

# STORMWATER DETENTION ANALYSIS

Four Seasons Center

95-179

18 October 1995

1. Gross Acreage = 7.28 A<sup>c</sup>

2. Estimate of Pre-developed Rate of Run-off

25 Year Frequency, 20 Minute Duration Rational Method Storm

$$Q = C I A$$

$$Q = 2.31 \text{ cfs/A}^c \times 7.28 \text{ A}^c$$

$$Q = 16.82 \text{ cfs}$$

3. Estimate of Post-developed Rate of Run-off

25 Year Frequency, 20 Minute Duration Rational Method Storm

$$Q = C I A$$

$$Q = 4.75 \text{ cfs/A}^c \times 7.28 \text{ A}^c$$

$$Q = 34.58 \text{ cfs}$$

4. Estimate of Increase in Rate of Run-off

25 Year Frequency, 20 Minute Duration Rational Method Storm

$$Q_{\text{increase}} = Q_{\text{post-dev}} - Q_{\text{pre-dev}}$$

$$= 34.58 \text{ cfs} - 16.82 \text{ cfs}$$

$$= 17.76 \text{ cfs}$$

5. Attenuation Req'd. =  $Q_{\text{increase}}$

$$= 17.76 \text{ cfs}$$

3

SCANNED  
JUN 10 2019

6. Inflow Hydrograph - 25 Year Frequency 20 Minute Duration  
Rational Method Storm

$$D.A. \text{ to Basin} \cong 7.46 \text{ A}^2$$

$$\text{Travel Distance pipe} \cong 738 \text{ LF} \quad (\text{FE 13} \rightarrow \text{AI 21})$$

$$\text{Travel Distance overland} \cong 310 \text{ LF} \quad (\text{To AI 21})$$

$$\text{Total Distance} \cong 1,048 \text{ LF}$$

$$T_c \cong 1,048 \text{ LF} \div 417 \text{ ft/sec} \cong 2.5 \text{ minutes}$$

$$\text{use } T_c = 2 \text{ minutes}$$

$$Q_{\text{peak}} = 7.46 \text{ A}^2 \cdot 4.75 \text{ cfs/A}^2 \cong 35.44 \text{ cfs}$$

TIME

(min.)

INFLOW RATE

(cfs)

0

0.00

1

17.72

2

35.44

3 - 19

35.44

20

35.44

21

17.72

22

0.00

# 7. Depth - Storage Volume Calculation

$\text{Elev. out} = 563.0'$

<u>ELEV.</u>	<u>AREA</u> ( $\text{ft}^2$ )	<u>AVG.</u> <u>AREA</u> ( $\text{ft}^2$ )	<u>INC. OF</u> <u>DEPTH</u> ( $\text{ft.}$ )	<u>INC. OF</u> <u>VOLUME</u> ( $\text{ft.}^3$ )	<u>TOTAL VOLUME</u> ( $\text{ft}^3$ )
563.0'	0				0
		1,670	1.0'	1,670	
564.0'	3,340				1,670
		4,110	2.0'	8,220	
566.0'	4,880				9,890
		5,730	2.5'	11,460	
568.0'	6,580				21,350
		7,037.5	1.5'	7,037	
569.0'	7,495				28,387
		7,977.5	1.5'	7,977	
570.0'	8,460				36,364

## 2. Depth - Outflow Calculations

Outflow estimated by orifice equation,

$$Q_{out} = C a \sqrt{2g h}$$

Outflow orifice is a "slot," 12" wide  $\times$  18" tall

$$\therefore a = 12" \times 18" = 216 \text{ in}^2 = 1.5 \text{ ft}^2$$

$$\begin{aligned} \therefore Q_{out} &= (0.6)(1.5) \sqrt{64.4 h} \\ &= 0.9 \sqrt{64.4 h} \end{aligned}$$

<u>ELEV.</u>	<u>h</u>	<u>Q<sub>out</sub> Slot</u> (cfs)	<u>Q<sub>out</sub> Riser</u> (cfs)	<u>TOTAL</u> <u>Q<sub>out</sub></u> (cfs)
563.0	0	0.00	0	0.00
564.0	0.25	<del>3.61</del> 3.00 <sup>+</sup>	0	3.00
566.0	2.25	10.83	0	10.83
568.0	4.25	14.89	0	14.89
569.0	5.25	16.55	0	16.55
570.0	6.25	18.06	33.0	51.06

$$Q_{out \text{ Riser}} = C L H^{3/2}$$

$$\text{where } L = 11.0 \text{ ft}$$

$$C = 3.0$$

$$H = \text{ELEV} - 469.0$$

$$\text{note: } h = \text{ELEV} - 563.75$$

\* Q<sub>out</sub> estimated by weir equation

$$Q_{out} = C L H^{3/2}$$

## 9. RESULTS OF ROUTING CALCULATIONS

25 YEAR, 20 MINUTE RATIONAL METHOD STORM

$$\text{Peak } Q_{in} = 35.44 \text{ cfs}$$

$$\text{Peak } Q_{out} = 16.39 \text{ cfs}$$

$$\text{ATTENUATION} = 35.44 - 16.39 = 19.05 \text{ cfs}$$

$$\text{HIGH WATER ELEV.} = 568.90$$

(See Exhibit "A" Attached)

## 10. CHECK STORM OF GREAT INTENSITY

Check 100 year frequency, 20 minute duration  
Rational Method Storm

$$Q_{100} \approx 7.46 \text{ A}^2 @ 6.08 \text{ cfs/A}^2$$

$$\approx 45.36 \text{ cfs}$$

Results of Routing Calculations

$$Q_{out} \approx 37 \text{ cfs}$$

$$\text{High Water Elevation} \approx 469.60$$

(See Exhibit "C" Attached)

11. Check 15 Year frequency 20 Minute Duration  
Rational Method Storm

$$Q_{in} = 28.71 \text{ cfs}$$

$$Q_{out} = 14.89 \text{ cfs}$$

$$\text{High Water Elevation} \approx 467.99$$

(See Exhibit "E" Attached)