Coeff.	=	3.000
K, Submerged	=	.000
K, Reverse	=	1.000
Kb, Barrel	=	.000000 (per ft of full flow)
Barrel Length	=	.00 ft
Mannings n	=	.0000

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OUTLET STRUCTURE INPUT DATA

Structure ID = CV
Structure Type = Culvert-Circular

No. Barrels = 1
Barrel Diameter = 1.5000 ft
Upstream Invert = 618.30 ft
Dnstream Invert = 618.00 ft
Horiz. Length = 30.00 ft
Barrel Length = 30.00 ft
Barrel Slope = .01000 ft/ft

OUTLET CONTROL DATA...

Mannings n = .0130
Ke = .5000 (forward entrance loss)
Kb = .018213 (per ft of full flow)
Kr = .5000 (reverse entrance loss)
HW Convergence = .001 +/- ft

INLET CONTROL DATA...

Equation form = 1
Inlet Control K = .0078
Inlet Control M = 2.0000
Inlet Control c = .03790
Inlet Control Y = .6900
T1 ratio (HW/D) = 1.131
T2 ratio (HW/D) = 1.291
Slope Factor = -.500

Use unsubmerged inlet control Form 1 equ. below T1 elev.
Use submerged inlet control Form 1 equ. above T2 elev.

In transition zone between unsubmerged and submerged inlet control,
interpolate between flows at T1 & T2...
At T1 Elev = 620.00 ft ---> Flow = 7.58 cfs
At T2 Elev = 620.24 ft ---> Flow = 8.66 cfs

Structure ID = TW
Structure Type = TW SETUP, DS Channel

FREE OUTFALL CONDITIONS SPECIFIED

CONVERGENCE TOLERANCES...

Maximum Iterations= 30
Min. TW tolerance = .01 ft
Max. TW tolerance = .01 ft
Min. HW tolerance = .01 ft
Max. HW tolerance = .01 ft
Min. Q tolerance = .10 cfs
Max. Q tolerance = .10 cfs

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***** COMPOSITE OUTFLOW SUMMARY *****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
619.00	.00	Free	Outfall	(no Q: SP,CV)
619.10	.00	Free	Outfall	(no Q: SP,CV)
619.20	.00	Free	Outfall	(no Q: SP,CV)
619.30	.00	Free	Outfall	(no Q: SP,CV)
619.40	.00	Free	Outfall	(no Q: SP,CV)
619.50	.00	Free	Outfall	(no Q: SP,CV)
619.60	.00	Free	Outfall	(no Q: SP,CV)
619.70	.00	Free	Outfall	(no Q: SP,CV)
619.80	.00	Free	Outfall	(no Q: SP,CV)
619.90	.00	Free	Outfall	(no Q: SP,CV)
620.00	.00	Free	Outfall	(no Q: SP,CV)
620.10	.00	Free	Outfall	(no Q: SP,CV)
620.20	.00	Free	Outfall	(no Q: SP,CV)
620.30	.00	Free	Outfall	(no Q: SP,CV)
620.40	.00	Free	Outfall	(no Q: SP,CV)
620.50	.00	Free	Outfall	(no Q: SP,CV)
620.60	.00	Free	Outfall	(no Q: SP,CV)
620.70	.00	Free	Outfall	(no Q: SP,CV)
620.80	.00	Free	Outfall	(no Q: SP,CV)
620.90	.00	Free	Outfall	(no Q: SP,CV)
621.00	.00	Free	Outfall	(no Q: SP,CV)
621.10	.00	Free	Outfall	(no Q: SP,CV)
621.20	.00	Free	Outfall	(no Q: SP,CV)
621.30	.00	Free	Outfall	(no Q: SP,CV)
621.40	.00	Free	Outfall	(no Q: SP,CV)
621.50	.00	Free	Outfall	(no Q: SP,CV)
621.60	.00	Free	Outfall	(no Q: SP,CV)
621.70	.00	Free	Outfall	(no Q: SP,CV)
621.80	.00	Free	Outfall	(no Q: SP,CV)
621.90	.00	Free	Outfall	(no Q: SP,CV)
622.00	.00	Free	Outfall	(no Q: SP,CV)
622.10	.00	Free	Outfall	(no Q: SP,CV)
622.20	.00	Free	Outfall	(no Q: SP,CV)
622.30	.00	Free	Outfall	(no Q: SP,CV)
622.40	.00	Free	Outfall	(no Q: SP,CV)
622.50	.00	Free	Outfall	(no Q: SP,CV)
622.60	.00	Free	Outfall	(no Q: SP,CV)
622.70	.00	Free	Outfall	(no Q: SP,CV)

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***** COMPOSITE OUTFLOW SUMMARY *****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
622.80	.00	Free	Outfall	(no Q: SP,CV)
622.90	.00	Free	Outfall	(no Q: SP,CV)
623.00	.00	Free	Outfall	(no Q: SP,CV)
623.10	.00	Free	Outfall	(no Q: SP,CV)
623.20	.60	Free	Outfall	SP,CV
623.30	1.69	Free	Outfall	SP,CV
623.40	3.10	Free	Outfall	SP,CV
623.50	4.77	Free	Outfall	SP,CV
623.60	6.66	Free	Outfall	SP,CV
623.70	8.76	Free	Outfall	SP,CV
623.80	11.04	Free	Outfall	SP,CV
623.90	13.49	Free	Outfall	SP,CV
624.00	14.34	Free	Outfall	SP,CV

SUMMARY FOR HYDROGRAPH ADDITION
 at Node: SED BASIN A IN

HYG Directory: H:\PONDPACK\A12500PLUS\12901\

```

=====
Upstream Link ID  Upstream Node ID  HYG file      HYG ID        HYG tag
-----
BASIN A INFLOW    BASIN A LABEL                                Basin A 25 in  25 Yr
=====
  
```

INFLOWS TO: SED BASIN A IN

```

-----
HYG file          HYG ID          HYG tag          Volume      Peak Time     Peak Flow
                   cu.ft          min              cfs
-----
                Basin A 25 in    25 Yr            4219         5.00         3.52
  
```

TOTAL FLOW INTO: SED BASIN A IN

```

-----
HYG file          HYG ID          HYG tag          Volume      Peak Time     Peak Flow
                   cu.ft          min              cfs
-----
                SED BASIN A IN    25              4219         5.00         3.52
  
```

TOTAL NODE INFLOW...
 HYG file =
 HYG ID = SED BASIN A IN
 HYG Tag = 25

 Peak Discharge = 3.52 cfs
 Time to Peak = 5.00 min
 HYG Volume = 4219 cu.ft

HYDROGRAPH ORDINATES (cfs)
 Output Time increment = 1.00 min
 Time on left represents time for first value in each row.

Time min	Time on left represents time for first value in each row.				
.00	.00	.70	1.40	2.10	2.80
5.00	3.52	3.52	3.52	3.52	3.52
10.00	3.52	3.52	3.52	3.52	3.52
15.00	3.52	3.52	3.52	3.52	3.52
20.00	3.52	2.80	2.10	1.40	.70
25.00	.00				

LEVEL POOL ROUTING SUMMARY

HYG Dir = H:\PONDPACK\A12500PLUS\12901\
Inflow HYG file = NONE STORED - SED BASIN A IN 25
Outflow HYG file = NONE STORED - SED BASIN A OUT 25

Pond Node Data = SED BASIN A
Pond Volume Data = SED BASIN A
Pond Outlet Data = Basin A outflow

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 623.10 ft
Starting Volume = 5272 cu.ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout = .00 cfs
Time Increment = 1.00 min

INFLOW/OUTFLOW HYDROGRAPH SUMMARY

=====
Peak Inflow = 3.52 cfs at 5.00 min
Peak Outflow = 3.52 cfs at 19.00 min
=====

Peak Elevation = 623.43 ft
Peak Storage = 5922 cu.ft
=====

MASS BALANCE (cu.ft)

+ Initial Vol = 5272
+ HYG Vol IN = 4219
- Infiltration = 0
- HYG Vol OUT = 4219
- Retained Vol = 5272

Unrouted Vol = 0 cu.ft (.000% of Inflow Volume)

Index of Starting Page Numbers for ID Names

----- B -----

BASIN A LABEL 25... 1.01
Basin A outflow... 3.01, 3.04

----- S -----

SED BASIN A... 2.01
SED BASIN A IN 25... 4.01, 4.03

**25 YEAR, 20 MINUTE
POND 9
Sediment Storage
Routing Calculations
For Temporary Sediment Basin B**

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***** RUNOFF HYDROGRAPHS *****

BASIN B INFLOW.. 25
Read HYG 1.01

***** POND VOLUMES *****

SED BASIN B..... Vol: Planimeter 2.01

***** OUTLET STRUCTURES *****

Basin B outflow Outlet Input Data 3.01
Composite Rating Curve 3.04

***** POND ROUTING *****

SED BASIN B IN 25
Node: Pond Inflow Summary 4.01

SED BASIN B OUT 25
Pond Routing Summary 4.03

HYG file =
 HYG ID = Basin B 25 in
 HYG Tag = 25 Yr

 Peak Discharge = 3.25 cfs
 Time to Peak = 5.00 min
 HYG Volume = 3900 cu.ft

HYDROGRAPH ORDINATES (cfs)
 Output Time increment = 1.00 min
 Time on left represents time for first value in each row.

Time min					
.00	.00	.65	1.30	1.95	2.60
5.00	3.25	3.25	3.25	3.25	3.25
10.00	3.25	3.25	3.25	3.25	3.25
15.00	3.25	3.25	3.25	3.25	3.25
20.00	3.25	2.60	1.95	1.30	.65
25.00	.00	.00	.00	.00	.00
30.00	.00	.00	.00	.00	.00
35.00	.00	.00	.00	.00	.00
40.00	.00	.00	.00	.00	.00
45.00	.00	.00	.00	.00	.00
50.00	.00	.00	.00	.00	.00
55.00	.00	.00	.00	.00	.00
60.00	.00	.00	.00	.00	.00
65.00	.00	.00	.00	.00	.00
70.00	.00	.00	.00	.00	.00
75.00	.00	.00	.00	.00	.00
80.00	.00	.00	.00	.00	.00
85.00	.00	.00	.00	.00	.00
90.00	.00	.00	.00	.00	.00
95.00	.00	.00	.00	.00	.00
100.00	.00	.00	.00	.00	.00
105.00	.00	.00	.00	.00	.00
110.00	.00	.00	.00	.00	.00
115.00	.00	.00	.00	.00	.00
120.00	.00	.00	.00	.00	.00
125.00	.00	.00	.00	.00	.00
130.00	.00	.00	.00	.00	.00
135.00	.00	.00	.00	.00	.00
140.00	.00	.00	.00	.00	.00
145.00	.00	.00	.00	.00	.00
150.00	.00	.00	.00	.00	.00

HYDROGRAPH ORDINATES (cfs)
 Output Time increment = 1.00 min
 Time on left represents time for first value in each row.

Time min						
155.00	.00	.00	.00	.00	.00	.00
160.00	.00	.00	.00	.00	.00	.00
165.00	.00	.00	.00	.00	.00	.00
170.00	.00	.00	.00	.00	.00	.00
175.00	.00	.00	.00	.00	.00	.00
180.00	.00	.00	.00	.00	.00	.00
185.00	.00	.00	.00	.00	.00	.00
190.00	.00	.00	.00	.00	.00	.00
195.00	.00	.00	.00	.00	.00	.00
200.00	.00	.00	.00	.00	.00	.00
205.00	.00	.00	.00	.00	.00	.00
210.00	.00	.00	.00	.00	.00	.00
215.00	.00	.00	.00	.00	.00	.00
220.00	.00	.00	.00	.00	.00	.00
225.00	.00	.00	.00	.00	.00	.00
230.00	.00	.00	.00	.00	.00	.00
235.00	.00	.00	.00	.00	.00	.00
240.00	.00	.00	.00	.00	.00	.00
245.00	.00	.00	.00	.00	.00	.00
250.00	.00					

File.... H:\PONDPACK\A12500PLUS\12901\SEDIMENT CALCS 12901 3-25-05 JD.PPW

POND VOLUME CALCULATIONS

Planimeter scale: 1.00 ft/in

Elevation (ft)	Planimeter (sq.in)	Area (acres)	A1+A2+sqrt(A1*A2) (acres)	Volume (cu.ft)	Volume Sum (cu.ft)
621.00	631.000	.0145	.0000	0	0
622.00	975.000	.0224	.0549	797	797
624.00	1514.000	.0348	.0850	2469	3266
626.00	2169.000	.0498	.1262	3663	6930

POND VOLUME EQUATIONS

* Incremental volume computed by the Conic Method for Reservoir Volumes.

$$\text{Volume} = (1/3) * (\text{EL2}-\text{EL1}) * (\text{Area1} + \text{Area2} + \text{sq.rt.}(\text{Area1}*\text{Area2}))$$

where: EL1, EL2 = Lower and upper elevations of the increment
Area1, Area2 = Areas computed for EL1, EL2, respectively
Volume = Incremental volume between EL1 and EL2

REQUESTED POND WS ELEVATIONS:

Min. Elev.= 621.00 ft
Increment = .10 ft
Max. Elev.= 626.00 ft

OUTLET CONNECTIVITY

---> Forward Flow Only (UpStream to DnStream)
<--- Reverse Flow Only (DnStream to UpStream)
<---> Forward and Reverse Both Allowed

Structure	No.		Outfall	E1, ft	E2, ft
Stand Pipe	SP	--->	CV	624.960	626.000
Culvert-Circular	CV	--->	TW	618.380	626.000
TW SETUP, DS Channel					

OUTLET STRUCTURE INPUT DATA

Structure ID	=	SP
Structure Type	=	Stand Pipe

# of Openings	=	1
Invert Elev.	=	624.96 ft
Diameter	=	2.0000 ft
Orifice Area	=	3.1416 sq.ft
Orifice Coeff.	=	.600
Weir Length	=	6.28 ft
Weir Coeff.	=	3.000
K, Submerged	=	.000
K, Reverse	=	1.000
Kb, Barrel	=	.000000 (per ft of full flow)
Barrel Length	=	.00 ft
Mannings n	=	.0000

File.... H:\PONDPACK\A12500PLUS\12901\SEDIMENT CALCS 12901 3-25-05 JD.PPW

OUTLET STRUCTURE INPUT DATA

Structure ID = CV
Structure Type = Culvert-Circular

No. Barrels = 1
Barrel Diameter = 1.5000 ft
Upstream Invert = 618.38 ft
Dnstream Invert = 618.00 ft
Horiz. Length = 38.00 ft
Barrel Length = 38.00 ft
Barrel Slope = .01000 ft/ft

OUTLET CONTROL DATA...

Mannings n = .0130
Ke = .5000 (forward entrance loss)
Kb = .018213 (per ft of full flow)
Kr = .5000 (reverse entrance loss)
HW Convergence = .001 +/- ft

INLET CONTROL DATA...

Equation form = 1
Inlet Control K = .0078
Inlet Control M = 2.0000
Inlet Control c = .03790
Inlet Control Y = .6900
T1 ratio (HW/D) = 1.131
T2 ratio (HW/D) = 1.291
Slope Factor = -.500

Use unsubmerged inlet control Form 1 equ. below T1 elev.
Use submerged inlet control Form 1 equ. above T2 elev.

In transition zone between unsubmerged and submerged inlet control,
interpolate between flows at T1 & T2...
At T1 Elev = 620.08 ft ---> Flow = 7.58 cfs
At T2 Elev = 620.32 ft ---> Flow = 8.66 cfs

Structure ID = TW
Structure Type = TW SETUP, DS Channel

FREE OUTFALL CONDITIONS SPECIFIED

CONVERGENCE TOLERANCES...

Maximum Iterations= 30
Min. TW tolerance = .01 ft
Max. TW tolerance = .01 ft
Min. HW tolerance = .01 ft
Max. HW tolerance = .01 ft
Min. Q tolerance = .10 cfs
Max. Q tolerance = .10 cfs

File.... H:\PONDPACK\A12500PLUS\12901\SEDIMENT CALCS 12901 3-25-05 JD.PPW

***** COMPOSITE OUTFLOW SUMMARY *****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
621.00	.00	Free Outfall		(no Q: SP,CV)
621.10	.00	Free Outfall		(no Q: SP,CV)
621.20	.00	Free Outfall		(no Q: SP,CV)
621.30	.00	Free Outfall		(no Q: SP,CV)
621.40	.00	Free Outfall		(no Q: SP,CV)
621.50	.00	Free Outfall		(no Q: SP,CV)
621.60	.00	Free Outfall		(no Q: SP,CV)
621.70	.00	Free Outfall		(no Q: SP,CV)
621.80	.00	Free Outfall		(no Q: SP,CV)
621.90	.00	Free Outfall		(no Q: SP,CV)
622.00	.00	Free Outfall		(no Q: SP,CV)
622.10	.00	Free Outfall		(no Q: SP,CV)
622.20	.00	Free Outfall		(no Q: SP,CV)
622.30	.00	Free Outfall		(no Q: SP,CV)
622.40	.00	Free Outfall		(no Q: SP,CV)
622.50	.00	Free Outfall		(no Q: SP,CV)
622.60	.00	Free Outfall		(no Q: SP,CV)
622.70	.00	Free Outfall		(no Q: SP,CV)
622.80	.00	Free Outfall		(no Q: SP,CV)
622.90	.00	Free Outfall		(no Q: SP,CV)
623.00	.00	Free Outfall		(no Q: SP,CV)
623.10	.00	Free Outfall		(no Q: SP,CV)
623.20	.00	Free Outfall		(no Q: SP,CV)
623.30	.00	Free Outfall		(no Q: SP,CV)
623.40	.00	Free Outfall		(no Q: SP,CV)
623.50	.00	Free Outfall		(no Q: SP,CV)
623.60	.00	Free Outfall		(no Q: SP,CV)
623.70	.00	Free Outfall		(no Q: SP,CV)
623.80	.00	Free Outfall		(no Q: SP,CV)
623.90	.00	Free Outfall		(no Q: SP,CV)
624.00	.00	Free Outfall		(no Q: SP,CV)
624.10	.00	Free Outfall		(no Q: SP,CV)
624.20	.00	Free Outfall		(no Q: SP,CV)
624.30	.00	Free Outfall		(no Q: SP,CV)
624.40	.00	Free Outfall		(no Q: SP,CV)
624.50	.00	Free Outfall		(no Q: SP,CV)
624.60	.00	Free Outfall		(no Q: SP,CV)
624.70	.00	Free Outfall		(no Q: SP,CV)

File.... H:\PONDPACK\A12500PLUS\12901\SEDIMENT CALCS 12901 3-25-05 JD.PPW

***** COMPOSITE OUTFLOW SUMMARY *****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
624.80	.00	Free Outfall		(no Q: SP,CV)
624.90	.00	Free Outfall		(no Q: SP,CV)
624.96	.00	Free Outfall		(no Q: SP,CV)
625.00	.15	Free Outfall		SP,CV
625.10	.99	Free Outfall		SP,CV
625.20	2.22	Free Outfall		SP,CV
625.30	3.74	Free Outfall		SP,CV
625.40	5.50	Free Outfall		SP,CV
625.50	7.48	Free Outfall		SP,CV
625.60	9.65	Free Outfall		SP,CV
625.70	12.00	Free Outfall		SP,CV
625.80	13.86	Free Outfall		SP,CV
625.90	14.66	Free Outfall		SP,CV
626.00	15.42	Free Outfall		SP,CV

SUMMARY FOR HYDROGRAPH ADDITION
 at Node: SED BASIN B IN

HYG Directory: H:\PONDPACK\A12500PLUS\12901\

```

=====
Upstream Link ID  Upstream Node ID  HYG file  HYG ID  HYG tag
-----
BASIN B INFLOW    BASIN B INFLOW                    Basin B 25 in  25 Yr
=====
  
```

```

INFLOWS TO:  SED BASIN B IN
-----
HYG file      HYG ID          HYG tag          Volume      Peak Time     Peak Flow
              HYG ID          HYG tag          cu.ft       min           cfs
-----
              Basin B 25 in    25 Yr            3900        5.00         3.25
  
```

```

TOTAL FLOW INTO:  SED BASIN B IN
-----
HYG file      HYG ID          HYG tag          Volume      Peak Time     Peak Flow
              HYG ID          HYG tag          cu.ft       min           cfs
-----
              SED BASIN B IN  25              3900        5.00         3.25
  
```

TOTAL NODE INFLOW...
 HYG file =
 HYG ID = SED BASIN B IN
 HYG Tag = 25

 Peak Discharge = 3.25 cfs
 Time to Peak = 5.00 min
 HYG Volume = 3900 cu.ft

HYDROGRAPH ORDINATES (cfs)
 Output Time increment = 1.00 min
 Time on left represents time for first value in each row.

Time min	Time on left represents time for first value in each row.				
.00	.00	.65	1.30	1.95	2.60
5.00	3.25	3.25	3.25	3.25	3.25
10.00	3.25	3.25	3.25	3.25	3.25
15.00	3.25	3.25	3.25	3.25	3.25
20.00	3.25	2.60	1.95	1.30	.65
25.00	.00				

LEVEL POOL ROUTING SUMMARY

HYG Dir = H:\PONDPACK\A12500PLUS\12901\
Inflow HYG file = NONE STORED - SED BASIN B IN 25
Outflow HYG file = NONE STORED - SED BASIN B OUT 25

Pond Node Data = SED BASIN B
Pond Volume Data = SED BASIN B
Pond Outlet Data = Basin B outflow

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 624.96 ft
Starting Volume = 4861 cu.ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = 1.00 min

INFLOW/OUTFLOW HYDROGRAPH SUMMARY

=====
Peak Inflow = 3.25 cfs at 5.00 min
Peak Outflow = 3.25 cfs at 19.00 min
=====

Peak Elevation = 625.27 ft
Peak Storage = 5435 cu.ft
=====

MASS BALANCE (cu.ft)

+ Initial Vol = 4861
+ HYG Vol IN = 3900
- Infiltration = 0
- HYG Vol OUT = 3899
- Retained Vol = 4863

Unrouted Vol = 0 cu.ft (.001% of Inflow Volume)

Index of Starting Page Numbers for ID Names

----- B -----

BASIN B INFLOW 25... 1.01

Basin B outflow... 3.01, 3.04

----- S -----

SED BASIN B... 2.01

SED BASIN B IN 25... 4.01, 4.03

**25 YEAR, 20 MINUTE
POND 9
Sediment Storage
Routing Calculations
For Temporary Sediment Basin C**

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***** RUNOFF HYDROGRAPHS *****

BASIN C INFLOW.. 25
 Read HYG 1.01

***** POND VOLUMES *****

SED BASIN C..... Vol: Planimeter 2.01

***** OUTLET STRUCTURES *****

Basin C outflow Outlet Input Data 3.01
 Composite Rating Curve 3.04

***** POND ROUTING *****

SED BASIN C IN 25
 Node: Pond Inflow Summary 4.01

SED BASIN C OUT 25
 Pond Routing Summary 4.03

HYG file =
 HYG ID = Basin C 25 in
 HYG Tag = 25 Yr

 Peak Discharge = 7.63 cfs
 Time to Peak = 5.00 min
 HYG Volume = 9156 cu.ft

HYDROGRAPH ORDINATES (cfs)
 Output Time increment = 1.00 min
 Time on left represents time for first value in each row.

Time min					
.00	.00	1.53	3.05	4.58	6.10
5.00	7.63	7.63	7.63	7.63	7.63
10.00	7.63	7.63	7.63	7.63	7.63
15.00	7.63	7.63	7.63	7.63	7.63
20.00	7.63	6.10	4.58	3.05	1.53
25.00	.00	.00	.00	.00	.00
30.00	.00	.00	.00	.00	.00
35.00	.00	.00	.00	.00	.00
40.00	.00	.00	.00	.00	.00
45.00	.00	.00	.00	.00	.00
50.00	.00	.00	.00	.00	.00
55.00	.00	.00	.00	.00	.00
60.00	.00	.00	.00	.00	.00
65.00	.00	.00	.00	.00	.00
70.00	.00	.00	.00	.00	.00
75.00	.00	.00	.00	.00	.00
80.00	.00	.00	.00	.00	.00
85.00	.00	.00	.00	.00	.00
90.00	.00	.00	.00	.00	.00
95.00	.00	.00	.00	.00	.00
100.00	.00	.00	.00	.00	.00
105.00	.00	.00	.00	.00	.00
110.00	.00	.00	.00	.00	.00
115.00	.00	.00	.00	.00	.00
120.00	.00	.00	.00	.00	.00
125.00	.00	.00	.00	.00	.00
130.00	.00	.00	.00	.00	.00
135.00	.00	.00	.00	.00	.00
140.00	.00	.00	.00	.00	.00
145.00	.00	.00	.00	.00	.00
150.00	.00	.00	.00	.00	.00

HYDROGRAPH ORDINATES (cfs)

Output Time increment = 1.00 min

Time on left represents time for first value in each row.

Time min					
155.00		.00	.00	.00	.00
160.00		.00	.00	.00	.00
165.00		.00	.00	.00	.00
170.00		.00	.00	.00	.00
175.00		.00	.00	.00	.00
180.00		.00	.00	.00	.00
185.00		.00	.00	.00	.00
190.00		.00	.00	.00	.00
195.00		.00	.00	.00	.00
200.00		.00	.00	.00	.00
205.00		.00	.00	.00	.00
210.00		.00	.00	.00	.00
215.00		.00	.00	.00	.00
220.00		.00	.00	.00	.00
225.00		.00	.00	.00	.00
230.00		.00	.00	.00	.00
235.00		.00	.00	.00	.00
240.00		.00	.00	.00	.00
245.00		.00	.00	.00	.00
250.00		.00			

File.... H:\PONDPACK\A12500PLUS\12901\SEDIMENT CALCS 12901 3-25-05 JD.PPW

POND VOLUME CALCULATIONS

Planimeter scale: 1.00 ft/in

Elevation (ft)	Planimeter (sq.in)	Area (acres)	A1+A2+sqrt(A1*A2) (acres)	Volume (cu.ft)	Volume Sum (cu.ft)
600.00	2340.000	.0537	.0000	0	0
602.00	3195.000	.0733	.1898	5513	5513
604.00	4153.000	.0953	.2523	7327	12840
606.00	5438.000	.1248	.3293	9562	22402

POND VOLUME EQUATIONS

* Incremental volume computed by the Conic Method for Reservoir Volumes.

$$\text{Volume} = (1/3) * (\text{EL2}-\text{EL1}) * (\text{Area1} + \text{Area2} + \text{sq.rt.}(\text{Area1}*\text{Area2}))$$

where: EL1, EL2 = Lower and upper elevations of the increment
Area1, Area2 = Areas computed for EL1, EL2, respectively
Volume = Incremental volume between EL1 and EL2

REQUESTED POND WS ELEVATIONS:

Min. Elev.= 600.00 ft
Increment = .10 ft
Max. Elev.= 606.00 ft

OUTLET CONNECTIVITY

---> Forward Flow Only (UpStream to DnStream)
<--- Reverse Flow Only (DnStream to UpStream)
<---> Forward and Reverse Both Allowed

Structure	No.	Outfall	E1, ft	E2, ft
Stand Pipe	SP	---> CV	604.000	606.000
Culvert-Circular	CV	---> TW	597.450	606.000
TW SETUP, DS Channel				

OUTLET STRUCTURE INPUT DATA

Structure ID	=	SP
Structure Type	=	Stand Pipe

# of Openings	=	1
Invert Elev.	=	604.00 ft
Diameter	=	2.0000 ft
Orifice Area	=	3.1416 sq.ft
Orifice Coeff.	=	.600
Weir Length	=	6.28 ft
Weir Coeff.	=	3.000
K, Submerged	=	.000
K, Reverse	=	1.000
Kb, Barrel	=	.000000 (per ft of full flow)
Barrel Length	=	.00 ft
Mannings n	=	.0000

File... H:\PONDPACK\A12500PLUS\12901\SEDIMENT CALCS 12901 3-25-05 JD.PPW

OUTLET STRUCTURE INPUT DATA

Structure ID = CV
Structure Type = Culvert-Circular

No. Barrels = 1
Barrel Diameter = 1.5000 ft
Upstream Invert = 597.45 ft
Dnstream Invert = 597.00 ft
Horiz. Length = 45.00 ft
Barrel Length = 45.00 ft
Barrel Slope = .01000 ft/ft

OUTLET CONTROL DATA...

Mannings n = .0130
Ke = .5000 (forward entrance loss)
Kb = .018213 (per ft of full flow)
Kr = .5000 (reverse entrance loss)
HW Convergence = .001 +/- ft

INLET CONTROL DATA...

Equation form = 1
Inlet Control K = .0078
Inlet Control M = 2.0000
Inlet Control c = .03790
Inlet Control Y = .6900
T1 ratio (HW/D) = 1.131
T2 ratio (HW/D) = 1.291
Slope Factor = -.500

Use unsubmerged inlet control Form 1 equ. below T1 elev.
Use submerged inlet control Form 1 equ. above T2 elev.

In transition zone between unsubmerged and submerged inlet control,
interpolate between flows at T1 & T2...

At T1 Elev = 599.15 ft ---> Flow = 7.58 cfs
At T2 Elev = 599.39 ft ---> Flow = 8.66 cfs

Structure ID = TW
Structure Type = TW SETUP, DS Channel

FREE OUTFALL CONDITIONS SPECIFIED

CONVERGENCE TOLERANCES...

Maximum Iterations= 30
Min. TW tolerance = .01 ft
Max. TW tolerance = .01 ft
Min. HW tolerance = .01 ft
Max. HW tolerance = .01 ft
Min. Q tolerance = .10 cfs
Max. Q tolerance = .10 cfs

File.... H:\PONDPACK\A12500PLUS\12901\SEDIMENT CALCS 12901 3-25-05 JD.PPW

***** COMPOSITE OUTFLOW SUMMARY *****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
600.00	.00	Free Outfall		(no Q: SP,CV)
600.10	.00	Free Outfall		(no Q: SP,CV)
600.20	.00	Free Outfall		(no Q: SP,CV)
600.30	.00	Free Outfall		(no Q: SP,CV)
600.40	.00	Free Outfall		(no Q: SP,CV)
600.50	.00	Free Outfall		(no Q: SP,CV)
600.60	.00	Free Outfall		(no Q: SP,CV)
600.70	.00	Free Outfall		(no Q: SP,CV)
600.80	.00	Free Outfall		(no Q: SP,CV)
600.90	.00	Free Outfall		(no Q: SP,CV)
601.00	.00	Free Outfall		(no Q: SP,CV)
601.10	.00	Free Outfall		(no Q: SP,CV)
601.20	.00	Free Outfall		(no Q: SP,CV)
601.30	.00	Free Outfall		(no Q: SP,CV)
601.40	.00	Free Outfall		(no Q: SP,CV)
601.50	.00	Free Outfall		(no Q: SP,CV)
601.60	.00	Free Outfall		(no Q: SP,CV)
601.70	.00	Free Outfall		(no Q: SP,CV)
601.80	.00	Free Outfall		(no Q: SP,CV)
601.90	.00	Free Outfall		(no Q: SP,CV)
602.00	.00	Free Outfall		(no Q: SP,CV)
602.10	.00	Free Outfall		(no Q: SP,CV)
602.20	.00	Free Outfall		(no Q: SP,CV)
602.30	.00	Free Outfall		(no Q: SP,CV)
602.40	.00	Free Outfall		(no Q: SP,CV)
602.50	.00	Free Outfall		(no Q: SP,CV)
602.60	.00	Free Outfall		(no Q: SP,CV)
602.70	.00	Free Outfall		(no Q: SP,CV)
602.80	.00	Free Outfall		(no Q: SP,CV)
602.90	.00	Free Outfall		(no Q: SP,CV)
603.00	.00	Free Outfall		(no Q: SP,CV)
603.10	.00	Free Outfall		(no Q: SP,CV)
603.20	.00	Free Outfall		(no Q: SP,CV)
603.30	.00	Free Outfall		(no Q: SP,CV)
603.40	.00	Free Outfall		(no Q: SP,CV)
603.50	.00	Free Outfall		(no Q: SP,CV)
603.60	.00	Free Outfall		(no Q: SP,CV)
603.70	.00	Free Outfall		(no Q: SP,CV)

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***** COMPOSITE OUTFLOW SUMMARY *****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
603.80	.00	Free Outfall		(no Q: SP,CV)
603.90	.00	Free Outfall		(no Q: SP,CV)
604.00	.00	Free Outfall		(no Q: SP,CV)
604.10	.60	Free Outfall		SP,CV
604.20	1.69	Free Outfall		SP,CV
604.30	3.10	Free Outfall		SP,CV
604.40	4.77	Free Outfall		SP,CV
604.50	6.66	Free Outfall		SP,CV
604.60	8.76	Free Outfall		SP,CV
604.70	11.04	Free Outfall		SP,CV
604.80	13.49	Free Outfall		SP,CV
604.90	14.34	Free Outfall		SP,CV
605.00	15.12	Free Outfall		SP,CV
605.10	15.86	Free Outfall		SP,CV
605.20	16.56	Free Outfall		SP,CV
605.30	17.24	Free Outfall		SP,CV
605.40	17.89	Free Outfall		SP,CV
605.50	18.52	Free Outfall		SP,CV
605.60	19.13	Free Outfall		SP,CV
605.70	19.71	Free Outfall		SP,CV
605.80	20.29	Free Outfall		SP,CV
605.90	20.84	Free Outfall		SP,CV
606.00	24.90	Free Outfall		SP,CV

SUMMARY FOR HYDROGRAPH ADDITION
 at Node: SED BASIN C IN

HYG Directory: H:\PONDPACK\A12500PLUS\12901\

```

=====
Upstream Link ID  Upstream Node ID  HYG file      HYG ID      HYG tag
-----
BASIN C INFLOW    BASIN C INFLOW                                Basin C 25 in  25 Yr
=====
  
```

```

INFLOWS TO:  SED BASIN C IN
-----
HYG file      HYG ID      HYG tag      Volume      Peak Time     Peak Flow
              cu.ft       min          cfs
-----
              Basin C 25 in  25 Yr       9156        5.00         7.63
  
```

```

TOTAL FLOW INTO:  SED BASIN C IN
-----
HYG file      HYG ID      HYG tag      Volume      Peak Time     Peak Flow
              cu.ft       min          cfs
-----
              SED BASIN C IN  25         9156        5.00         7.63
  
```

TOTAL NODE INFLOW...
 HYG file =
 HYG ID = SED BASIN C IN
 HYG Tag = 25

 Peak Discharge = 7.63 cfs
 Time to Peak = 5.00 min
 HYG Volume = 9156 cu.ft

HYDROGRAPH ORDINATES (cfs)
 Output Time increment = 1.00 min
 Time on left represents time for first value in each row.

Time min	Time on left represents time for first value in each row.				
.00	.00	1.53	3.05	4.58	6.10
5.00	7.63	7.63	7.63	7.63	7.63
10.00	7.63	7.63	7.63	7.63	7.63
15.00	7.63	7.63	7.63	7.63	7.63
20.00	7.63	6.10	4.58	3.05	1.53
25.00	.00				

LEVEL POOL ROUTING SUMMARY

HYG Dir = H:\PONDPACK\A12500PLUS\12901\
Inflow HYG file = NONE STORED - SED BASIN C IN 25
Outflow HYG file = NONE STORED - SED BASIN C OUT 25

Pond Node Data = SED BASIN C
Pond Volume Data = SED BASIN C
Pond Outlet Data = Basin C outflow

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 603.71 ft
Starting Volume = 11657 cu.ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout = .00 cfs
Time Increment = 1.00 min

INFLOW/OUTFLOW HYDROGRAPH SUMMARY

=====
Peak Inflow = 7.63 cfs at 5.00 min
Peak Outflow = 7.41 cfs at 20.00 min

Peak Elevation = 604.54 ft
Peak Storage = 15152 cu.ft
=====

MASS BALANCE (cu.ft)

+ Initial Vol = 11657
+ HYG Vol IN = 9156
- Infiltration = 0
- HYG Vol OUT = 7971
- Retained Vol = 12842

Unrouted Vol = 0 cu.ft (.001% of Inflow Volume)

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----- B -----

BASIN C INFLOW 25... 1.01
Basin C outflow... 3.01, 3.04

----- S -----

SED BASIN C... 2.01
SED BASIN C IN 25... 4.01, 4.03

**25 YEAR, 20 MINUTE
POND 9
Sediment Storage
Routing Calculations
For Temporary Sediment Basin D**

Table of Contents

***** RUNOFF HYDROGRAPHS *****

BASIN D INFLOW.. 25
 Read HYG 1.01

***** POND VOLUMES *****

SED BASIN D..... Vol: Planimeter 2.01

***** OUTLET STRUCTURES *****

Basin D outflow Outlet Input Data 3.01
 Composite Rating Curve 3.04

***** POND ROUTING *****

SED BASIN D IN 25
 Node: Pond Inflow Summary 4.01

SED BASIN D OUT 25
 Pond Routing Summary 4.03

HYG file =
 HYG ID = Basin D 25 in
 HYG Tag = 25 Yr

Peak Discharge = 5.68 cfs
 Time to Peak = 5.00 min
 HYG Volume = 6816 cu.ft

HYDROGRAPH ORDINATES (cfs)
 Output Time increment = 1.00 min
 Time on left represents time for first value in each row.

Time min	Time on left represents time for first value in each row.				
.00	.00	1.14	2.27	3.41	4.54
5.00	5.68	5.68	5.68	5.68	5.68
10.00	5.68	5.68	5.68	5.68	5.68
15.00	5.68	5.68	5.68	5.68	5.68
20.00	5.68	4.54	3.41	2.27	1.14
25.00	.00	.00	.00	.00	.00
30.00	.00	.00	.00	.00	.00
35.00	.00	.00	.00	.00	.00
40.00	.00	.00	.00	.00	.00
45.00	.00	.00	.00	.00	.00
50.00	.00	.00	.00	.00	.00
55.00	.00	.00	.00	.00	.00
60.00	.00	.00	.00	.00	.00
65.00	.00	.00	.00	.00	.00
70.00	.00	.00	.00	.00	.00
75.00	.00	.00	.00	.00	.00
80.00	.00	.00	.00	.00	.00
85.00	.00	.00	.00	.00	.00
90.00	.00	.00	.00	.00	.00
95.00	.00	.00	.00	.00	.00
100.00	.00	.00	.00	.00	.00
105.00	.00	.00	.00	.00	.00
110.00	.00	.00	.00	.00	.00
115.00	.00	.00	.00	.00	.00
120.00	.00	.00	.00	.00	.00
125.00	.00	.00	.00	.00	.00
130.00	.00	.00	.00	.00	.00
135.00	.00	.00	.00	.00	.00
140.00	.00	.00	.00	.00	.00
145.00	.00	.00	.00	.00	.00
150.00	.00	.00	.00	.00	.00

HYDROGRAPH ORDINATES (cfs)
 Output Time increment = 1.00 min
 Time on left represents time for first value in each row.

Time min					
155.00	.00	.00	.00	.00	.00
160.00	.00	.00	.00	.00	.00
165.00	.00	.00	.00	.00	.00
170.00	.00	.00	.00	.00	.00
175.00	.00	.00	.00	.00	.00
180.00	.00	.00	.00	.00	.00
185.00	.00	.00	.00	.00	.00
190.00	.00	.00	.00	.00	.00
195.00	.00	.00	.00	.00	.00
200.00	.00	.00	.00	.00	.00
205.00	.00	.00	.00	.00	.00
210.00	.00	.00	.00	.00	.00
215.00	.00	.00	.00	.00	.00
220.00	.00	.00	.00	.00	.00
225.00	.00	.00	.00	.00	.00
230.00	.00	.00	.00	.00	.00
235.00	.00	.00	.00	.00	.00
240.00	.00	.00	.00	.00	.00
245.00	.00	.00	.00	.00	.00
250.00	.00				

POND VOLUME CALCULATIONS

Planimeter scale: 1.00 ft/in

Elevation (ft)	Planimeter (sq.in)	Area (acres)	A1+A2+sqr(A1*A2) (acres)	Volume (cu.ft)	Volume Sum (cu.ft)
612.00	1961.000	.0450	.0000	0	0
614.00	2845.000	.0653	.1646	4779	4779
616.00	3851.000	.0884	.2297	6671	11449
618.00	4975.000	.1142	.3031	8802	20251
620.00	6203.000	.1424	.3841	11155	31407

POND VOLUME EQUATIONS

* Incremental volume computed by the Conic Method for Reservoir Volumes.

$$\text{Volume} = (1/3) * (\text{EL2}-\text{EL1}) * (\text{Areal} + \text{Area2} + \text{sq.rt.}(\text{Areal}*\text{Area2}))$$

where: EL1, EL2 = Lower and upper elevations of the increment
Areal, Area2 = Areas computed for EL1, EL2, respectively
Volume = Incremental volume between EL1 and EL2

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REQUESTED POND WS ELEVATIONS:

Min. Elev.= 612.00 ft
Increment = .10 ft
Max. Elev.= 620.00 ft

OUTLET CONNECTIVITY

---> Forward Flow Only (UpStream to DnStream)
<--- Reverse Flow Only (DnStream to UpStream)
<---> Forward and Reverse Both Allowed

Structure	No.	Outfall	E1, ft	E2, ft
Stand Pipe	SP	---> CV	617.000	620.000
Culvert-Circular	CV	---> TW	609.100	620.000
TW SETUP, DS Channel				

OUTLET STRUCTURE INPUT DATA

Structure ID	=	SP
Structure Type	=	Stand Pipe

# of Openings	=	1
Invert Elev.	=	617.00 ft
Diameter	=	2.0000 ft
Orifice Area	=	3.1416 sq.ft
Orifice Coeff.	=	.600
Weir Length	=	6.28 ft
Weir Coeff.	=	3.000
K, Submerged	=	.000
K, Reverse	=	1.000
Kb, Barrel	=	.000000 (per ft of full flow)
Barrel Length	=	.00 ft
Mannings n	=	.0000

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OUTLET STRUCTURE INPUT DATA

Structure ID = CV
Structure Type = Culvert-Circular

No. Barrels = 1
Barrel Diameter = 1.5000 ft
Upstream Invert = 609.10 ft
Dnstream Invert = 600.00 ft
Horiz. Length = 65.00 ft
Barrel Length = 65.63 ft
Barrel Slope = .14000 ft/ft

OUTLET CONTROL DATA...

Mannings n = .0130
Ke = .5000 (forward entrance loss)
Kb = .018213 (per ft of full flow)
Kr = .5000 (reverse entrance loss)
HW Convergence = .001 +/- ft

INLET CONTROL DATA...

Equation form = 1
Inlet Control K = .0078
Inlet Control M = 2.0000
Inlet Control c = .03790
Inlet Control Y = .6900
T1 ratio (HW/D) = 1.066
T2 ratio (HW/D) = 1.226
Slope Factor = -.500

Use unsubmerged inlet control Form 1 equ. below T1 elev.
Use submerged inlet control Form 1 equ. above T2 elev.

In transition zone between unsubmerged and submerged inlet control,
interpolate between flows at T1 & T2...

At T1 Elev = 610.70 ft ---> Flow = 7.58 cfs
At T2 Elev = 610.94 ft ---> Flow = 8.66 cfs

Structure ID = TW
Structure Type = TW SETUP, DS Channel

FREE OUTFALL CONDITIONS SPECIFIED

CONVERGENCE TOLERANCES...

Maximum Iterations= 30
Min. TW tolerance = .01 ft
Max. TW tolerance = .01 ft
Min. HW tolerance = .01 ft
Max. HW tolerance = .01 ft
Min. Q tolerance = .10 cfs
Max. Q tolerance = .10 cfs

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***** COMPOSITE OUTFLOW SUMMARY *****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
612.00	.00	Free Outfall		(no Q: SP,CV)
612.10	.00	Free Outfall		(no Q: SP,CV)
612.20	.00	Free Outfall		(no Q: SP,CV)
612.30	.00	Free Outfall		(no Q: SP,CV)
612.40	.00	Free Outfall		(no Q: SP,CV)
612.50	.00	Free Outfall		(no Q: SP,CV)
612.60	.00	Free Outfall		(no Q: SP,CV)
612.70	.00	Free Outfall		(no Q: SP,CV)
612.80	.00	Free Outfall		(no Q: SP,CV)
612.90	.00	Free Outfall		(no Q: SP,CV)
613.00	.00	Free Outfall		(no Q: SP,CV)
613.10	.00	Free Outfall		(no Q: SP,CV)
613.20	.00	Free Outfall		(no Q: SP,CV)
613.30	.00	Free Outfall		(no Q: SP,CV)
613.40	.00	Free Outfall		(no Q: SP,CV)
613.50	.00	Free Outfall		(no Q: SP,CV)
613.60	.00	Free Outfall		(no Q: SP,CV)
613.70	.00	Free Outfall		(no Q: SP,CV)
613.80	.00	Free Outfall		(no Q: SP,CV)
613.90	.00	Free Outfall		(no Q: SP,CV)
614.00	.00	Free Outfall		(no Q: SP,CV)
614.10	.00	Free Outfall		(no Q: SP,CV)
614.20	.00	Free Outfall		(no Q: SP,CV)
614.30	.00	Free Outfall		(no Q: SP,CV)
614.40	.00	Free Outfall		(no Q: SP,CV)
614.50	.00	Free Outfall		(no Q: SP,CV)
614.60	.00	Free Outfall		(no Q: SP,CV)
614.70	.00	Free Outfall		(no Q: SP,CV)
614.80	.00	Free Outfall		(no Q: SP,CV)
614.90	.00	Free Outfall		(no Q: SP,CV)
615.00	.00	Free Outfall		(no Q: SP,CV)
615.10	.00	Free Outfall		(no Q: SP,CV)
615.20	.00	Free Outfall		(no Q: SP,CV)
615.30	.00	Free Outfall		(no Q: SP,CV)
615.40	.00	Free Outfall		(no Q: SP,CV)
615.50	.00	Free Outfall		(no Q: SP,CV)
615.60	.00	Free Outfall		(no Q: SP,CV)
615.70	.00	Free Outfall		(no Q: SP,CV)

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***** COMPOSITE OUTFLOW SUMMARY *****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
615.80	.00	Free Outfall		(no Q: SP,CV)
615.90	.00	Free Outfall		(no Q: SP,CV)
616.00	.00	Free Outfall		(no Q: SP,CV)
616.10	.00	Free Outfall		(no Q: SP,CV)
616.20	.00	Free Outfall		(no Q: SP,CV)
616.30	.00	Free Outfall		(no Q: SP,CV)
616.40	.00	Free Outfall		(no Q: SP,CV)
616.50	.00	Free Outfall		(no Q: SP,CV)
616.60	.00	Free Outfall		(no Q: SP,CV)
616.70	.00	Free Outfall		(no Q: SP,CV)
616.80	.00	Free Outfall		(no Q: SP,CV)
616.90	.00	Free Outfall		(no Q: SP,CV)
617.00	.00	Free Outfall		(no Q: SP,CV)
617.10	.60	Free Outfall		SP,CV
617.20	1.69	Free Outfall		SP,CV
617.30	3.10	Free Outfall		SP,CV
617.40	4.77	Free Outfall		SP,CV
617.50	6.66	Free Outfall		SP,CV
617.60	8.76	Free Outfall		SP,CV
617.70	11.04	Free Outfall		SP,CV
617.80	13.49	Free Outfall		SP,CV
617.90	14.34	Free Outfall		SP,CV
618.00	15.12	Free Outfall		SP,CV
618.10	15.86	Free Outfall		SP,CV
618.20	16.56	Free Outfall		SP,CV
618.30	17.24	Free Outfall		SP,CV
618.40	17.89	Free Outfall		SP,CV
618.50	18.52	Free Outfall		SP,CV
618.60	19.13	Free Outfall		SP,CV
618.70	19.71	Free Outfall		SP,CV
618.80	20.29	Free Outfall		SP,CV
618.90	20.84	Free Outfall		SP,CV
619.00	21.38	Free Outfall		SP,CV
619.10	21.91	Free Outfall		SP,CV
619.20	22.43	Free Outfall		SP,CV
619.30	22.93	Free Outfall		SP,CV
619.40	23.42	Free Outfall		SP,CV
619.50	23.91	Free Outfall		SP,CV

File.... H:\PONDPACK\A12500PLUS\12901\SEDIMENT CALCS 12901 3-25-05 JD.PPW

***** COMPOSITE OUTFLOW SUMMARY *****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
619.60	28.08	Free Outfall		SP,CV
619.70	28.23	Free Outfall		SP,CV
619.80	28.37	Free Outfall		SP,CV
619.90	28.52	Free Outfall		SP,CV
620.00	28.66	Free Outfall		SP,CV

SUMMARY FOR HYDROGRAPH ADDITION
 at Node: SED BASIN D IN

HYG Directory: H:\PONDPACK\A12500PLUS\12901\

```

=====
Upstream Link ID  Upstream Node ID  HYG file  HYG ID  HYG tag
-----
BASIN D INFLOW    BASIN D INFLOW                    Basin D 25 in  25 Yr
=====
  
```

INFLOWS TO: SED BASIN D IN

```

-----
HYG file  HYG ID  HYG tag  Volume  Peak Time  Peak Flow
          cu.ft  min      cfs
-----
          Basin D 25 in  25 Yr    6816     5.00     5.68
  
```

TOTAL FLOW INTO: SED BASIN D IN

```

-----
HYG file  HYG ID  HYG tag  Volume  Peak Time  Peak Flow
          cu.ft  min      cfs
-----
          SED BASIN D IN  25      6816     5.00     5.68
  
```

TOTAL NODE INFLOW...
 HYG file =
 HYG ID = SED BASIN D IN
 HYG Tag = 25

 Peak Discharge = 5.68 cfs
 Time to Peak = 5.00 min
 HYG Volume = 6816 cu.ft

HYDROGRAPH ORDINATES (cfs)
 Output Time increment = 1.00 min
 Time on left represents time for first value in each row.

Time min					
.00	.00	1.14	2.27	3.41	4.54
5.00	5.68	5.68	5.68	5.68	5.68
10.00	5.68	5.68	5.68	5.68	5.68
15.00	5.68	5.68	5.68	5.68	5.68
20.00	5.68	4.54	3.41	2.27	1.14
25.00	.00				

LEVEL POOL ROUTING SUMMARY

HYG Dir = H:\PONDPACK\A12500PLUS\12901\
 Inflow HYG file = NONE STORED - SED BASIN D IN 25
 Outflow HYG file = NONE STORED - SED BASIN D OUT 25

Pond Node Data = SED BASIN D
 Pond Volume Data = SED BASIN D
 Pond Outlet Data = Basin D outflow

No Infiltration

INITIAL CONDITIONS

```

-----
Starting WS Elev = 615.20 ft
Starting Volume = 8538 cu.ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = 1.00 min
  
```

INFLOW/OUTFLOW HYDROGRAPH SUMMARY

```

=====
Peak Inflow = 5.68 cfs at 5.00 min
Peak Outflow = .00 cfs at 1.00 min
-----
Peak Elevation = 616.95 ft
Peak Storage = 15354 cu.ft
=====
  
```

MASS BALANCE (cu.ft)

```

-----
+ Initial Vol = 8538
+ HYG Vol IN = 6816
- Infiltration = 0
- HYG Vol OUT = 0
- Retained Vol = 15354
-----
Unrouted Vol = -1 cu.ft (.009% of Inflow Volume)
  
```

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----- B -----

BASIN D INFLOW 25... 1.01

Basin D outflow... 3.01, 3.04

----- S -----

SED BASIN D... 2.01

SED BASIN D IN 25... 4.01, 4.03