

PICKETT RAY & SILVER

333 MID RIVERS MALL DRIVE
ST. PETERS, MISSOURI 63376
314/397-1211 FAX 314/397-1104

LETTER OF TRANSMITTAL

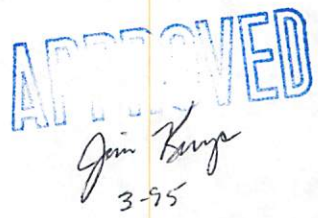
DATE 01/24/95	JOB NO. 89-204
ATTENTION MR. JIM KUNZA	
RE TURTLE CREEK (GLEN EAGLE)	

TO CITY OF O'FALLON
138 SOUTH MAIN
O'FALLON, MISSOURI 63366

GENTLEMEN:

WE ARE SENDING YOU

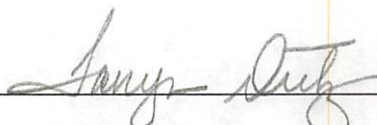
- Attached Prints Plans Specifications Copy of letter
 DELIVERED

COPIES	DATE	NO.	DESCRIPTION
1			"LAKE 13" DETENTION CALCULATIONS 

THESE ARE TRANSMITTED as checked below:

- For your use For approval For review and comment
 As requested Revised as requested FOR BIDS DUE _____ 19____

REMARKS

COPY TO ED SCHULTZ W/COPY SIGNED: 
TANYA DIETZ

1/24/95

Turtle Creek 89204

1.24.95

Lake 13 - Calculations
(Information by others)
(Provided by Ed Schultz)

HYDROGRAPH REPORT

RECORD NUMBER : 1
TYPE : MOD. RATIONAL
DESCRIPTION : 25 YEAR 30 MIN INFLOW "13"

[HYDROGRAPH INFORMATION]

Peak Discharge..... = 165.87 (cfs)
Volume..... = 6.86 (acft)
Time Interval..... = 5 (min)
Time to Peak..... = 30.00 (min)
Time of Base..... = 60.00 (min)
Multiplication factor..... = 1.00

[RATIONAL HYDROGRAPH INFORMATION]

Runoff Coefficient..... = 0.00000 (cfs)
Receding limb factor..... = 1.00000

[BASIN DESCRIPTION]

Watershed Area..... = 54.86 (ac)
Curve Number..... = 0
Runoff coefficient..... = 0.75

[TIME CONCENTRATION -- USER DEFINED]

Time of Concentration..... = 30.00 (min)

[RAINFALL DESCRIPTION]

Distribution Type..... = SYNTHETIC
Total Precipitation..... = 2.02 (in)
Return Period..... = 25 (yr)
Storm Duration..... = 0.50 (hr)

HYDROGRAPH REPORT

RECORD NUMBER : 2
 TYPE : RESER STOR. IND
 DESCRIPTION : 25 YEAR 30 MIN OUTFLOW

[HYDROGRAPH INFORMATION]

Peak Discharge.....	=	16.75 (cfs)
Volume.....	=	6.28 (acft)
Time Interval.....	=	5 (min)
Time to Peak.....	=	55.00 (min)
Time of Base.....	=	1240.00 (min)
Peak Elevation.....	=	583.88 (ft)

[RESERVOIR STRUCTURE INFORMATION]

Reservoir #..... = 1
 Description..... = LAKE "13"
 Storage type..... = MAN STAGE/AREA
 Max storage..... = 272626.00 sqft
 Discharge type..... = COMP STAGE/DISC
 Max discharge..... = 17.02 cfs

[RESERVOIR INFORMATION]

Reservoir #..... = 1
 Reservoir Description..... = LAKE "13"

[INFLOW HYDROGRAPH INFORMATION]

Hydrograph #..... = 1
 Hydrograph Description..... = 25 YEAR 30 MIN INFLOW "13"

RESERVOIR REPORT

RECORD NUMBER : 1
STORAGE TYPE : MAN STAGE/AREA
DISCHARGE TYPE : COMP STAGE/DISC
DESCRIPTION : LAKE "13"

[RATING CURVE LIMIT]

Minimum Elevation..... = 578.00 (ft)
Maximum Elevation..... = 584.00 (ft)
Elevation Increment..... = 1.00 (ft)

[STAGE STORAGE INFORMATION]

Input file = NULL
Output file = NULL

[Manual Contour Area vs. Elevation]

ELEVATION (ft)	CONTOUR AREA (sqft)
579.90	0.01
580.00	50568.80
582.00	60760.10
584.00	75252.60

[STAGE DISCHARGE INFORMATION]

OUTLET STRUCTURE:
STR # : 1
TYPE : STAND PIPE WEIR
DESCRIPTION : LAKE "13"

OUTLET STRUCTURE REPORT

RECORD NUMBER : 1
TYPE : STAND PIPE WEIR
DESCRIPTION : LAKE "13"

[RATING CURVE LIMIT]

Minimum Elevation..... = 578.00 (ft)
Maximum Elevation..... = 584.00 (ft)
Elevation Increment..... = 0.50 (ft)

[STANDPIPE INFORMATION]

DESCRIPTION : 18" STANDPIPE LAKE "13"

[OUTLET STRUCTURE INFORMATION]

Radius..... = 0.75 (ft)
Crest Length..... = 4.71 (ft)
Crest Elevation..... = 580.00 (ft)
Fraction Open Area..... = 1.00000

[RECTANGULAR STAND PIPE EQUATION]

ORIFICE EQ: $Q = C_o \times A (2gh)^{0.5}$
WEIR EQ: $Q = C_w \times L \times H^{\text{exp}}$
Coefficient C_o = 0.60000
Coefficient C_w = 3.00000
Exponential..... = 1.50000

[DEFINITIONS]

H = Headwater depth above inlet control section invert, (ft)
A = Wetted area, (sqft)
L = Crest length, (ft)

[CULVERT INFORMATION]

DESCRIPTION : LAKE "13"

[OUTLET STRUCTURE INFORMATION]

Circular Radius..... = 1.00 (ft)
Culvert Invert Elevation..... = 578.00 (ft)
Slope..... = 0.01000
Manning's N-value..... = 0.01300
Orifice Coefficient..... = 0.60000
Tailwater..... = 577.10 (ft)
Number barrels..... = 1

OUTLET STRUCTURE REPORT

RECORD NUMBER : 1
 TYPE : STAND PIPE WEIR
 DESCRIPTION : LAKE "13"

[UNSUBMERGED EQUATION]

$$H/Diam = H_c/Diam + K \times (Q/A \times Diam^{0.5})^2 - 0.5 \times S^2$$

Coefficient K..... = 0.04500

coefficient H..... = 2.00000

[SUBMERGED EQUATION]

$$H/Diam = c \times (Q/(A \times Diam^{0.5}))^2 + Y - 0.5 \times S^2$$

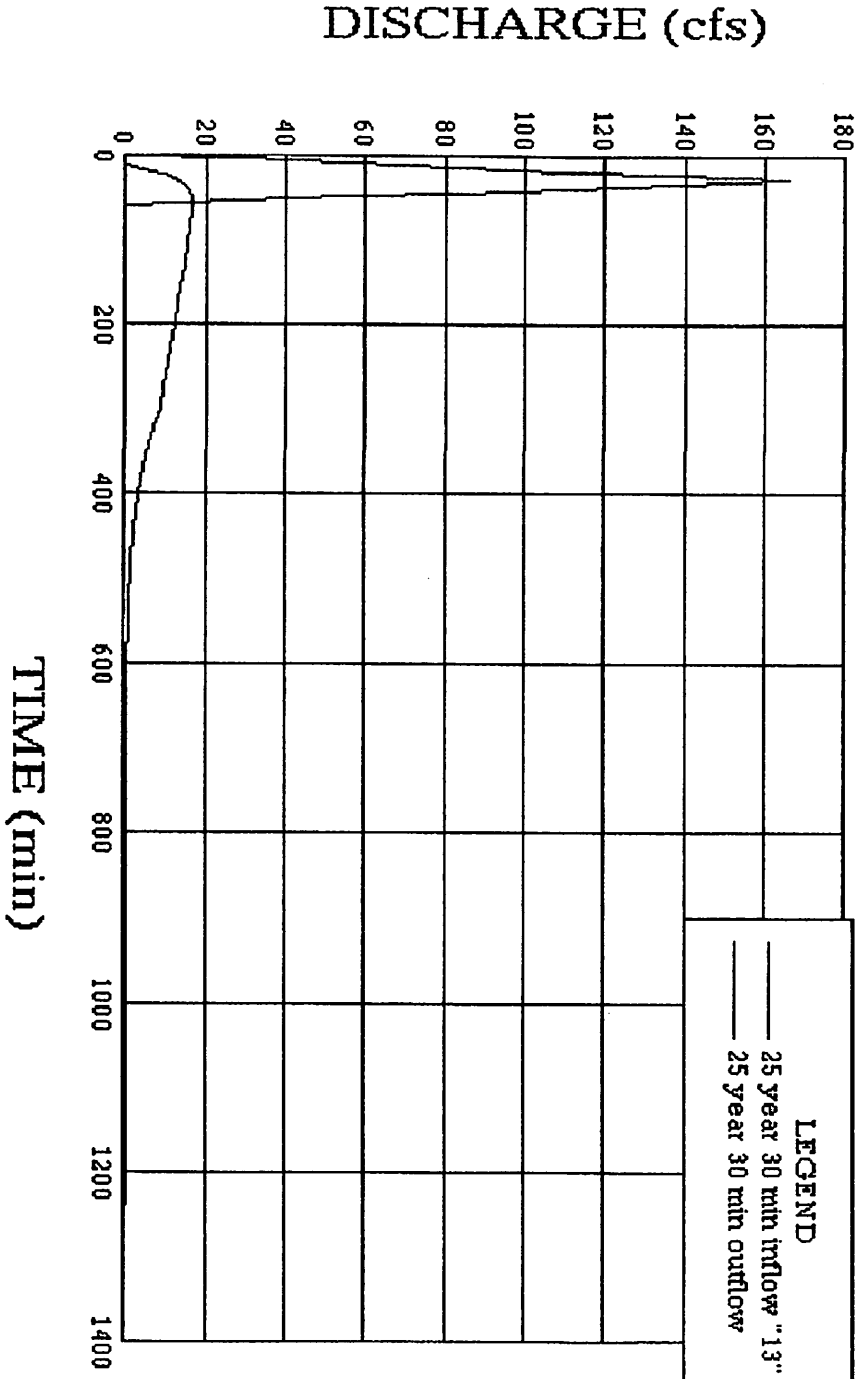
Coefficient c..... = 0.03170

Coefficient Y..... = 0.69000

[DEFINITIONS]

H = Headwater depth above inlet control section invert, (ft)
 Diam = Interior height of culvert barrel, (ft)
 Hc = Specific head at critical depth ($d_c + V_c^2/2g$), (ft)
 Q = Discharge, (cuft/s)
 A = Full cross sectional area of culvert barrel, (sqft)
 S = Culvert barrel slope, (ft/ft)

HYDROGRAPH



FILE

RECEIVED
NOV 18 1994
ENGINEERING DEPT

GEORGE BUTLER ASSOCIATES, INC
Engineers • Architects
Suite 200, 225 South Main Street
O'Fallon, Missouri 63366-2892
Telephone (314) 240-2444
FAX (314) 978-2901

November 16, 1994

Mr. Benny Hedden, P.E.
City Engineer
City of O'Fallon
138 South Main Street
O'Fallon, Missouri 63366

Re: Plan Review
Glen Eagle Subdivision: Detention Calculations (11/15/91)
Plans Revised (7/16/92)

Dear Benny:

Please find the enclosed review comments for the above referenced project as requested.

If you have any questions, please call me.

Very truly yours,

*FAXED TO TONYA
12-12-94*

GEORGE BUTLER ASSOCIATES, INC.

J.J. Choinka
John J. Choinka, P.E.

JJC:ml

Enclosure

cc: Jerry Hurlbert



Other Offices: Oklahoma City, Oklahoma
Kansas City, Missouri
Springfield, Missouri
Lenexa, Kansas

25

PLAN REVIEW
CITY OF O'FALLON, MISSOURI

GLEN EAGLE SUBDIVISION
DETENTION CALCULATIONS

LAKE 5

1. Off-site drainage area map of the construction plans does not match the off-site drainage areas shown in the detention report.
2. The proposed 66" standpipe will act as a wier under low head; however, at heads of 2'-3' and greater the standpipe will act as an orifice.
3. The "C" values used in the wier equation should be close to 2.65 (depending on the head provided.)
4. 15 year storm routing has not been provided.
5. Sediment storage calculations have not been provided.
6. The detention report calls for a 66" diameter standpipe; however, the plans indicate a 7' x 7' concrete structure is to be used.
7. Provide buoyancy calculations for the outfall structure.
8. Provide calculations indicating that headwater inside the outfall structure has been checked or accounted for.

LAKE 8

1. Sediment storage calculations have not been provided.
2. 15 year storm routing has not been provided.
3. Provide a concrete spillway in lieu of a grass swale.
4. Calculations are for a 52' wide spillway; however, revisions indicate a 46.15' wide spillway is proposed.
5. A "C" coefficient of approximately 2.6 should be used for this length of wier (depending on the water depth).
6. Extend spillway on plans to three feet below the water elevation as shown on the detail.

7. Does not appear the calculations account for the 3:1 side slopes on the wier.

LAKE 13

1. Off-site drainage area map of the construction plans does not match the off-site drainage areas shown in the detention report.
2. Sediment storage calculations have not been provided.
3. 15 year storm routing has not been provided.

LAKE 14

1. A 10' wide spillway is shown on the plans and used in the calculations; however, the revisions call for a 8.26' wide spillway to be constructed.
2. The calculations do not appear to account for the 3:1 side slopes shown on the proposed wier.
3. Sediment storage calculations have not been provided.
4. 15 year storm routing has not been provided.

LAKE 17

1. Basin calculations/locations will not conform to current subdivision design.
2. Off-site drainage area map of the construction plans does not match the off-site drainage areas shown in the detention report.
3. Sediment storage calculations have not been provided.
4. 15 year storm routing has not been provided.

PICKETT, RAY & SILVER, INC.
 333 MID RIVERS MAIN DRIVE
 ST. PETERS, MISSOURI 63376

LETTER OF TRANSMITTAL

314/441-1211

314/278-1211

DATE 11/22/91	JOB NO. 89-204
ATTENTION Mr. Frank Godwin	
RE Glen Eagle Golf Course	

TO City of O'Fallon
 138 South Main Street
 O'Fallon, Missouri 63366

RECEIVED
 NOV 25 1991
 BUILDING DEPT.

GENTLEMEN:

WE ARE SENDING YOU Attached Prints Plans Specifications Copy of letter

COPIES	DATE	NO.	DESCRIPTION
1		6	Lake Design Plans
1			Lake Design Calculations
1		2	Hydraulic Data
1		5	Culvert Design @ Sta. 20+11.50
1		2	Lake #5 O.S. Information

THESE ARE TRANSMITTED as checked below:

- For your use For approval For review and comment
 As requested Revised as requested FOR BIDS DUE _____ 19____

REMARKS

Howard Chilcutt/The Jones Co.
 John Allen/Vatterott

COPY TO _____

SIGNED: Tanya Dietz
 Tanya Dietz

PICKETT RAY & SILVER

333 MID RIVERS MALL DRIVE
ST. PETERS, MO. 63376
(314) 441-1211
(314) 278-1211

Civil Engineers
Planners
Land Surveyors

PROJECT	JOB NO.
GLEN EAGLE	89-204

DATE 06/02/94	PAGE 2 OF 2
---------------	-------------

PREPARED BY T. Dietz	MEETING NO.
----------------------	-------------

Communication:

MEETING	TELEPHONE	MEMO	X
---------	-----------	------	---

ENCLOSURES	
------------	--

Note: Recipients are requested to review all items and inform PR&S of any discrepancies.

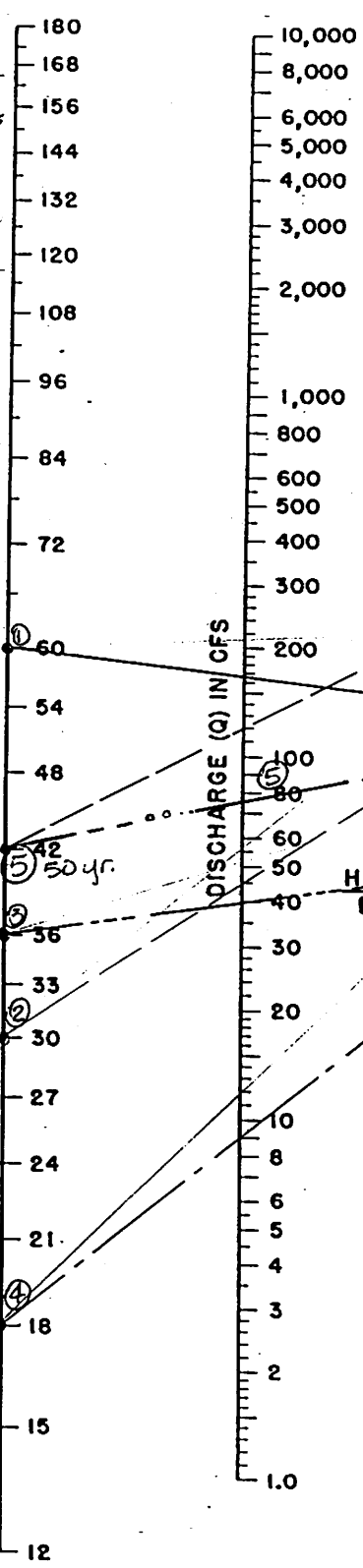
	Organization	Individuals and Copies		
In Attendance	City of O'Fallon	Frank Godwin		
	Fm. Tanya Dietz			
Additional Distribution				

Item No.	Topic / Description	Action By
4	FE S-67	
	HW 1.83	
	HW ₅₀ 2.39	
	<u>£ 592.00</u>	
	593.83	
	594.39	
	Top Berm = 598.0 vs 594.39 ok	
<i>Tdf</i>	<i>Rev. 8-2-94 Tdf</i>	
	<i>FE S-75</i>	
	<i>HW 50 4.41</i>	
	<i>£ 588.00</i>	
	<i>592.41</i>	
	<i>Top Berm = 594.5 vs 592.41</i>	

CHART 1

- ① FE 5-8
106.89 cfs
60" RCP
H = 5.8'
 - ② FE 5-18a
45.30 cfs
30" RCP
H = 4.3'
 - ③ FE 5-33
40.71 cfs
36" RCP
H = 3.12
 - ④ FE 5-47
9.40 cfs
18" RCP
H = 1.83'
 - ⑤ FE 5-75
59.19 cfs
42" RCP
(75.76 cfs = 50 yr)
H = 4.41'
- # 588 + 4.41 = 592.41
 Top Perm = 594.5

50 yr. 213.6 cfs
 H = 7.3'
 50 yr. 57.98 cfs
 H = 5.9'
 50 yr. 52.11 cfs
 H = 3.78'
 50 yr. 12.03 cfs
 H = 2.39'
 50 yr. 75.76 cfs
 H = 4.41'

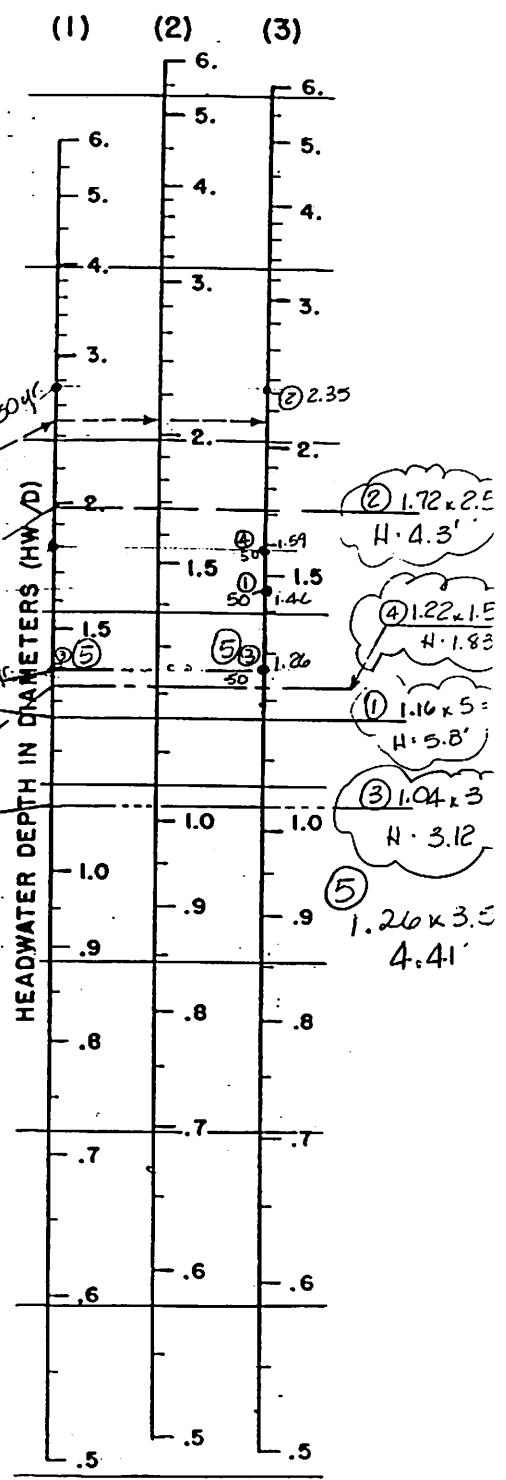


EXAMPLE

D = 42 inches (3.5 feet)
Q = 120 cfs

	HW/D	HW feet
(1)	2.5	8.8
(2)	2.1	7.4
(3)	2.2	7.7

*D in feet



To use scale (2) or (3) project horizontally to scale (1), then use straight inclined line through D and Q scales, or reverse as illustrated.

HEADWATER DEPTH FOR CONCRETE PIPE CULVERTS WITH INLET CONTROL

PICKETT RAY & SILVER

333 MID RIVERS MALL DRIVE
 ST. PETERS, MISSOURI 63376
 314/441-1211 314/278-1211
 FAX 314/278-1104

LETTER OF TRANSMITTAL

DATE 08/02/94	JOB NO. 89-204
ATTENTION MR. FRANK GODWIN	
RE GLEN EAGLE - SOUTH	

TO
 CITY OF O'FALLON
 138 SOUTH MAIN
 O'FALLON, MISSOURI 63366

GENTLEMEN:

WE ARE SENDING YOU

Attached
 Prints
 Plans
 Specifications
 Copy of letter

COPIES	DATE	NO.	DESCRIPTION
1		37	IMPROVEMENT PLANS
1		4	TRANSMITTAL 6/2/94 INLET CONTROL CALCULATIONS CHART
1		1	CHART 1 (78" PIPE)
1		1	OVERALL DAM
1		4	HYDRAULIC CALCULATIONS

THESE ARE TRANSMITTED as checked below:

For your use
 For approval
 For review and comment
 As requested
 Revised as requested
 FOR BIDS DUE _____ 19_____

REMARKS

PER CITY COMMENTS

1. FIRE DISTRICT APPROVAL - FORTHCOMING
2. SANITARY SEWER - APPROVED
3. STREET CURVE DATA - SEE RECORD PLAT
4. STORM SEWERS - REVISED -

4K - Per St Charles Co Hwy (they wrote program) there is no negative loss - the program will use 0 loss verse a negative

MCKELVEY HOMES
 MAYER HOMES

COPY TO _____
 GLEN EAGLE ASSOCIATION

SIGNED: *James Duff*

PICKETT HAY & SILVER

333 MID RIVERS MALL DRIVE
 ST. PETERS, MISSOURI 63376
 314/441-1211 FAX 314/278-1104

TO CITY OF O'FALLON
 138 SOUTH MAIN
 O'FALLON, MISSOURI 63366

DATE: 08/02/94
 JOB NO: 89-204
 ATTENTION: MR. FRANK GODWIN
 RE: GLEN EAGLE - SOUTH

WE ARE SENDING YOU: Attached Prints Plans Specifications Copy of letter

NO.	DESCRIPTION	QUANTITY	UNIT
1	HYDRAULIC CALCULATIONS	1	
1	OVERALL DAM	1	
1	CHART 1 (78" PIPE)	1	
1	TRANSMITTAL 6/2/94 INLET CONTROL CALCULATIONS CHART	4	
1	IMPROVEMENT PLANS	37	

REMARKS: THESE ARE TRANSMITTED AS REQUESTED. For your use For approval Revised as requested

DATE: 08/02/94
 FOR REVIEW AND COMMENT:
 FOR RIDS DUE:

- PER CITY COMMENTS
1. FIRE DISTRICT APPROVAL - FORTHCOMING
 2. SANITARY SEWER - APPROVED
 3. STREET CURVE DATA - SEE RECORD PLAT
 4. STORM SEWERS - REVISED

Copy of letter Specifications Plans

COPY TO: GLEN EAGLE ASSOCIATION
 MAYER HOMES
 MCKELVEY HOMES
 Signed: *Frank Godwin*
 Note: program that is to be used - the program will use 0.05 loss vs 0.10

PICKETT RAY & SILVER

LETTER OF TRANSMITTAL

333 MID RIVERS MALL DRIVE
ST. PETERS, MISSOURI 63376
314/441-1211 314/278-1211
FAX 314/278-1104

DATE	06/02/94	JOB NO.	89-204
ATTENTION	FRANK		
RE	GLEN EAGLE		

TO

CITY OF O'FALLON
138 SOUTH MAIN
O'FALLON, MISSOURI 63366

GENTLEMEN:

WE ARE SENDING YOU

- Attached Prints Plans Specifications Copy of letter
 MAIL

COPIES	DATE	NO.	DESCRIPTION
1		3	CALCULATIONS FOR "INLET CONTROL"

THESE ARE TRANSMITTED as checked below:

- For your use For approval For review and comment
 As requested Revised as requested FOR BIDS DUE _____ 19____

REMARKS

(PREVIOUSLY FAXED)

COPY TO _____

SIGNED: Tanya Dietz

TANYA DIETZ

PICKETT RAY & SILVER

333 MID RIVERS MALL DRIVE
ST. PETERS, MO. 63376
(314) 441-1211
(314) 278-1211

Civil Engineers
Planners
Land Surveyors

PROJECT	JOB NO.
GLEN EAGLE	89-204

DATE 06/02/94	PAGE 2 OF 2
---------------	-------------

PREPARED BY T. Dietz	MEETING NO.
----------------------	-------------

Note: Recipients are requested to review all items and inform PR&S of any discrepancies.

Communication:

MEETING	TELEPHONE	MEMO	X
---------	-----------	------	---

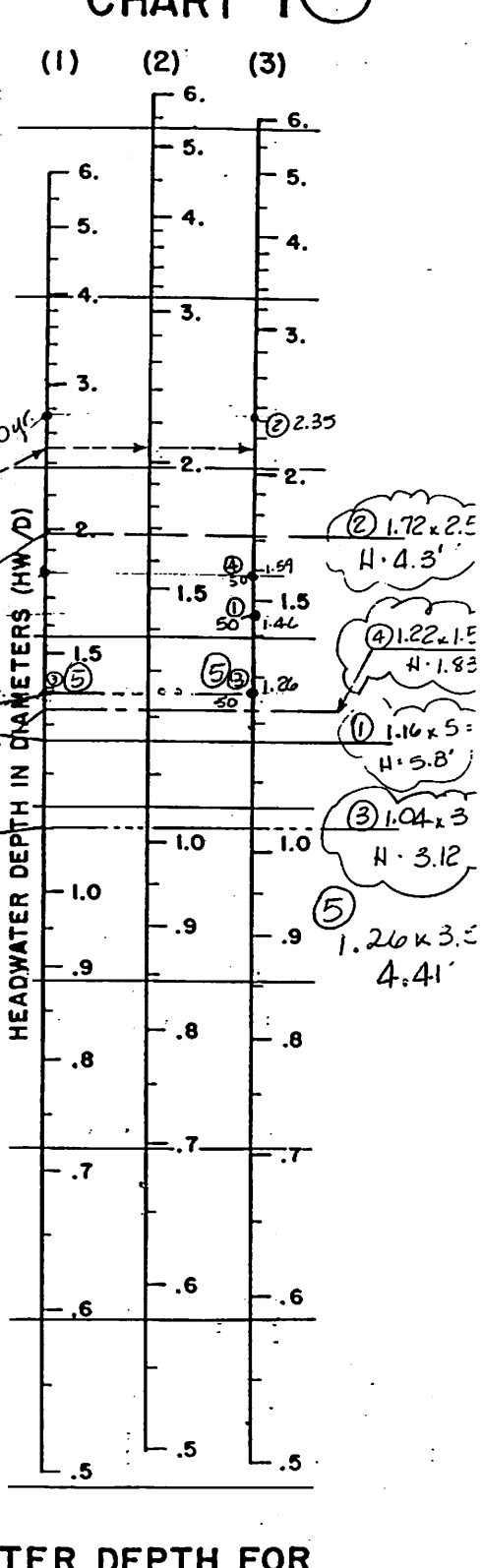
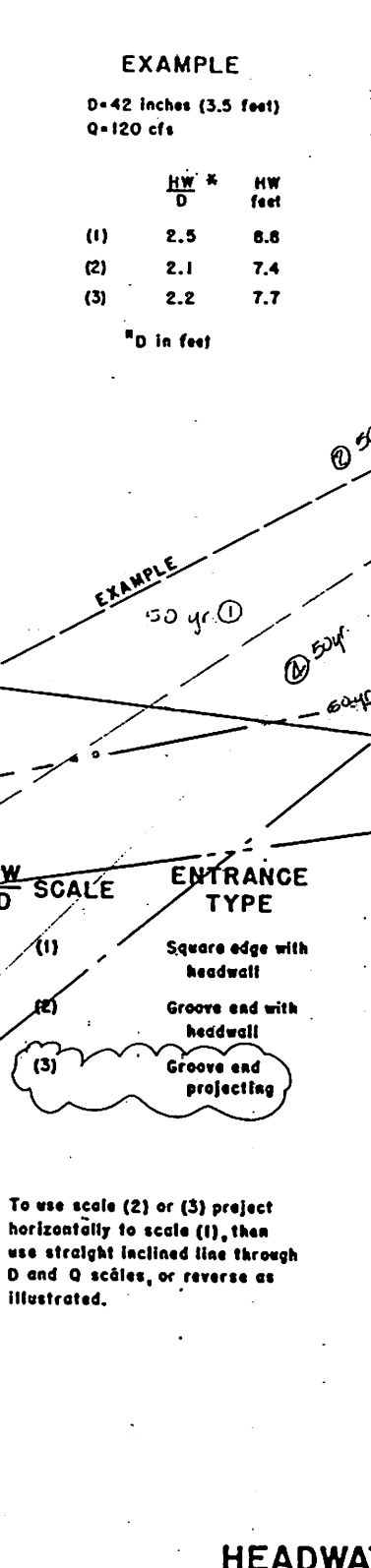
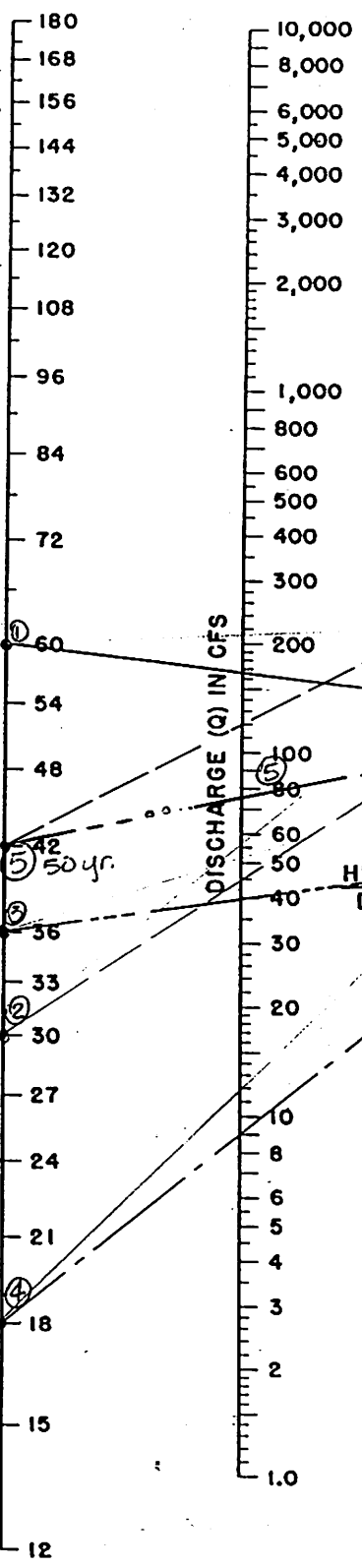
ENCLOSURES	
------------	--

	Organization	Individuals and Copies		
In Attendance	City of O'Fallon	Frank Godwin		
	Fm. Tanya Dietz			
Additional Distribution				

Item No.	Topic / Description	Action By
4	FE S-67	
	HW 1.83	
	HW ₅₀ 2.39	
	<u>£ 592.00</u>	
	£ 592.00	
	593.83	
	594.39	
	Top Berm = 598.0 vs 594.39 ok	
<i>Tdk</i>	<i>Rev. 8-2-94 Tdk</i>	
	<i>FE S-75</i>	
	<i>HW₅₀ 4.41</i>	
	<i>£ 588.00</i>	
	<i>592.41</i>	
	<i>Top Berm = 594.5 vs 592.41</i>	<i>2' OK</i>

CHART 1

- ① FE 5-8
106.89 cfs
60" RCP
H = 5.8'
- ② FE 5-18a
45.30 cfs
30" RCP
H = 4.3'
- ③ FE 5-33
40.71 cfs
36" RCP
H = 3.12
- ④ FE 5-67
9.40 cfs
18" RCP
H = 1.83'
- ⑤ FE 5-75
59.19 cfs
42" RCP
(75.76 cfs = 50 yr)
H = 4.41'



EXAMPLE
D = 42 inches (3.5 feet)
Q = 120 cfs

	$\frac{HW}{D}$	HW feet
(1)	2.5	8.8
(2)	2.1	7.4
(3)	2.2	7.7

"D in feet"

ENTRANCE TYPE

- (1) Square edge with headwall
- (2) Groove end with headwall
- (3) Groove end projecting

To use scale (2) or (3) project horizontally to scale (1), then use straight inclined line through D and Q scales, or reverse as illustrated.

② 1.72 x 2.5 = 4.3'

④ 1.22 x 1.5 = 1.83'

① 1.16 x 5 = 5.8'

③ 1.04 x 3 = 3.12'

⑤ 1.26 x 3.5 = 4.41'

HEADWATER DEPTH FOR CONCRETE PIPE CULVERTS WITH INLET CONTROL

HEADWATER SCALES 2&3
BUREAU OF PUBLIC ROADS JAN. 1963
REVISED MAY 1964

GLEN EAGLE
89204
Rev. 8/2/94

CHART 1

Low in Road =
577.0

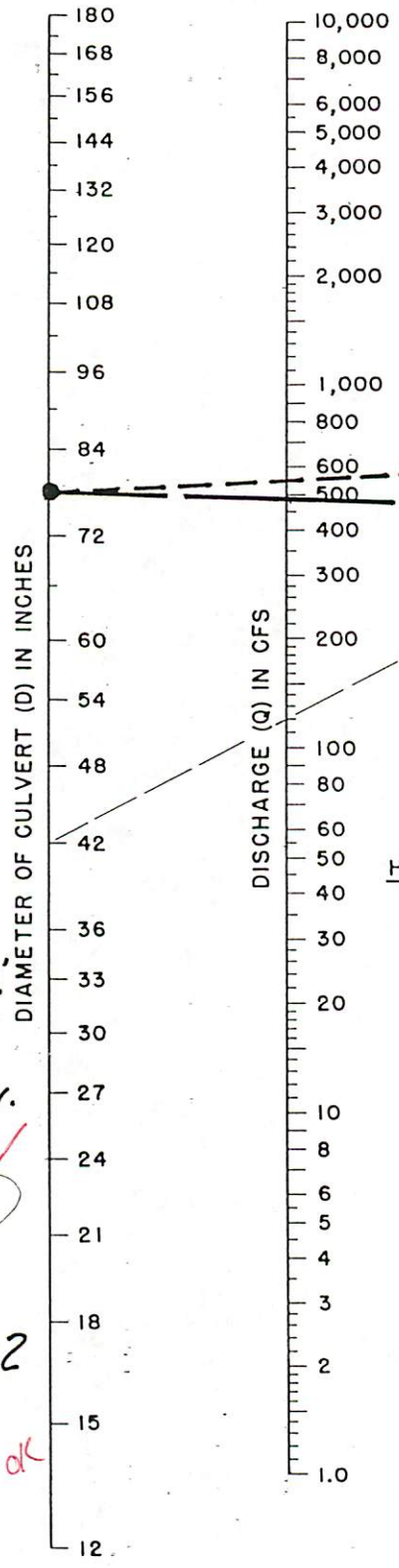
78" RCP
@ 0.9%
160'
563.22
to
561.78

50 yr. :
6.5 x 1.73 = 11.24'
563.22 + 11.24 =
574.46 ELEV.

VS
577.4 ✓
FB = 2.9'

100 yr. :
6.5 x 2.05 = 13.32'
563.22 + 13.32 =
576.54

9' FB ok



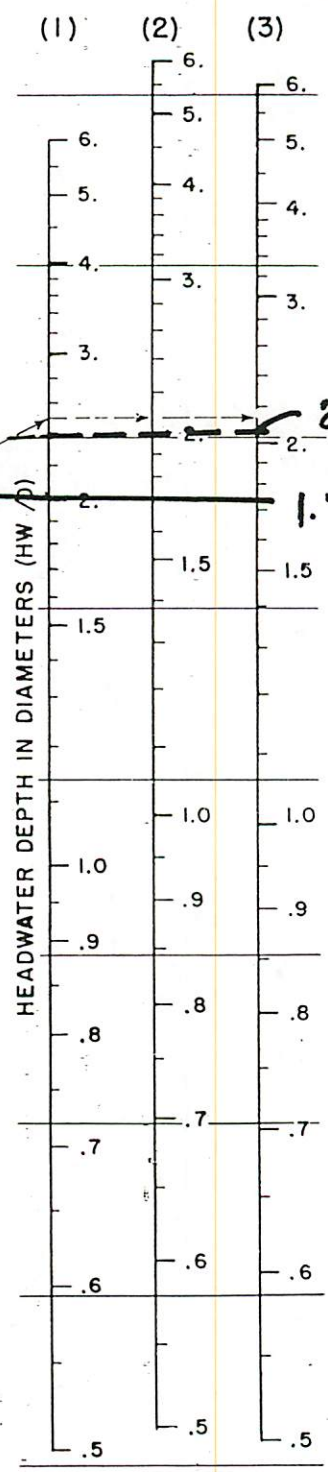
EXAMPLE
D = 42 inches (3.5 feet)
Q = 120 cfs

	HW D	HW feet
(1)	2.5	8.8
(2)	2.1	7.4
(3)	2.2	7.7

*D in feet

HW D	SCALE	ENTRANCE TYPE
(1)		Square edge with headwall
(2)		Groove end with headwall
(3)		Groove end projecting

To use scale (2) or (3) project
horizontally to scale (1), then
use straight inclined line through
D and Q scales, or reverse as
illustrated.



HEADWATER DEPTH FOR CONCRETE PIPE CULVERTS WITH INLET CONTROL

HEADWATER SCALES 283
REVISED MAY 1964

BUREAU OF PUBLIC ROADS JAN. 1963

344.25 cfs

50 yr. : x 1.10 x 1.28 = 484.70 cfs

100 yr. : x 1.15 x 1.39 = 550.28 cfs

(Street Elev 577.4)

89204

PICKETT RAY & SILVER

333 MID RIVERS MALL DRIVE
ST. PETERS, MO. 63376
(314) 441-1211
(314) 278-1211

Civil Engineers
Planners
Land Surveyors

PROJECT	JOB NO.
GLEN EAGLE	89-204

DATE 06/02/94	PAGE 1 OF
---------------	-----------

PREPARED BY T. Dietz	MEETING NO.
----------------------	-------------

Communication:

MEETING	TELEPHONE	MEMO	X
---------	-----------	------	---

ENCLOSURES	
------------	--

Note: Recipients are requested to review all items and inform PR&S of any discrepancies.

In Attendance	Organization	Individuals and Copies		
	Fm Tanya Dietz	City of O'Fallon	Frank Godwin	
Additional Distribution				

Item No.	Topic / Description	Action By
1.	"Headwater Depth for Concrete Pipe Culverts with Inlet Control"	
	1 FE S-8	
	HW 5.8 HW ₅₀ 7.3	
	£ 589.03 £ 589.03	
	594.03 596.33	
	Top Berm = 600.2 vs 596.33 ok ✓	
	2 FE S-18a	
	HW 4.3 HW ₅₀ 5.9	
	£ 603.0 £ 603.0	
	607.3 608.9	
	Top Berm = 609.8 vs 608.9 ok	
	3 FE S-33	
	HW 3.12 HW ₅₀ 3.78	
	£ 604.00 £ 604.00	
	607.12 607.78	
	Top Berm = 609.0 vs 607.78 ok	



CITY OF O'FALLON
State of Missouri

May 6, 1992

Ms. Tanya Dietz
Pickett, Ray & Silver
333 Mid Rivers Mall Dr
St. Peters, Mo. 63376

RE: Glen Eagle Municipal Golf Course - Construction Site Plans

Dear Ms. Dietz

The plans for the project referenced above have been reviewed. Plans are approved as noted. Approval is contingent upon required easements being recorded. Construction authorization is issued on the condition that a copy of the on-site DNR permit is submitted to this office.

Once the State DNR permit has been received, sanitary sewer construction can begin. Please notify the City of O'Fallon at least 48 hours in advance of the start of construction to facilitate inspection scheduling. Also, please contact Jerry Schoenherr to set up a pre-construction meeting as soon as possible. Upon completion of the improvements and necessary tests, an engineer shall certify that construction took place according to plan with all changes noted. Please insure that the as-builts show accurately the sanitary service locations. One (1) set of reproducible as-builts should then be submitted along with three (3) copies. With this information, the City of O'Fallon can proceed to accept the public portion of these improvements.

Thank you for your cooperation in this matter. If you have any additional questions, please contact this office.

Sincerely yours

Frank Godwin

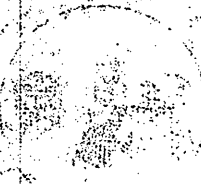
Frank Godwin
Technical Engineer/Inspector

FG/pl

Enc: Plans

cc. P. Nasi
T. Price
J. Hurlbert w/plans

15



UNITED STATES DEPARTMENT OF JUSTICE

FEDERAL BUREAU OF INVESTIGATION

Washington, D. C. 20535

TO : DIRECTOR, FBI (100-442100)
FROM : SAC, NEW YORK (100-100000)
SUBJECT: [Illegible]

[Extremely faint and illegible body text, likely a memorandum or report content.]

Very truly yours,
[Illegible Signature]

SPECIAL AGENT IN CHARGE

100-100000

FBI

12

PICKETT RAY & SILVER

333 MID RIVERS MALL DRIVE
 ST. PETERS, MISSOURI 63376
 314/441-1211 314/278-1211
 FAX 314/278-1104

LETTER OF TRANSMITTAL

DATE	08/06/92	JOB NO.	89-204
ATTENTION	MR. FRANK GODWIN		
RE	GLEN EAGLE		

RECEIVED
AUG 6 - 1992
BUILDING DEPT.

TO CITY OF O'FALLON
 138 SOUTH MAIN STREET
 O'FALLON, MISSOURI 63366

GENTLEMEN:

WE ARE SENDING YOU Attached Prints Plans Specifications Copy of letter

COPIES	DATE	NO.	DESCRIPTION
1		1	SECTION THRU PRIMARY SPILLWAY-LAKE 17
1		1	REVISED "PRINCIPLE SPILLWAY"

THESE ARE TRANSMITTED as checked below:

For your use For approval For review and comment
 As requested Revised as requested FOR BIDS DUE _____ 19____

REMARKS

VATTEROTT

COPY TO THE JONES COMPANY/HOWARD CHILICUTT

SIGNED: *Tanya*

TANYA DIETZ

12

333 MID RIVERS MALL DRIVE
ST. PETERS, MISSOURI 63376
314/447-1511 314/278-1241
FAX 314/278-1104

CITY OF O'FALLON
188 SOUTH MAIN STREET
O'FALLON, MISSOURI 63366

DATE 08/06/92
JOB NO. 89-304
ATTENTION MR. FRANK GODWIN
RE GLEN EAGLE

GENTLEMEN

WE ARE SENDING YOU

Attached
 Prints
 Plans
 Specifications
 Copy of letter

COPIES	DATE	DESCRIPTION
1		SECTION THRU PRIMARY SPILLWAY-LAKE 1V
1		REVISED "PRINCIPLE SPILLWAY"

THESE ARE TRANSMITTED as checked below

For your use
 For approval
 As requested
 Revised as requested
 For review and comment
 FOR BIDS DUE

REMARKS

12

TANYA DIETE
SIGNED
VATTEROTT
COPY TO THE JONES COMPANY/HOWARD CHILCOTT

Lake #17

Revise "Principle Spillway" :

$Q_{25} = 38.19 \text{ cfs}$

$C = 2.68$

$L = 11.0$

$D = 1.2$

$Q_{100} = 48.35$

$C = 2.66$

$L = 11.0$

$D = 1.4$

$Q = CLH^{3/2}$
 $= 2.68(11.0)(1.2)^{3/2}$

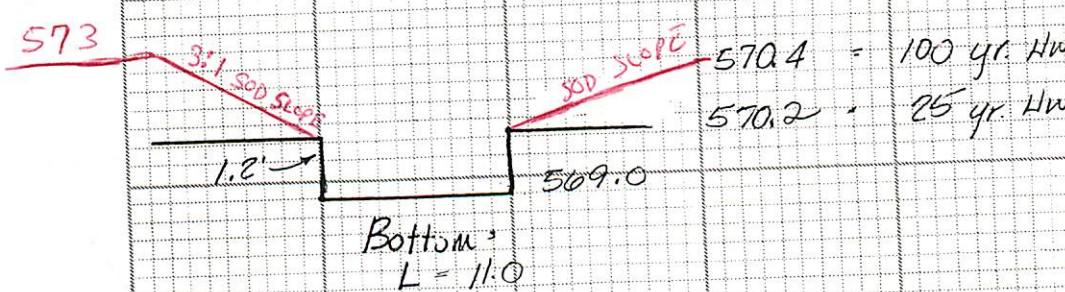
$Q = 38.75 \text{ cfs}$

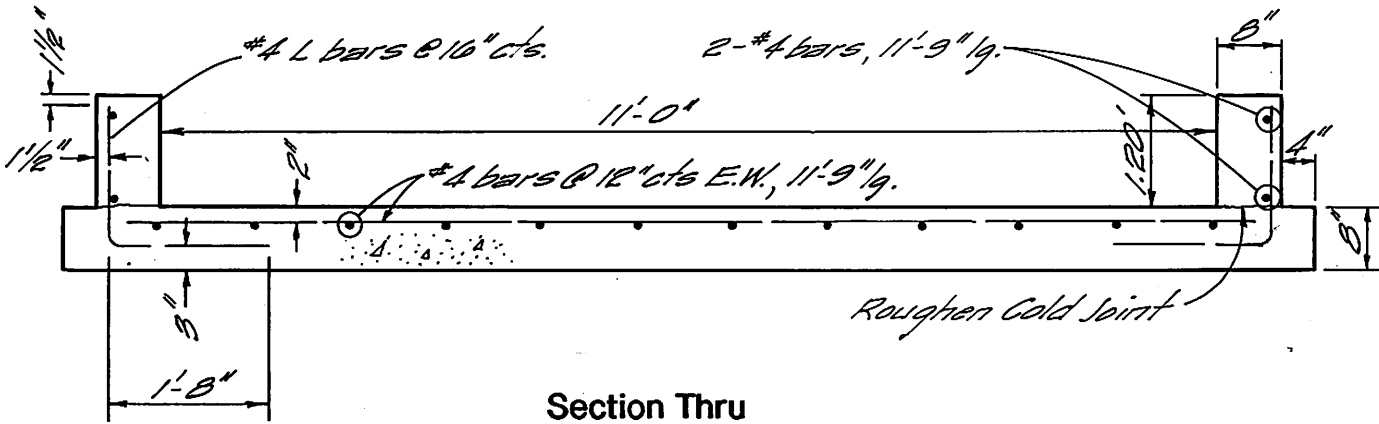
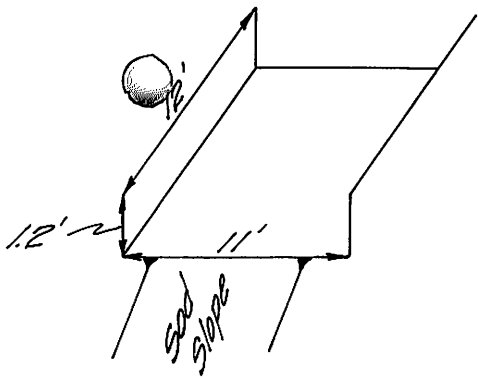
$Q = CLH^{3/2}$
 $= 2.66(11)(1.4)^{3/2}$

$Q = 48.47 \text{ cfs}$

$V = \frac{Q}{A} = \frac{38.75}{13.2} = 2.93 \text{ F.P.S.}$
OK

$V =$





Section Thru
Primary Spillway Lake 17

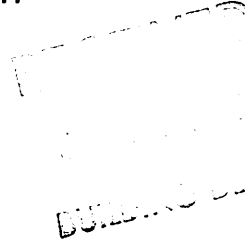
PICKETT, RAY & SILVER, INC.
333 MID RIVERS MAIN DRIVE
ST. PETERS, MISSOURI 63376

LETTER OF TRANSMITTAL

314/441-1211

314/278-1211

TO CITY OF O'FALLON
138 SOUTH MAIN
O'FALLON, MO 63366



DATE 1/24/92	JOB NO. 89-204
ATTENTION MR. FRANK GODWIN	
RE GLEN EAGLE	

GENTLEMEN:

WE ARE SENDING YOU Attached Prints Plans Specifications Copy of letter

COPIES	DATE	NO.	DESCRIPTION
2		2	SHEETS 2 & 8 OF 13
2		2	HYDRAULIC DATA SHEETS


THESE ARE TRANSMITTED as checked below:

For your use For approval For review and comment

As requested Revised as requested FOR BIDS DUE _____ 19 ____

REMARKS

COPY TO VATTEROTT/JOHN ALLEN

SIGNED: 
TANYA J. DIETZ

PROJECT: Glen Eagle
89-204

DESIGNER: J. Dietz
 DATE: 10-15-91

HYDROLOGIC AND CHANNEL INFORMATION

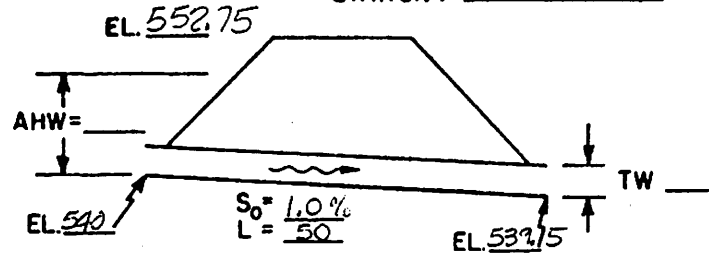
Flood Plain Elev.: $542.5 \pm$
 Flood Way $544.0 \pm$

$Q_1 =$ _____ $TW_1 =$ _____
 $Q_2 =$ 3602₁₀₀ $TW_2 =$ _____

(Q_1 = DESIGN DISCHARGE, SAY Q_{25}
 Q_2 = CHECK DISCHARGE, SAY Q_{50} OR Q_{100})

SKETCH

STATION: 20+11.50



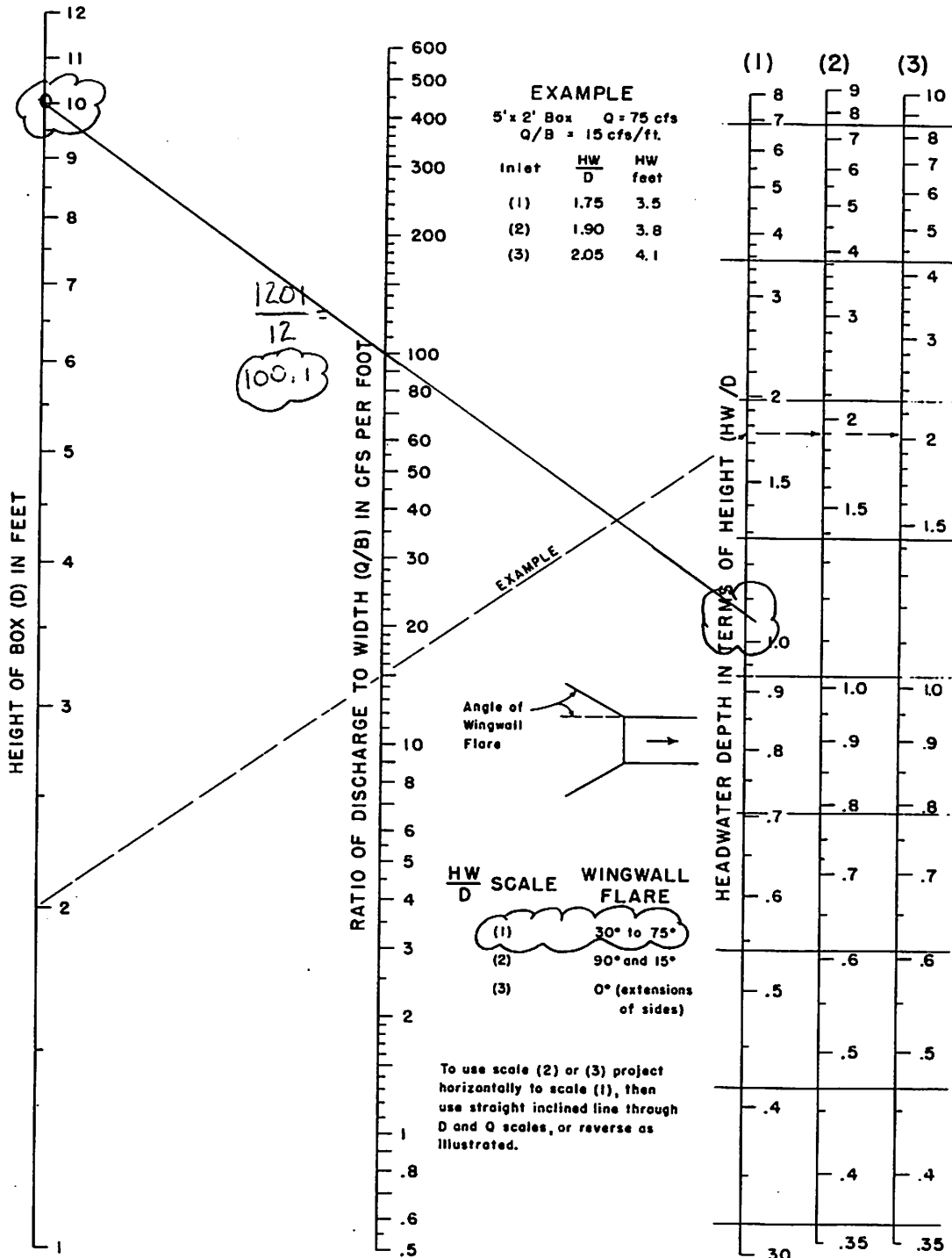
MEAN STREAM VELOCITY = _____
 MAX. STREAM VELOCITY = _____

CULVERT DESCRIPTION (ENTRANCE TYPE)	Q	SIZE	HEADWATER COMPUTATION										CONTROLLING HW	OUTLET VELOCITY	COST	COMMENTS		
			INLET CONT.		OUTLET CONTROL $HW = H + h_0 - LS_0$						TW	h_0					LS_0	HW
			$\frac{HW}{D}$	HW	K_e	H	d_c	$\frac{d_c + D}{2}$	h_0	LS_0								
1-45° Wing Walls Box Culvert	(1300) 1201	3- 12x10	1.05	10.5	0.4	2.4	6.8	8.4	5.0	8.4	.5	10.3	10.5					

SUMMARY & RECOMMENDATIONS:

$\frac{6.8 + 10}{2} = 8.4$ use > of $\frac{d_c + D}{2} + TW$

CHART 8

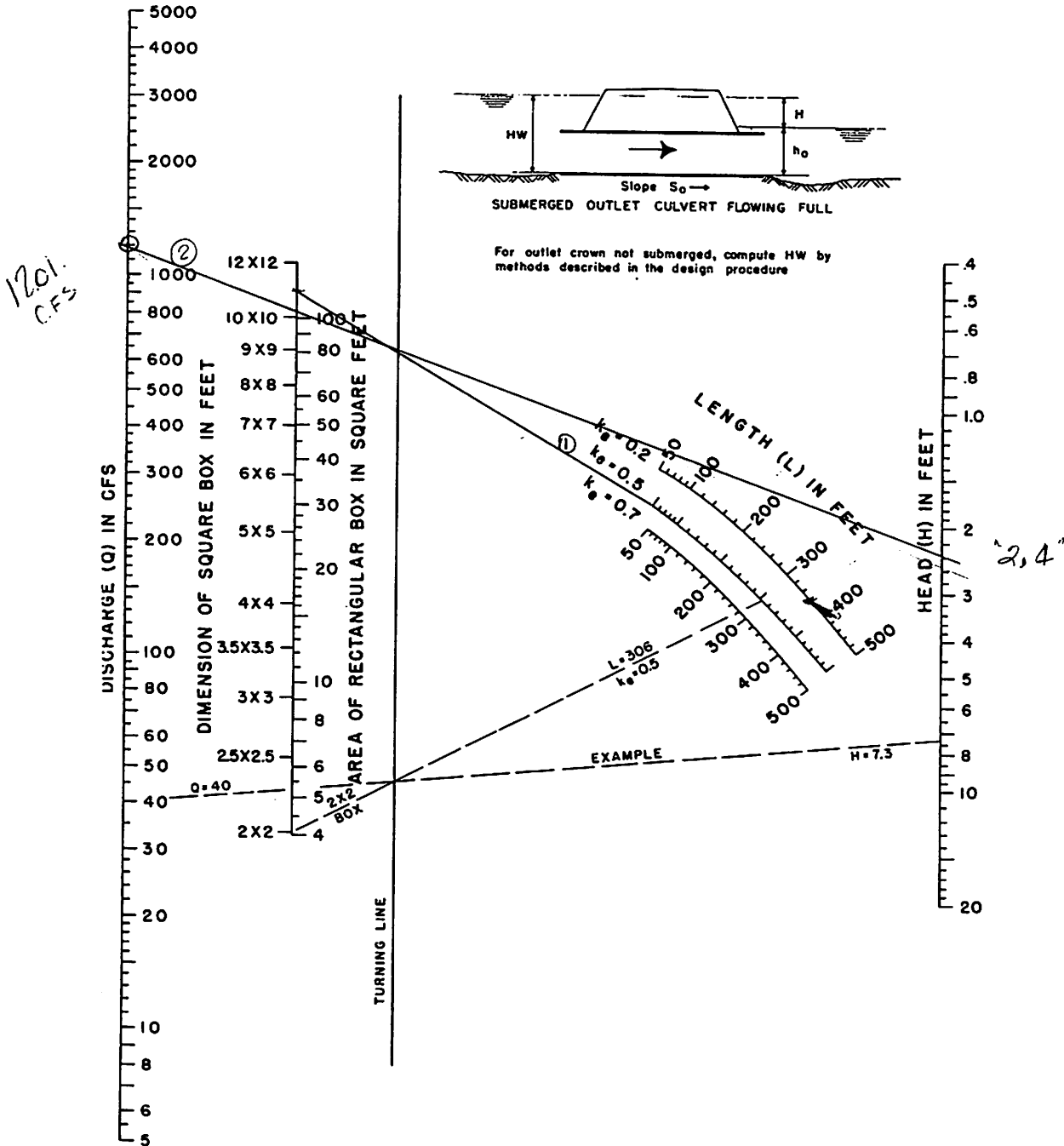


HEADWATER DEPTH FOR BOX CULVERTS WITH INLET CONTROL

BUREAU OF PUBLIC ROADS JAN. 1963

* 3 - 12' W x 10' H Box.
 $Q_{in} = 3600 \div 3 = 1201$ cfs/Box

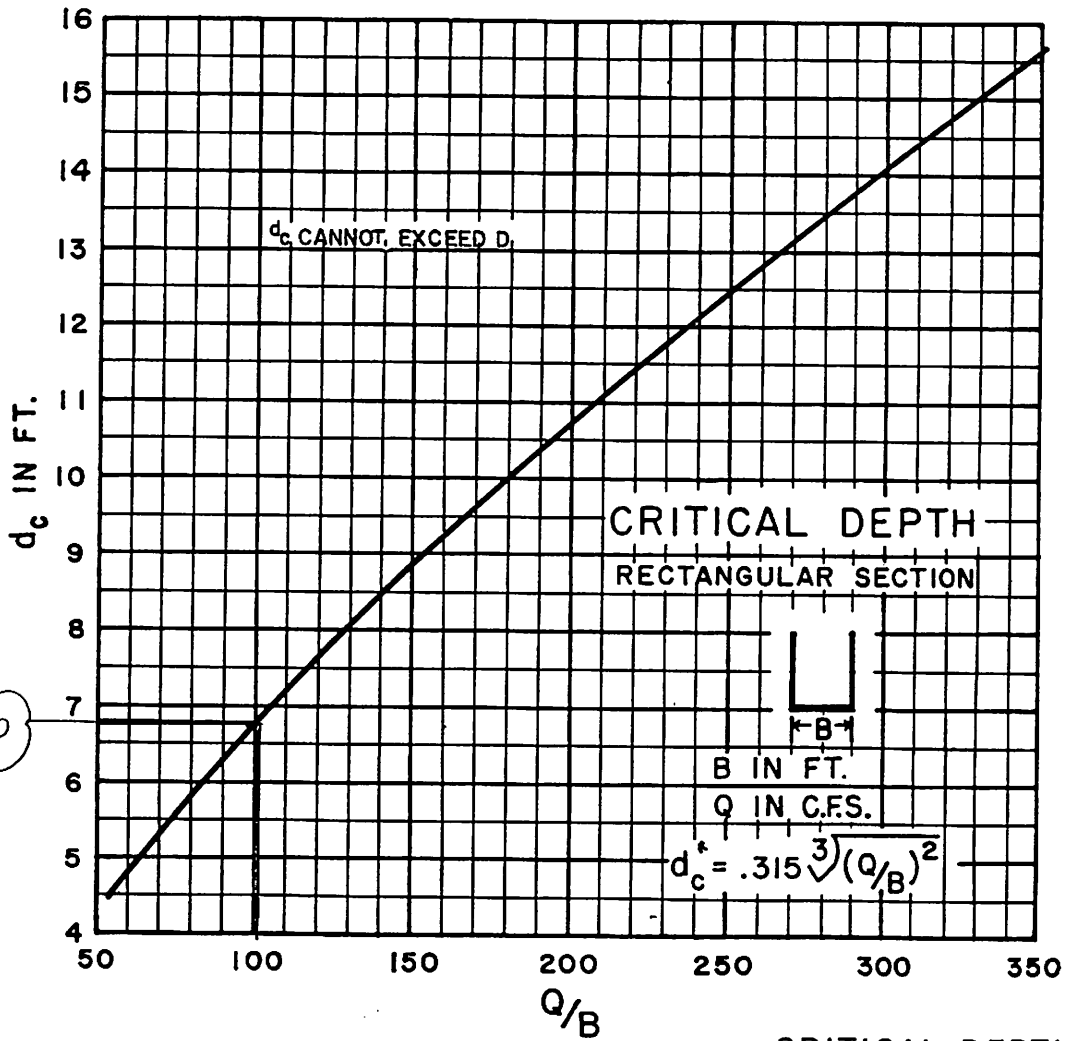
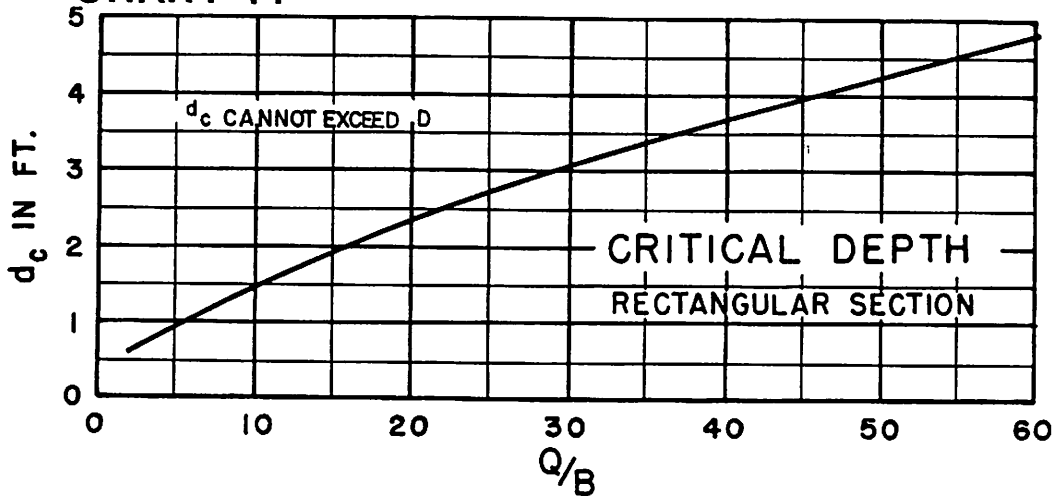
CHART 15



**HEAD FOR
CONCRETE BOX CULVERTS
FLOWING FULL
 $n = 0.012$**



CHART 14

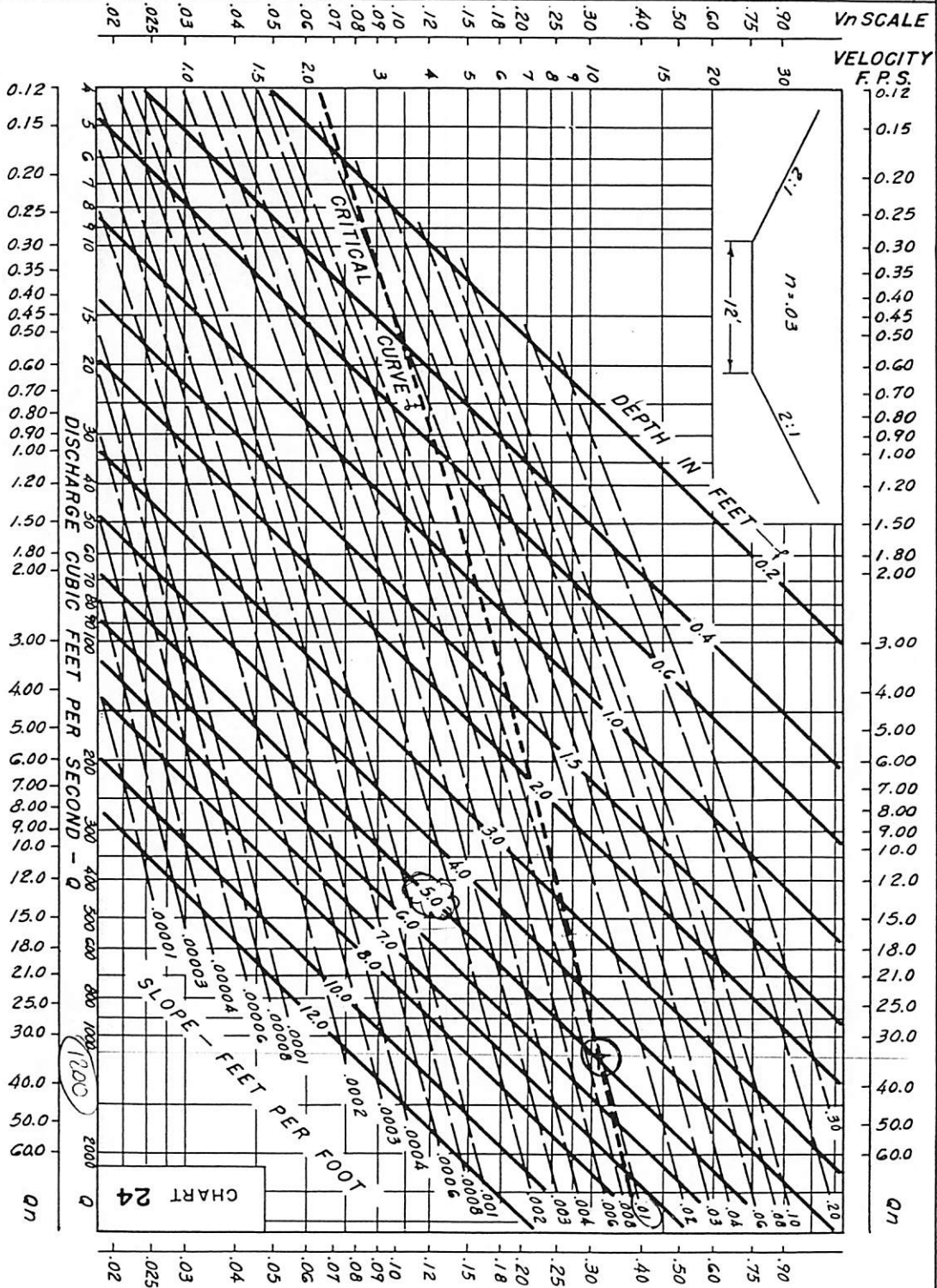


BUREAU OF PUBLIC ROADS JAN. 1963

5-38

CRITICAL DEPTH
RECTANGULAR SECTION

CHANNEL CHART
2:1 b = 12 FT.



PICKETT RAY & SILVER



333 Mid Rivers Mall Dr
St Peters, MO 63376

Civil Engineers
Planners
Land Surveyors

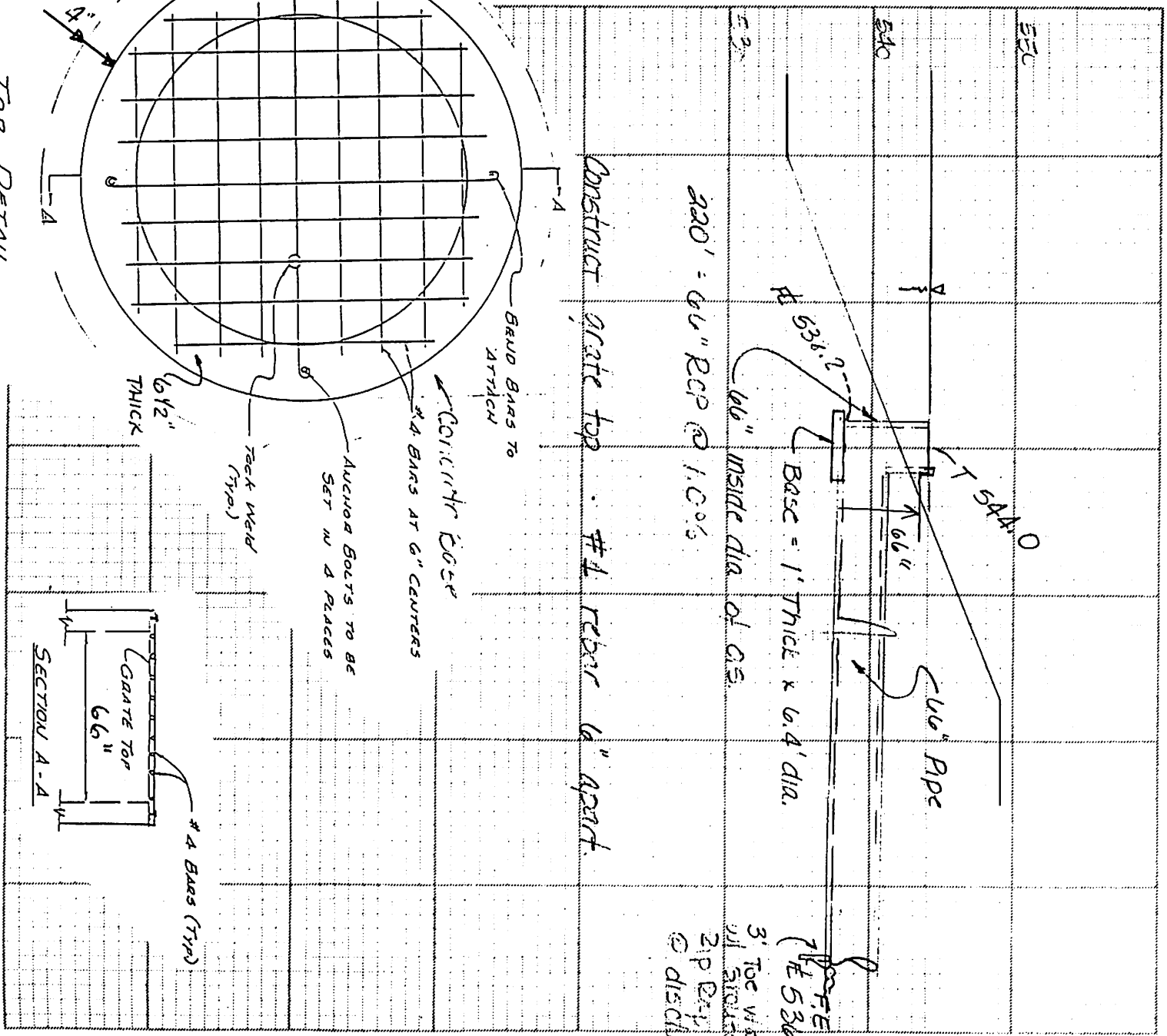
441.1211
278.1211

PROJECT NAME #5 OUTFALL STRUCTURE
PROJECT #/JOB ORDER # 29204

DATE 11-15-01

DESIGNER T. Deitz

PAGE 1 of 2



TOP DETAIL
OVERFLOW STRUCTURE

PROJECT: _____

DESIGNER: I. Dieb

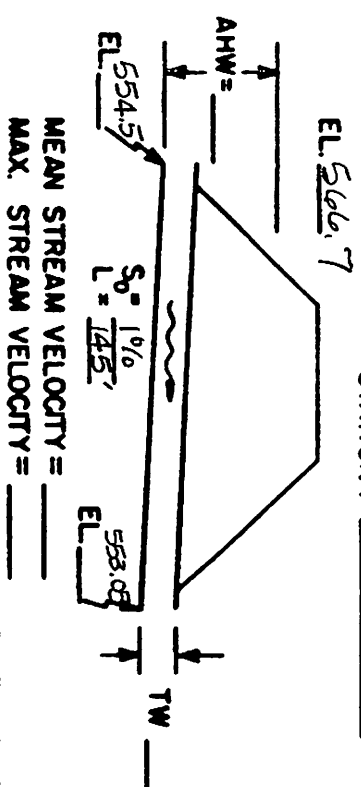
DATE: 1-20-92

HYDROLOGIC AND CHANNEL INFORMATION

SKETCH STATION: 39+95 ±

$Q_1 = \underline{\hspace{2cm}}$ $TW_1 = \underline{\hspace{2cm}}$
 $Q_2 = \underline{543.2}$ $TW_2 = \underline{\hspace{2cm}}$
 (50)

$(Q_1 = \text{DESIGN DISCHARGE, SAY } Q_{25}$
 $Q_2 = \text{CHECK DISCHARGE, SAY } Q_{50} \text{ OR } Q_{100})$

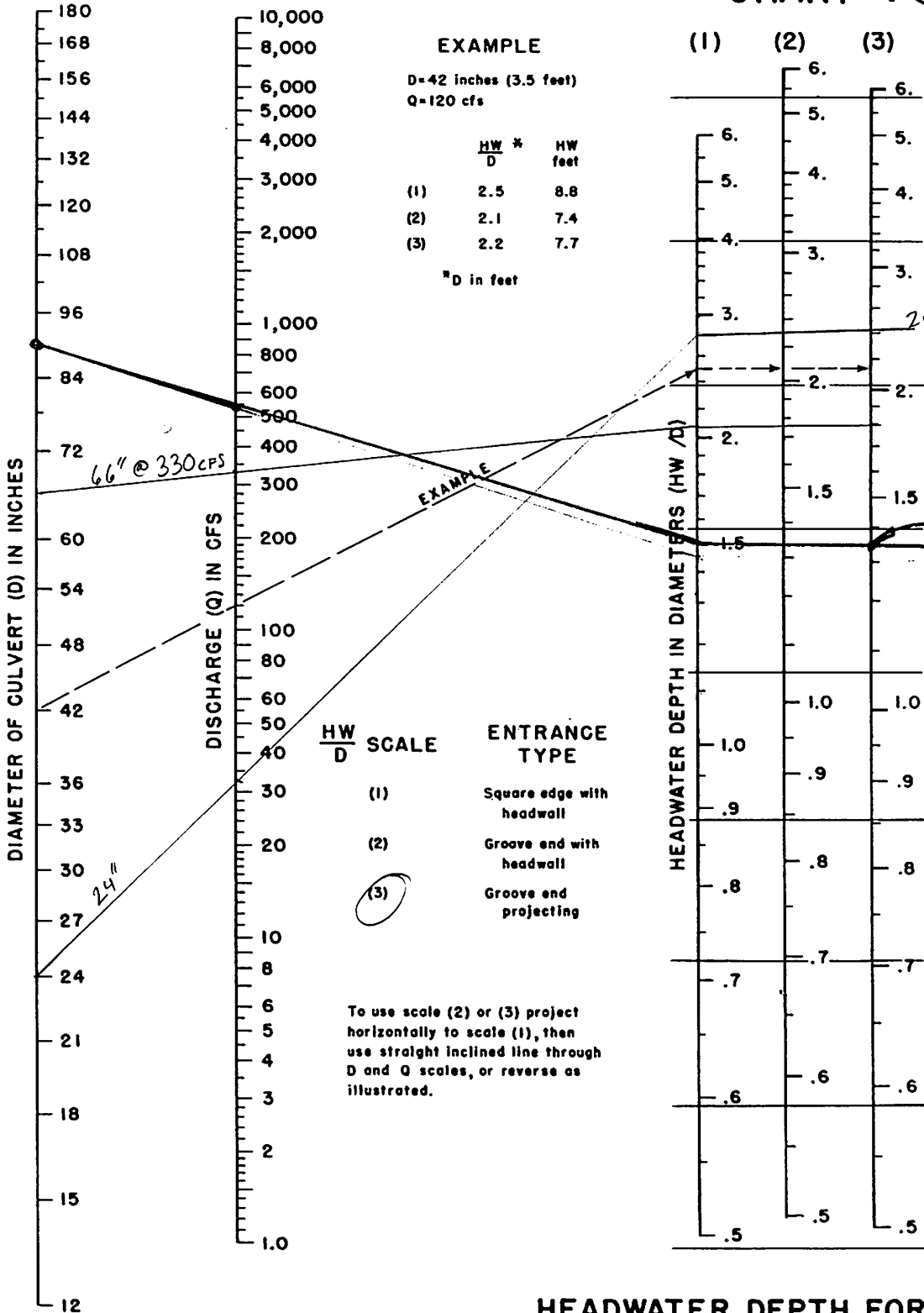


MEAN STREAM VELOCITY = _____
 MAX. STREAM VELOCITY = _____

CULVERT DESCRIPTION (ENTRANCE TYPE)	SIZE	HEADWATER COMPUTATION										CONTROLLING PI	OUTLET VELOCITY	COST	COMMENTS
		INLET CONT.		OUTLET CONTROL				HW=H + h ₀ - LS ₀							
		HW D	HW	K ₀	H	d _c	$\frac{d_c + D}{2}$	TW	h ₀	LS ₀	HW				
90" RCP	90"	1.36	10.2	05	4.0	5.9	6.7	3.5	6.7	1.45	9.25	10.2			

SUMMARY & RECOMMENDATIONS:

CHART 1

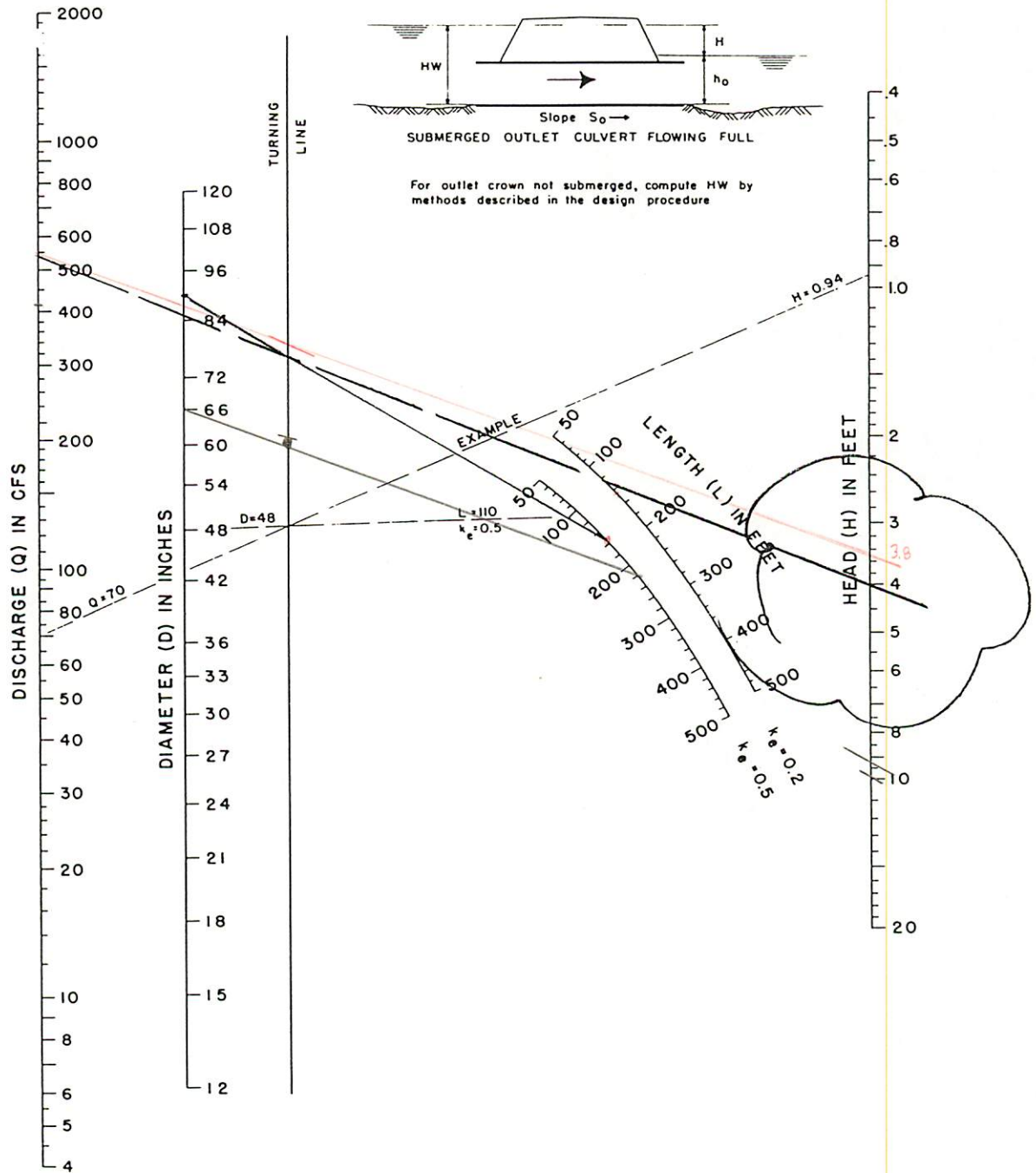


HEADWATER DEPTH FOR CONCRETE PIPE CULVERTS WITH INLET CONTROL

HEADWATER SCALES 2 & 3
 REVISED MAY 1964

BUREAU OF PUBLIC ROADS JAN. 1963

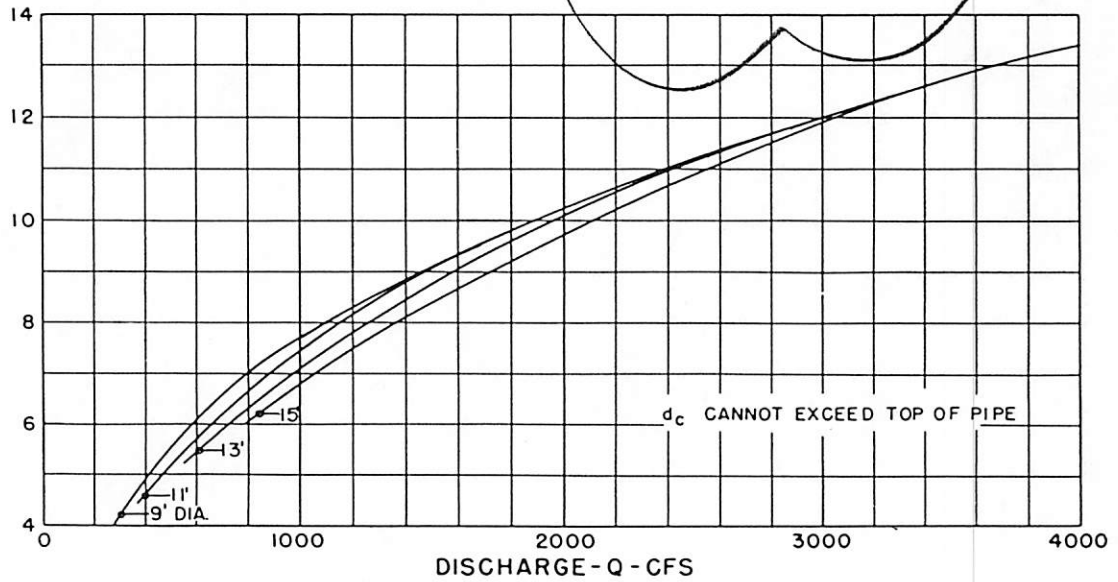
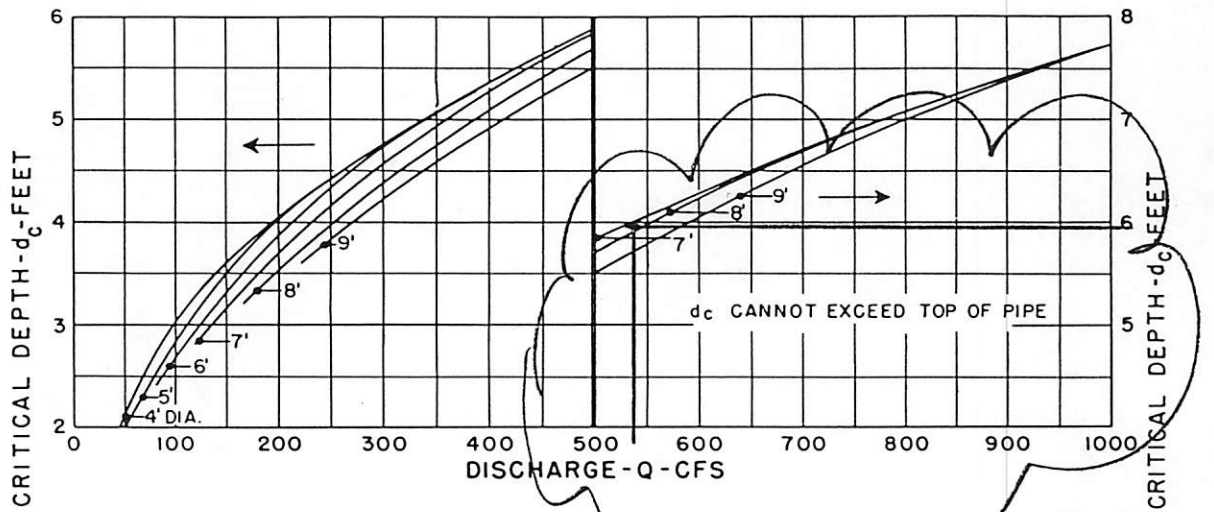
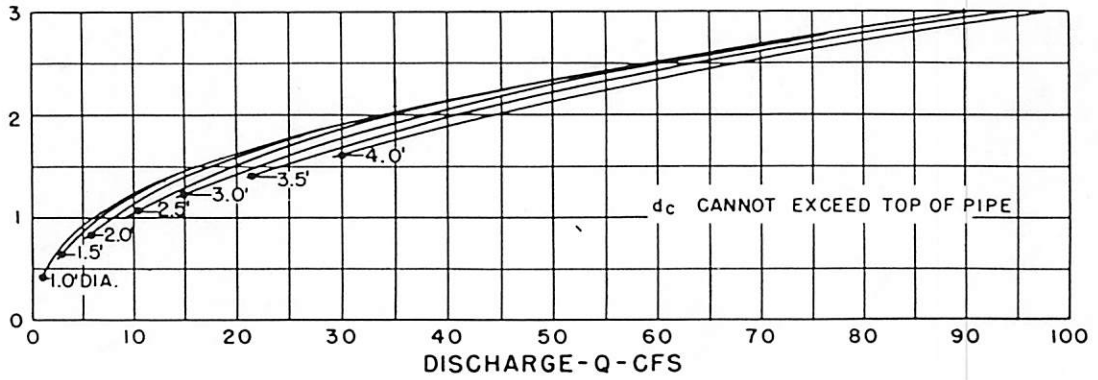
CHART 5



**HEAD FOR
 CONCRETE PIPE CULVERTS
 FLOWING FULL**
 $n = 0.012$



CHART 4



BUREAU OF PUBLIC ROADS
JAN. 1964

CRITICAL DEPTH CIRCULAR PIPE

PICKETT RAY & SILVER

333 MID RIVERS MALL DRIVE
ST. PETERS, MISSOURI 63376
314/441-1211 314/278-1211
FAX 314/278-1104

LETTER OF TRANSMITTAL

DATE 07/17/92	JOB NO. 89-204
ATTENTION MR. FRANK GODWIN	
RE GLEN EAGLE	

TO
CITY OF O'FALLON
138 SOUTH MAIN
O'FALLON, MO 63366

GENTLEMEN:

WE ARE SENDING YOU

- Attached Prints Plans Specifications Copy of letter

COPIES	DATE	NO.	DESCRIPTION
2		4	REVISED LAKE #17 PRINCIPLE SPILLWAY

THESE ARE TRANSMITTED as checked below:

- For your use For approval For review and comment
 As requested Revised as requested FOR BIDS DUE _____ 19_____

REMARKS

COPY TO _____

SIGNED _____

TANYA DIETZ

11

333 MID RIVERS MALL DRIVE
ST. PETERS, MISSOURI 63378
314/447-1211 314/278-1211
FAX 314/278-1104

CITY OF O'FALDON
138 SOUTH MAIN
O'FALDON, MO. 63366

TO

DATE	07/17/92
JOB NO.	89-204
ATTENTION	MR. FRANK GODWIN
RE	GREEN EAGLE

GENTLEMEN

WE ARE SENDING YOU

- Attached
- Prints
- Plans
- Specifications
- Copy of letter

NO.	DESCRIPTION
4	REVISED LAKE #17 PRINCIPLE SPILLWAY

THESE ARE TRANSMITTED as checked below:

- For your use
- For approval
- As requested
- Revised as requested

For review and comment

FOR BIDS DUE

REMARKS

11

Frank Godwin
SIGNED

COPY TO

Lake #17

Revise "Principle Spillway" :

$$Q_{25} = 38.19 \text{ cfs}$$

$$C = 2.68$$

$$L = 11.0$$

$$D = 1.2$$

$$Q_{100} = 48.35$$

$$C = 2.66$$

$$L = 11.0$$

$$D = 1.4$$

$$Q = CLH^{3/2}$$

$$= 2.68(11.0)(1.2)^{3/2}$$

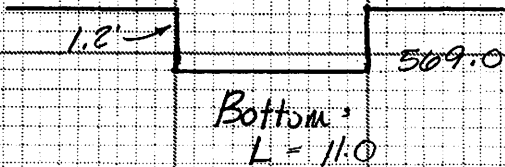
$$Q = 38.75 \text{ cfs}$$

$$Q = CLH^{3/2}$$

$$= 2.66(11)(1.4)^{3/2}$$

$$Q = 48.47 \text{ cfs}$$

570.4 = 100 yr. HW
570.2 = 25 yr. HW



Rev. 7-17-92 *tdh*
 Rev. 3-31-92 *tdh*

```

#####5 OUTLET STRUCTURES #####
: Reservoir: 5
: CULVERT STRUC A.  $Q=C_oA[2gh/k]^1.5$  CULVERT STRUC B.  $Q=C_oA[2gh/k]^1.5$ 
:
: 1. WIDTH (in) = 0.. 9. WIDTH (in) = 0..
: 2. HEIGHT (in) = 0.. 10. HEIGHT (in) = 0..
: 3. No. BARRELS = 0.. 11. No. BARRELS = 0..
: 4. INVERT ELEV. = 0..... 12. INVERT ELEV. = 0.....
: 5.  $C_o = 0.60$  13.  $C_o = 0.60$ 
: 6. CULVERT LENGTH (ft) = 0... 14. CULVERT LENGTH (ft) = 0...
: 7. CULVERT SLOPE (%) = 0... 15. CULVERT SLOPE (%) = 0...
: 8. MANNING'S N-VALUE = .013 16. MANNING'S N-VALUE = .013
: 17. MULTI-STAGE OPTION ? (Y/N) N
:
: WEIR STRUCTURE A.  $Q=C_wLH^EXP$  WEIR STRUCTURE B.  $Q=C_wLH^EXP$ 
:
: 18. CREST LENGTH (ft) = 6..... 23. CREST LENGTH (ft) = 0.....
: 19. CREST ELEVATION = 569..... 24. CREST ELEVATION = 0.....
: 20.  $C_w = 3.00$  25.  $C_w = 3.00$ 
: 21. EXP = 1.50 26. EXP = 1.50
: 22. MULTI-STAGE OPTION ? (Y/N) N 27. MULTI-STAGE OPTION ? (Y/N) N
:
  
```

WEIR STRUCTURE A. $Q=C_wLH^EXP$

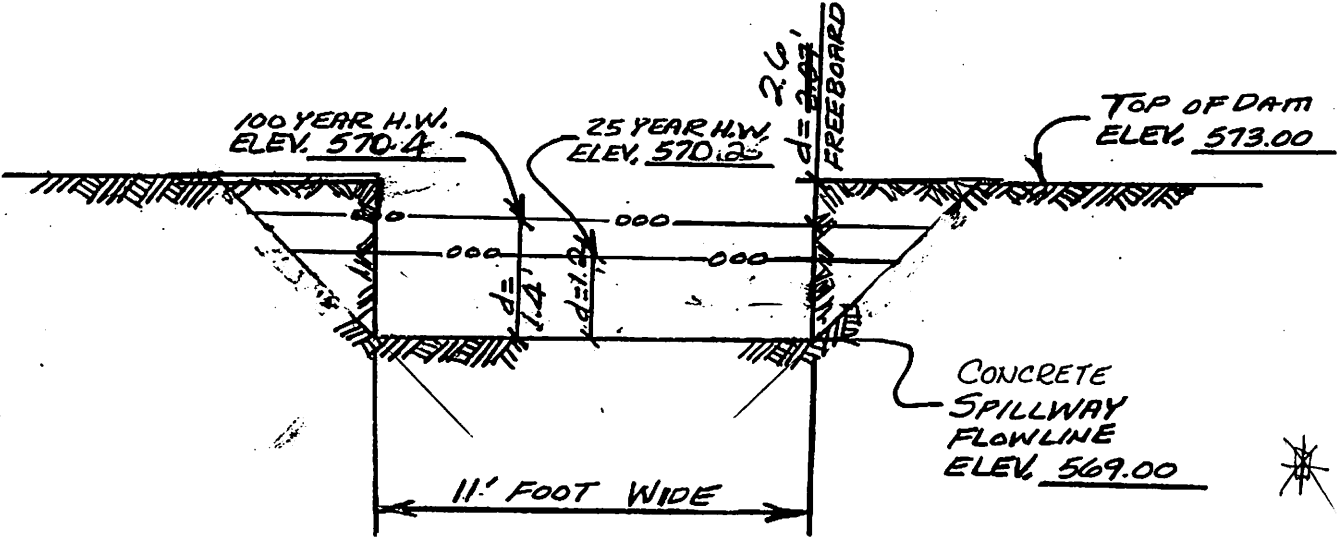
18. CREST LENGTH (ft) = 6.....
 19. CREST ELEVATION = 569.....
 20. $C_w = 3.00$
 21. EXP = 1.50
 22. MULTI-STAGE OPTION ? (Y/N) N

WEIR STRUCTURE B. $Q=C_wLH^EXP$

23. CREST LENGTH (ft) = 0.....
 24. CREST ELEVATION = 0.....
 25. $C_w = 3.00$
 26. EXP = 1.50
 27. MULTI-STAGE OPTION ? (Y/N) N

Change item number: 0

DY to cont



SPILLWAY DETAIL

N.T.S.

12

Rev. 7-17-92 *tkz*

5 OUTLET STRUCTURES #####;

- Reservoir: 5
- CULVERT STRUC A. $Q=C_oA[2gh/k]^0.5$ CULVERT STRUC B. $Q=C_oA[2gh/k]^0.5$
- | | |
|-------------------------------|----------------------------------|
| 1. WIDTH (in) = 0.. | 9. WIDTH (in) = 0.. |
| 2. HEIGHT (in) = 0.. | 10. HEIGHT (in) = 0.. |
| 3. No. BARRELS = 0.. | 11. No. BARRELS = 0.. |
| 4. INVERT ELEV. = 0..... | 12. INVERT ELEV. = 0..... |
| 5. $C_o = 0.60$ | 13. $C_o = 0.60$ |
| 6. CULVERT LENGTH (ft) = 0... | 14. CULVERT LENGTH (ft) = 0... |
| 7. CULVERT SLOPE (%) = 0... | 15. CULVERT SLOPE (%) = 0... |
| 8. MANNING'S N-VALUE = .013 | 16. MANNING'S N-VALUE = .013 |
| | 17. MULTI-STAGE OPTION ? (Y/N) N |

WEIR STRUCTURE A. $Q=C_wLH^EXP$

18. CREST LENGTH (ft) = 6.....

19. CREST ELEVATION = 569....

20. $C_w = 3.00$

21. EXP = 1.50

22. MULTI-STAGE OPTION ? (Y/N) N

WEIR STRUCTURE B. $Q=C_wLH^EXP$

23. CREST LENGTH (ft) = 0.....

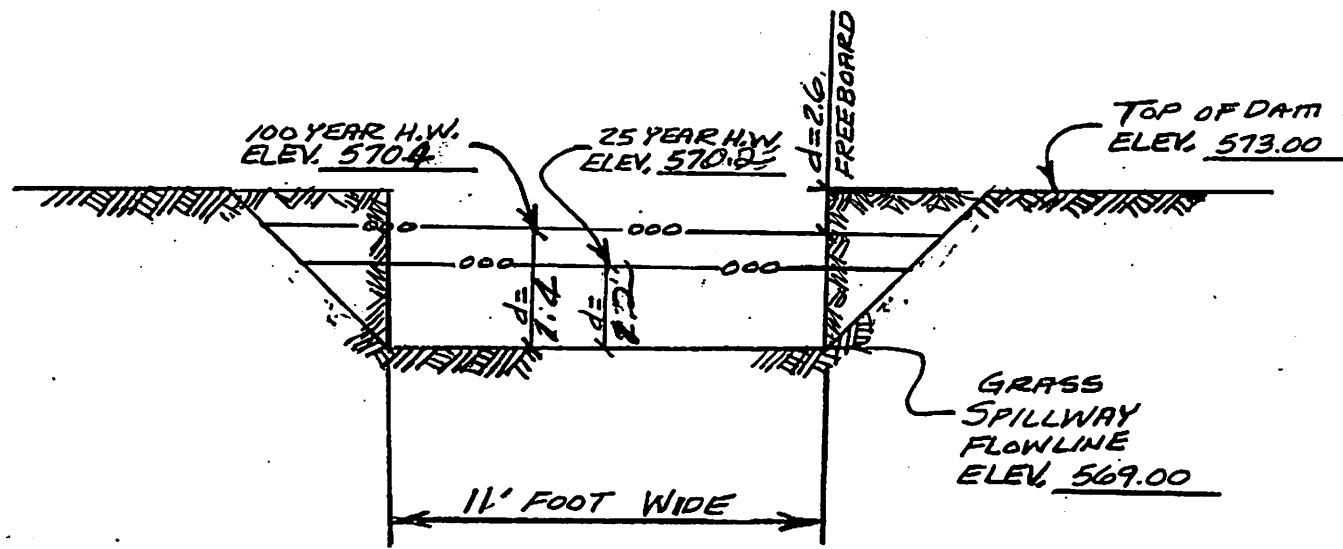
24. CREST ELEVATION = 0.....

25. $C_w = 3.00$

26. EXP = 1.50

27. MULTI-STAGE OPTION ? (Y/N) N

#####
Change item number: 0 DY to cont



SPILLWAY DETAIL
N.T.S.

Table 5-3. Values of C in the Formula $Q = CLH^{3/2}$ for Broad-crested Weirs

Measured head in feet, H	Breadth of crest of weir in feet														
	0.50	0.75	1.00	1.50	2.00	2.50	3.00	4.00	5.00	10.00	15.00	20.00	25.00	30.00	35.00
0.2	2.80	2.75	2.69	2.62	2.54	2.48	2.44	2.38	2.34	2.49	2.68	2.92	3.16	3.40	3.64
0.4	2.92	2.80	2.72	2.64	2.61	2.60	2.68	2.54	2.50	2.56	2.70	3.08	3.29	3.52	3.75
0.6	3.08	2.89	2.75	2.64	2.61	2.60	2.68	2.69	2.70	2.70	2.70	3.30	3.04	2.85	2.68
0.8	3.30	3.04	2.85	2.68	2.60	2.60	2.67	2.68	2.68	2.69	2.64	3.32	3.14	2.98	2.75
1.0	3.32	3.14	2.98	2.75	2.66	2.64	2.65	2.67	2.68	2.68	2.68	3.32	3.20	3.08	2.86
1.2	3.32	3.20	3.08	2.86	2.70	2.65	2.64	2.67	2.66	2.69	2.64	3.32	3.26	3.20	2.92
1.4	3.32	3.26	3.20	2.92	2.77	2.68	2.64	2.65	2.65	2.67	2.64	3.32	3.29	3.28	3.07
1.6	3.32	3.29	3.28	3.07	2.89	2.75	2.68	2.66	2.65	2.64	2.63	3.32	3.32	3.31	3.07
1.8	3.32	3.32	3.31	3.07	2.88	2.74	2.68	2.66	2.65	2.64	2.63	3.32	3.32	3.31	3.03
2.0	3.32	3.31	3.30	3.03	2.85	2.76	2.72	2.68	2.65	2.64	2.63	3.32	3.32	3.31	3.07
2.5	3.32	3.32	3.31	3.28	3.07	2.89	2.81	2.72	2.67	2.64	2.63	3.32	3.32	3.32	3.32
3.0	3.32	3.32	3.32	3.32	3.29	3.05	2.92	2.73	2.66	2.64	2.63	3.32	3.32	3.32	3.32
3.5	3.32	3.32	3.32	3.32	3.32	3.19	2.97	2.76	2.68	2.64	2.63	3.32	3.32	3.32	3.32
4.0	3.32	3.32	3.32	3.32	3.32	3.23	3.07	2.79	2.70	2.64	2.63	3.32	3.32	3.32	3.32
4.5	3.32	3.32	3.32	3.32	3.32	3.32	3.32	3.32	3.32	2.74	2.64	2.63	3.32	3.32	3.32
5.0	3.32	3.32	3.32	3.32	3.32	3.32	3.32	3.32	3.32	2.79	2.64	2.63	3.32	3.32	3.32
5.5	3.32	3.32	3.32	3.32	3.32	3.32	3.32	3.32	3.32	2.88	2.64	2.63	3.32	3.32	3.32

Table 5-4. Values of C in the Formula $Q = CLH^{3/2}$ for Models of Broad-crested Weirs with Rounded Upstream Corner

Name of experimenter	Radius of curve in feet	Breadth of weir in feet, B	Height of weir in feet, P	Head in feet, H												
				0	4	0.6	0.8	1.0	1.5	2.0	2.5	3.0	4.0	5.0		
Bazin.....	0.33	2.62	2.46	2.69	2.97	2.98	3.01	3.04								
Bazin S.....	0.33	6.56	2.46	2.70	2.82	2.87	2.89	2.92								
Waterways S.....	0.33	2.62	4.57	2.77	2.80	2.83	2.92	3.00	3.08	3.17	3.34	3.50			
Waterways Deep.....	0.33	6.56	4.56	2.83	2.83	2.83	2.82	2.82	2.82	2.82	2.82	2.82	2.81		

Table 5-5. Values of C in the Formula $Q = CLH^{3/2}$ for Broad-crested Weirs with Crests Inclined Slightly Downward

Crest	Energy head = H_e									
	0.5	0.6	0.7	0.8	0.9	1.0	1.2	1.4	1.5	
Level.....	2.78	2.79	2.80	2.81	2.82	2.83	2.85	2.85	2.85	
Slope = 0.004.....	2.95	2.94	2.93	2.92	2.91	2.90	2.88	2.87	2.87	
Slope = 0.026.....	3.07	3.06	3.05	3.04	3.03	3.02	3.00	2.99		

(b)

Slope of crest	Length of weir in feet	Head in feet, H						
		0.1	0.2	0.3	0.4	0.5	0.6	0.7
12 to 1.....	3.0	2.58	2.87	2.57	2.60	2.84	2.81	2.70
18 to 1.....	3.0	2.91	2.92	2.53	2.60	2.80	2.74	2.62
18 to 1.....	10.0	2.52	2.68	2.73	2.80	2.90	2.80	2.68

Table 5-6. Values of C in the Formula $Q = CLH^{3/2}$ for Weirs of Triangular Cross Section with Vertical Upstream Face and Sloping Downstream Face

Slope of downstream face	Height of weir in feet, P	Head in feet, H													
		0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0	1.2	1.5			
Hor. Vert. 1 to 1	2.46	3.88	3.85	3.85	3.85	3.85	3.85	3.85	3.85	3.85	3.85	3.85	3.85	3.85	3.85
2 to 1	2.46	3.48	3.48	3.49	3.50	3.50	3.50	3.50	3.50	3.50	3.51	3.51	3.51	3.51	3.51
2 to 1	1.64	3.56	3.47	3.47	3.51	3.54	3.57	3.58	3.58	3.58	3.59	3.57	3.57	3.57	3.57
3 to 1	1.64	2.90	3.11	3.22	3.26	3.33	3.37	3.40	3.40	3.41	3.41	3.41	3.41	3.41
5 to 1	2.46	3.08	3.06	3.05	3.05	3.07	3.09	3.12	3.13	3.13	3.13	3.13	3.13	3.13
10 to 1	2.46	2.82	2.83	2.84	2.86	2.89	2.90	2.91	2.91	2.92	2.92	2.92	2.92	2.92

Lake #17

Revise "Principle Spillway" :

$$Q_{25} = 38.19 \text{ cfs}$$

$$C = 2.68$$

$$L = 11.0$$

$$D = 1.2$$

$$Q_{100} = 48.35$$

$$C = 2.66$$

$$L = 11.0$$

$$D = 1.4$$

$$Q = CLH^{3/2}$$

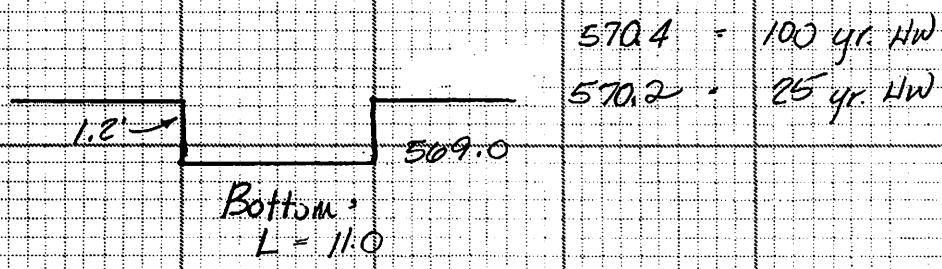
$$= 2.68(11.0)(1.2)^{3/2}$$

$$Q = 38.75 \text{ cfs}$$

$$Q = CLH^{3/2}$$

$$= 2.66(11)(1.4)^{3/2}$$

$$Q = 48.47 \text{ cfs}$$



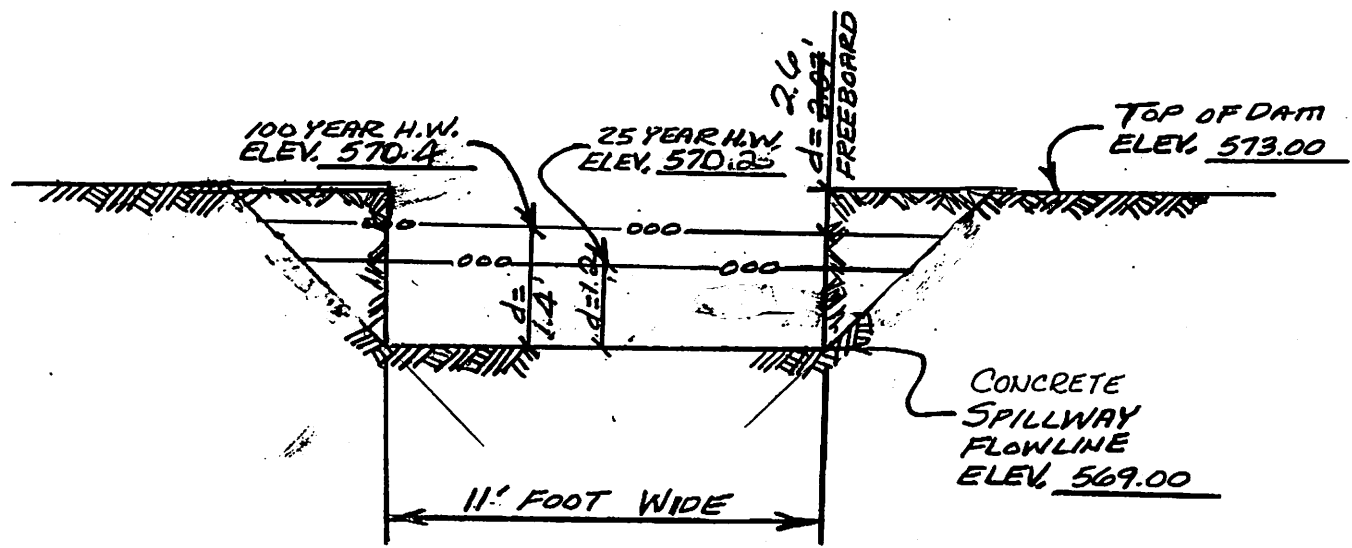
Rev. 7-17-92 TCS
 Rev. 3-31-92 TCS

```

#####5 OUTLET STRUCTURES #####
: Reservoir: 5
: CULVERT STRUC A. Q=CoA[2gh/k]^1.5    CULVERT STRUC B. Q=CoA[2gh/k]^1.5
:
: 1. WIDTH (in) = 0..                9. WIDTH (in) = 0..
: 2. HEIGHT (in) = 0..                10. HEIGHT (in) = 0..
: 3. No. BARRELS = 0..                11. No. BARRELS = 0..
: 4. INVERT ELEV. = 0.....           12. INVERT ELEV. = 0.....
: 5. Co = 0.60                        13. Co = 0.60
: 6. CULVERT LENGTH (ft) = 0...       14. CULVERT LENGTH (ft) = 0...
: 7. CULVERT SLOPE (%) = 0...         15. CULVERT SLOPE (%) = 0...
: 8. MANNING'S N-VALUE = .013         16. MANNING'S N-VALUE = .013
:                                     17. MULTI-STAGE OPTION ? (Y/N) N
:
: WEIR STRUCTURE A.      Q=CwLH^EXP    WEIR STRUCTURE B.      Q=CwLH^EXP
:
: 18. CREST LENGTH (ft) = 6.....      23. CREST LENGTH (ft) = 0.....
: 19. CREST ELEVATION = 569....        24. CREST ELEVATION = 0.....
: 20. Cw = 3.00                       25. Cw = 3.00
: 21. EXP = 1.50                       26. EXP = 1.50
: 22. MULTI-STAGE OPTION ? (Y/N) N     27. MULTI-STAGE OPTION ? (Y/N) N
:
  
```

Change item number: 0

DY to cont



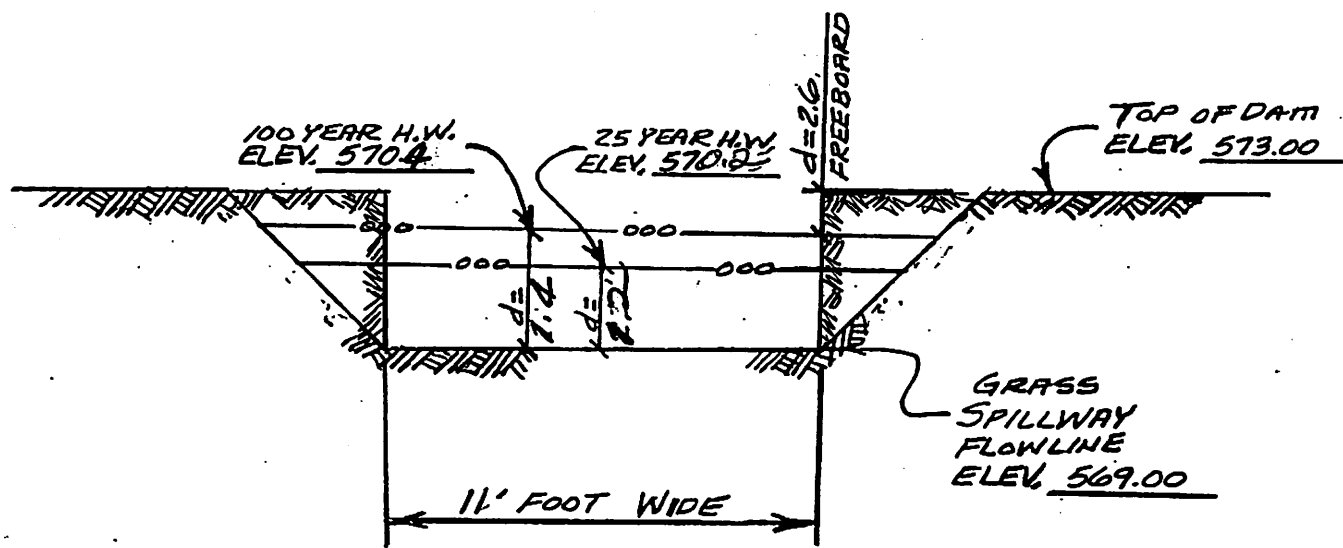
SPILLWAY DETAIL

N.T.S.

Rev. 7-17-92 *tk*

*****5 OUTLET STRUCTURES *****
 : Reservoir: 5
 : CULVERT STRUC A. $Q=C_0A[2gh/k]^{.5}$ CULVERT STRUC B. $Q=C_0A[2gh/k]^{.5}$
 :
 : 1. WIDTH (in) = 0.. 9. WIDTH (in) = 0..
 : 2. HEIGHT (in) = 0.. 10. HEIGHT (in) = 0..
 : 3. No. BARRELS = 0.. 11. No. BARRELS = 0..
 : 4. INVERT ELEV. = 0..... 12. INVERT ELEV. = 0.....
 : 5. $C_0 = 0.60$ 13. $C_0 = 0.60$
 : 6. CULVERT LENGTH (ft) = 0... 14. CULVERT LENGTH (ft) = 0...
 : 7. CULVERT SLOPE (%) = 0... 15. CULVERT SLOPE (%) = 0...
 : 8. MANNING'S N-VALUE = .013 16. MANNING'S N-VALUE = .013
 : 17. MULTI-STAGE OPTION ? (Y/N) N
 :
 WEIR STRUCTURE A. $Q=C_wLH^{EXP}$ WEIR STRUCTURE B. $Q=C_wLH^{EXP}$
 : 18. CREST LENGTH (ft) = 6..... 23. CREST LENGTH (ft) = 0.....
 : 19. CREST ELEVATION = 569.... 24. CREST ELEVATION = 0.....
 : 20. $C_w = 3.00$ 25. $C_w = 3.00$
 : 21. EXP = 1.50 26. EXP = 1.50
 : 22. MULTI-STAGE OPTION ? (Y/N) N 27. MULTI-STAGE OPTION ? (Y/N) N
 :

Change item number: 0 DY to cont



SPILLWAY DETAIL
 N.T.S.

Table 5-3. Values of C in the Formula $Q = CLH^{3/2}$ for Broad-crested Weirs

Measured head in feet, H	Breadth of crest of weir in feet										
	0.50	0.75	1.00	1.50	2.00	2.50	3.00	4.00	5.00	10.00	15.00
0.2	2.80	2.75	2.69	2.62	2.54	2.48	2.44	2.38	2.34	2.49	2.68
0.4	2.92	2.80	2.72	2.64	2.61	2.60	2.58	2.54	2.50	2.56	2.70
0.6	3.08	2.89	2.75	2.64	2.61	2.60	2.68	2.69	2.70	2.70	2.70
0.8	3.30	3.04	2.85	2.68	2.60	2.60	2.67	2.68	2.68	2.69	2.64
1.0	3.32	3.14	2.98	2.75	2.66	2.64	2.65	2.67	2.68	2.68	2.63
1.2	3.32	3.20	3.08	2.86	2.70	2.65	2.64	2.67	2.65	2.69	2.64
1.4	3.32	3.26	3.20	2.92	2.77	2.68	2.64	2.65	2.65	2.67	2.64
1.6	3.32	3.29	3.28	3.07	2.89	2.75	2.68	2.66	2.65	2.64	2.63
1.8	3.32	3.32	3.31	3.07	2.88	2.74	2.68	2.66	2.65	2.64	2.63
2.0	3.32	3.31	3.30	3.03	2.85	2.76	2.72	2.68	2.65	2.64	2.63
2.5	3.32	3.32	3.31	3.28	3.07	2.89	2.81	2.72	2.67	2.64	2.63
3.0	3.32	3.32	3.32	3.32	3.20	3.05	2.92	2.73	2.66	2.64	2.63
3.5	3.32	3.32	3.32	3.32	3.32	3.19	2.97	2.76	2.68	2.64	2.63
4.0	3.32	3.32	3.32	3.32	3.32	3.32	3.07	2.79	2.70	2.64	2.63
4.5	3.32	3.32	3.32	3.32	3.32	3.32	3.32	2.88	2.74	2.64	2.63
5.0	3.32	3.32	3.32	3.32	3.32	3.32	3.32	3.07	2.79	2.64	2.63
5.5	3.32	3.32	3.32	3.32	3.32	3.32	3.32	3.32	2.88	2.64	2.63

Table 5-4. Values of C in the Formula $Q = CLH^{3/2}$ for Models of Broad-crested Weirs with Rounded Upstream Corner

Name of experimenter	Radius of curve in feet	Breadth of weir in feet, B	Height of weir in feet, p	Head in feet, H												
				0	4	0.6	0.8	1.0	1.5	2.0	2.5	3.0	4.0	5.0		
Bazin	0.33	2.62	2.46	2.93	2.97	2.98	3.01	3.04								
Bazin	0.33	6.56	2.46	2.70	2.82	2.87	2.89	2.92								
U. Waterways	0.33	2.62	4.57	2.77	2.80	2.83	2.92	3.00	3.08	3.17	3.34	3.50				
U. Waterways	0.33	6.56	4.56	2.83	2.83	2.83	2.82	2.82	2.82	2.82	2.82	2.81				

Table 5-5. Values of C in the Formula $Q = CLH^{3/2}$ for Broad-crested Weirs with Crests Inclined Slightly Downward

Crest	Energy head = H_e									
	0.5	0.6	0.7	0.8	0.9	1.0	1.2	1.4	1.5	
Level	2.78	2.79	2.80	2.81	2.82	2.83	2.85	2.85	2.85	
Slope = 0.004	2.95	2.94	2.93	2.92	2.91	2.90	2.88	2.87	2.87	
Slope = 0.026	3.07	3.06	3.05	3.04	3.03	3.02	3.00	2.99		

(a)

Slope of crest	Length of weir in feet	Head in feet, H						
		0.1	0.2	0.3	0.4	0.5	0.6	0.7
12 to 1	3.0	2.58	2.87	2.57	2.60	2.84	2.81	2.70
18 to 1	3.0	2.91	2.82	2.53	2.60	2.80	2.74	2.62
18 to 1	10.0	2.52	2.68	2.73	2.80	2.90	2.80	2.68

Table 5-6. Values of C in the Formula $Q = CLH^{3/2}$ for Weirs of Triangular Cross Section with Vertical Upstream Face and Sloping Downstream Face

Slope of down-stream face	Height of weir in feet, p	Head in feet, H											
		0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0	1.2	1.5	
Hor. Vert.	2.46	3.88	3.85	3.85	3.85	3.85	3.85	3.85	3.85	3.85	3.85	3.85	3.85
1 to 1	2.46	3.48	3.48	3.49	3.50	3.50	3.50	3.50	3.50	3.50	3.51	3.51	3.51
2 to 1	1.64	3.56	3.47	3.47	3.51	3.54	3.57	3.58	3.58	3.58	3.59	3.59	3.57
3 to 1	1.64	2.90	3.11	3.22	3.26	3.33	3.37	3.40	3.41	3.41	3.41	3.41	3.41
5 to 1	2.46	3.08	3.06	3.05	3.07	3.09	3.12	3.13	3.13	3.13	3.13	3.13	3.13
10 to 1	2.46	2.82	2.83	2.84	2.86	2.89	2.90	2.91	2.91	2.92	2.92	2.92	2.93

PICKETT RAY & SILVER

333 Mid Rivers Mall Dr
St. Peters, MO 63376

Civil Engineers
Planners
Land Surveyors

441-1211
278-1211

PROJECT NAME Glen Eagle

PROJECT #/JOB ORDER # _____

DATE _____

DESIGNER _____

PAGE 1

Lake #5

1. tributary area: 104.90 A^c
2. Rational method runoff coefficient = C = 0.6
3. Sediment storage (per chart) = 80 cf./yr.
4. Total sediment storage = $80 \times 104.9 = 8392 \times 2 \text{ yrs} = 16,784 \text{ cf}$

per Calc = total storage = 282,073
req'd storage =

Lake #8

1. area: 134 A^c
2. C = 0.6
3. storage = 78 cf/yr.
4. Total = $78 \times 134 = 10452 \times 2 \text{ yrs} = 20,904 \text{ cf}$

Lake #13

1. 35.3 acres
2. C = 0.6
3. storage = 110 cf/yr
4. Total = $110 \times 35.3 = 3883 \times 2 \text{ yrs} = 7,766$

8

PICKETT RAY & SILVER

333 Mid Rivers Mall Dr
St. Peters, MO 63376

Civil Engineers
Planners
Land Surveyors

441-1211
278-1211

PROJECT NAME _____

PROJECT #/JOB ORDER # _____

DATE _____

DESIGNER _____

PAGE 2

Lake #14

1. Area = 12 A^s
2. C = 0.0
3. Storage = 145
4. Total = $145 \times 12 = 1740 \times 2 \text{ yrs} = 3480 \text{ cf}$

per Calc. = total storage = 98,824
Storage Req'd = 60,300 cf (for 33.5 cfs)

Lake # 17

1. Area = 14.7 A^s
2. C = 0.6
3. Storage = 140
4. Total = $140 \times 14.7 = 2058 \times 2 \text{ yrs} = 4116 \text{ cf}$

per Calc. total storage = 134,437
Storage Req'd = 132,660 (73.70 cfs)
(1,777 cf difference)

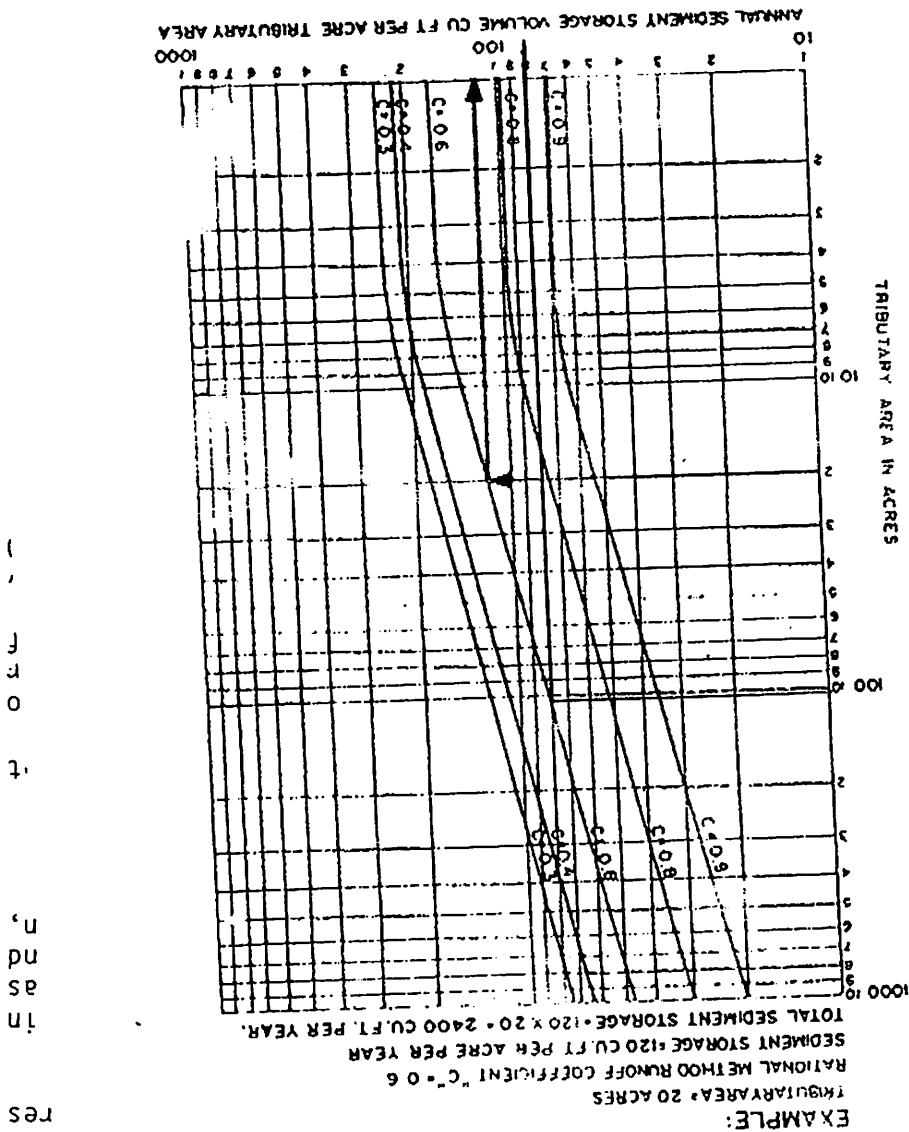
by using Figure 6. All other detention facilities shall provide storage for 2 years of sediment accumulation by using Figure 1, except for those using roofs of buildings, paved parking areas or other facilities designed to preclude

Handwritten notes:
7/27/90
7/27/90
7/27/90
7/27/90

Handwritten notes:
Change
1-92
10/28

FIG. 6

ANNUAL SEDIMENT STORAGE



les
ety
res
in
as
nd
n,
t
o
r
f
)

#5

FIGURE 1

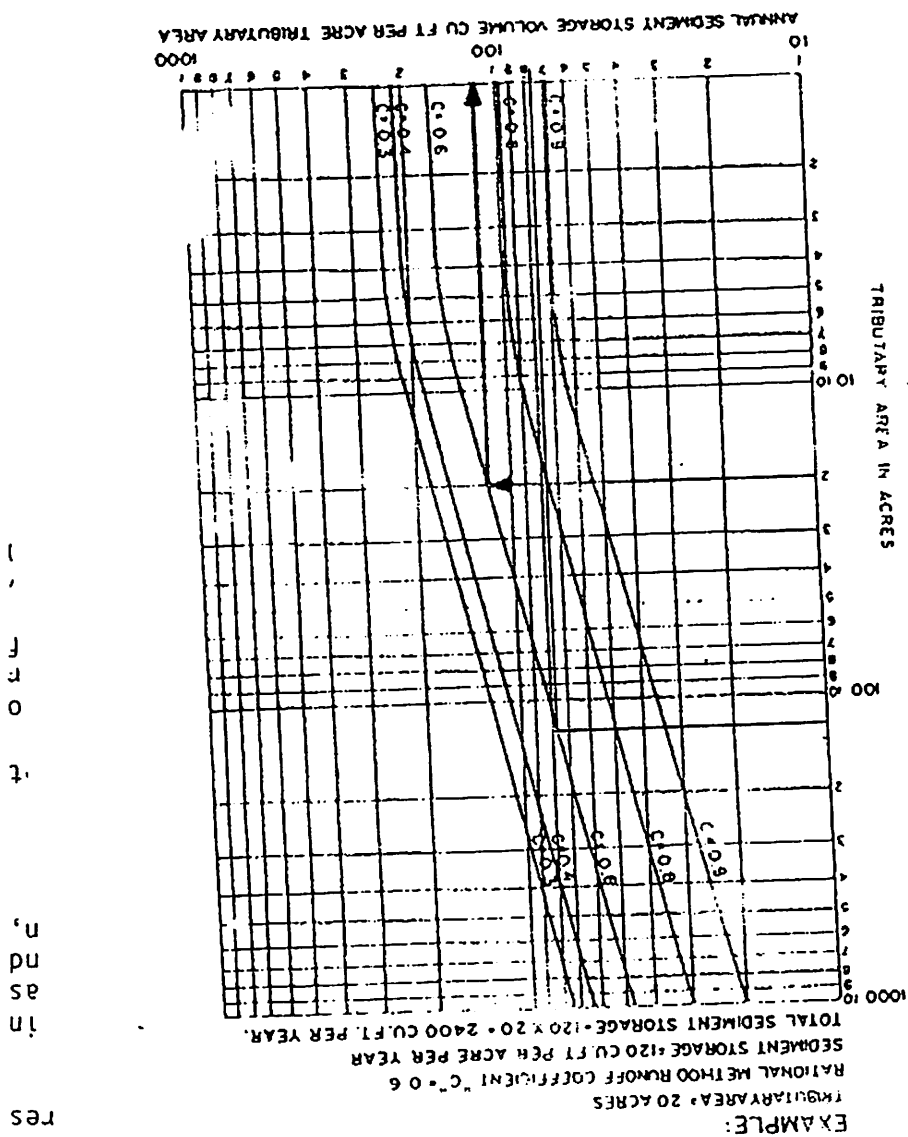
7/27/90

by using figures. All other detention facilities shall provide storage for 2 years of sediment accumulation by using figure 1, except for those using roofs of buildings, paved parking areas or other facilities designed to preclude

Handwritten notes:
1.02
1.03
1.04
1.05
1.06
1.07
1.08
1.09
1.10

Handwritten notes in a circle:
Always
1.02
1.03
1.04
1.05

FIG. 6 ANNUAL SEDIMENT STORAGE



les
ety
res
in
as
nd
n,
t.
o
r
f
)

FIGURE 1

7/27/90

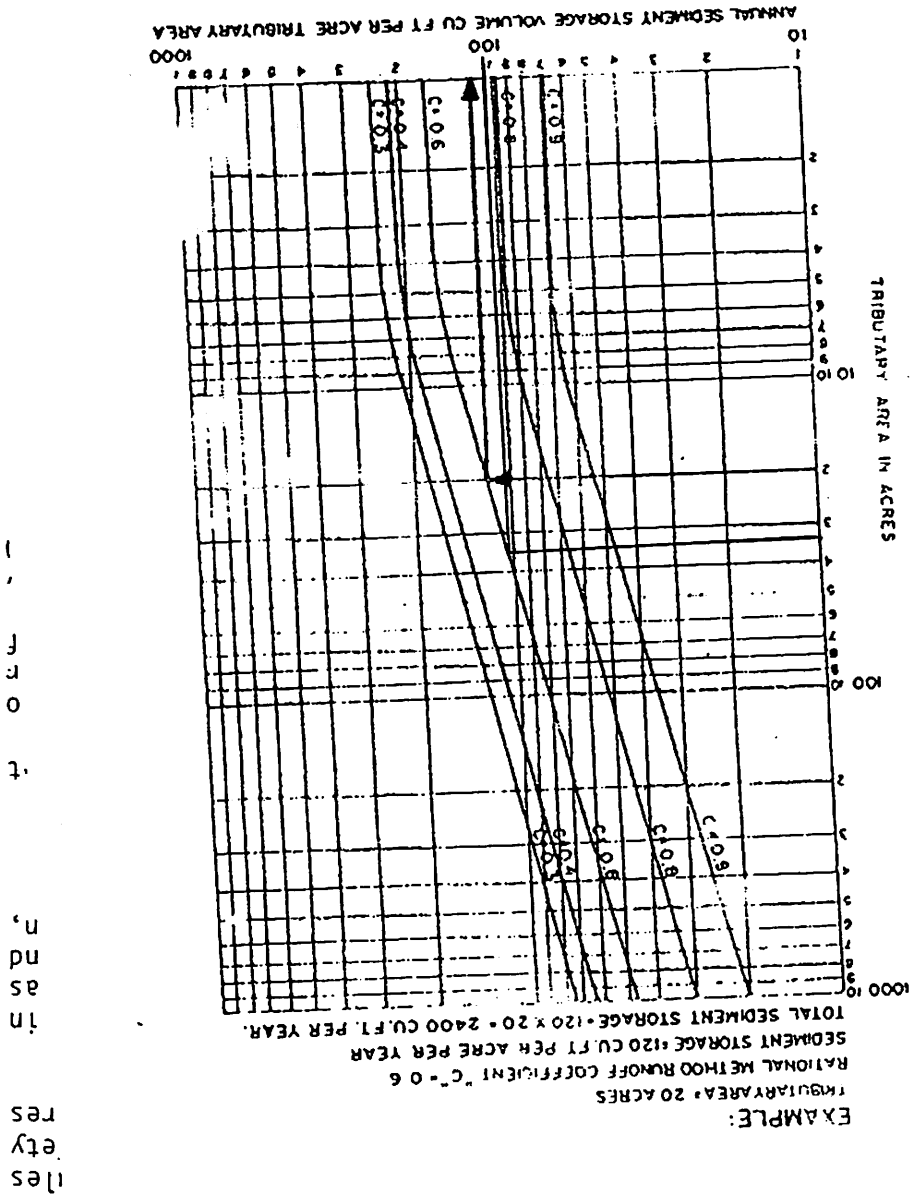
#8

by using Figure 6. All other detention facilities shall provide storage for 2 years of sediment accumulation by using Figure 1, except for those using roofs of buildings, paved parking areas or other facilities designed to preclude

Handwritten notes:
See
7/27/90
1.92
1.03
1.06
1.09

Handwritten notes:
1.92
1.03
1.06
1.09

FIG. 6 ANNUAL SEDIMENT STORAGE



iles
ety
res
in
as
nd
n,
t.
0
n
f
)

#13

FIGURE 1

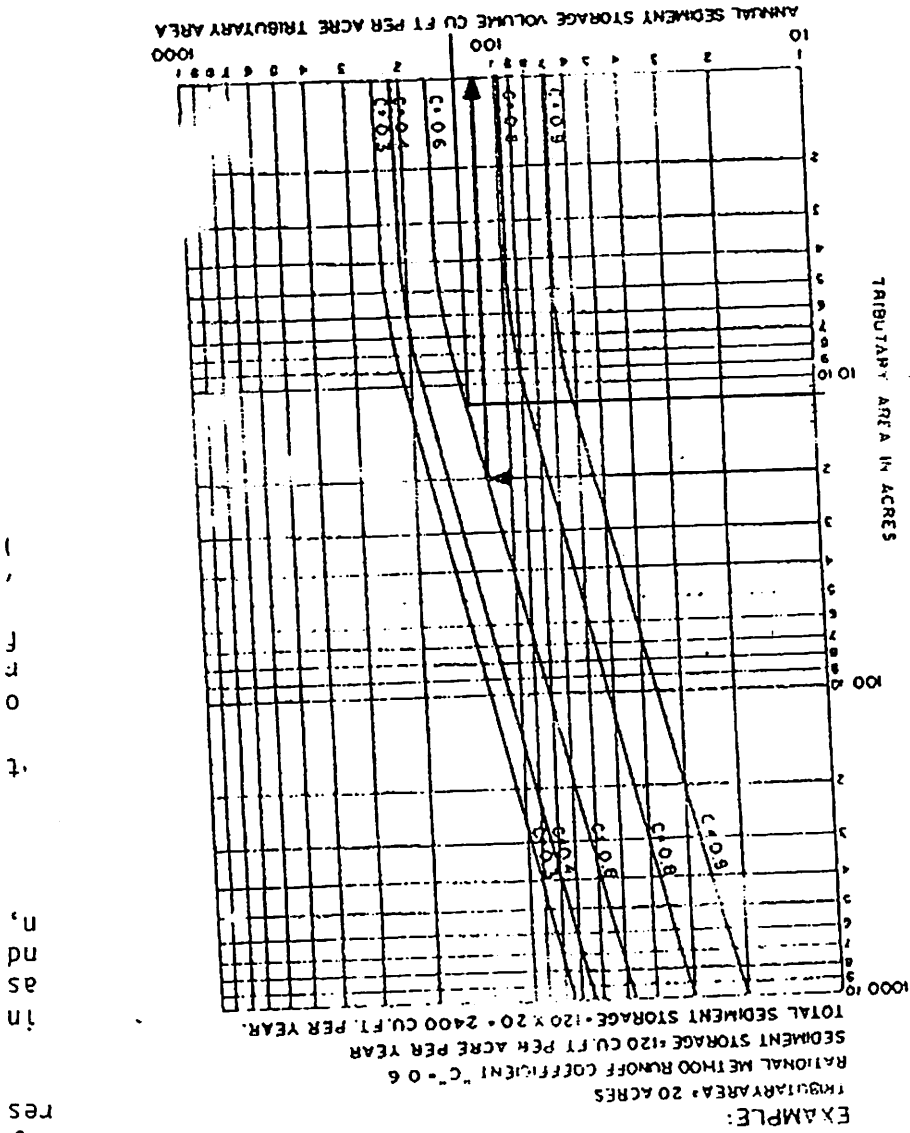
7/27/90

by using Figure 6. All other detention facilities shall provide storage for 2 years of sediment accumulation by using Figure 1, except for those using roofs of buildings, paved parking areas or other facilities designed to preclude

Handwritten notes:
1.05
1.00
0.95
0.90
0.85
0.80
0.75
0.70
0.65
0.60
0.55
0.50
0.45
0.40
0.35
0.30
0.25
0.20
0.15
0.10
0.05
0.00

Handwritten notes:
1.05
1.00
0.95
0.90
0.85
0.80
0.75
0.70
0.65
0.60
0.55
0.50
0.45
0.40
0.35
0.30
0.25
0.20
0.15
0.10
0.05
0.00

FIG. 6 ANNUAL SEDIMENT STORAGE



in
as
nd
n,
t.
0
r
f
1

FIGURE 1
7/27/90

#14

6

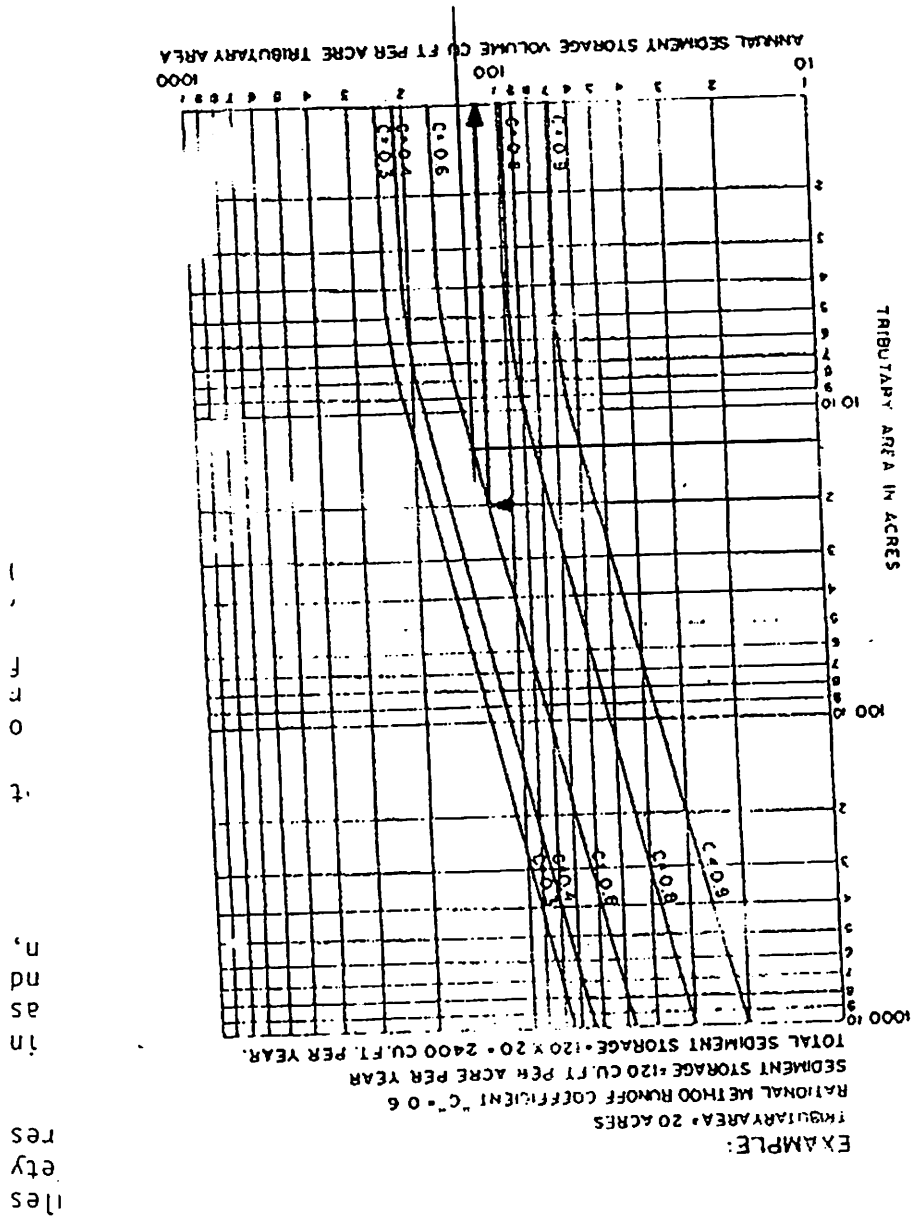
by using Figure 6. All other detention facilities shall provide storage for 2 years of sediment accumulation by using Figure 7, except for those using roofs of buildings, paved parking areas or other facilities designed to preclude

Handwritten notes:
7/27/90
1.92
1.92
1.92
1.92

Handwritten notes:
1.92
1.92
1.92
1.92

FIG. 6

ANNUAL SEDIMENT STORAGE



les
ety
res
in
as
nd
n,
t
o
r
f
l

FIGURE 1

7/27/90

#17

PROJECT: Glen Eagle
89-204

DESIGNER: J. Dietz
DATE: 10-15-91

HYDROLOGIC AND CHANNEL INFORMATION

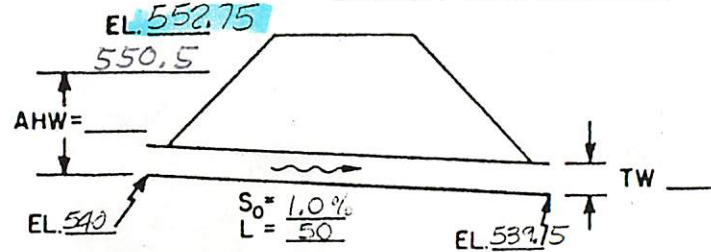
Flood Plain Elev.: 542.5 ±
Flood way : 544.0 ±

$Q_1 =$ _____ $TW_1 =$ _____
 $Q_2 =$ 3602₁₀₀ $TW_2 =$ _____

(Q_1 = DESIGN DISCHARGE, SAY Q_{25}
 Q_2 = CHECK DISCHARGE, SAY Q_{50} OR Q_{100})

SKETCH

STATION: 20+11.50



MEAN STREAM VELOCITY = _____
MAX. STREAM VELOCITY = _____

2' FB
OK

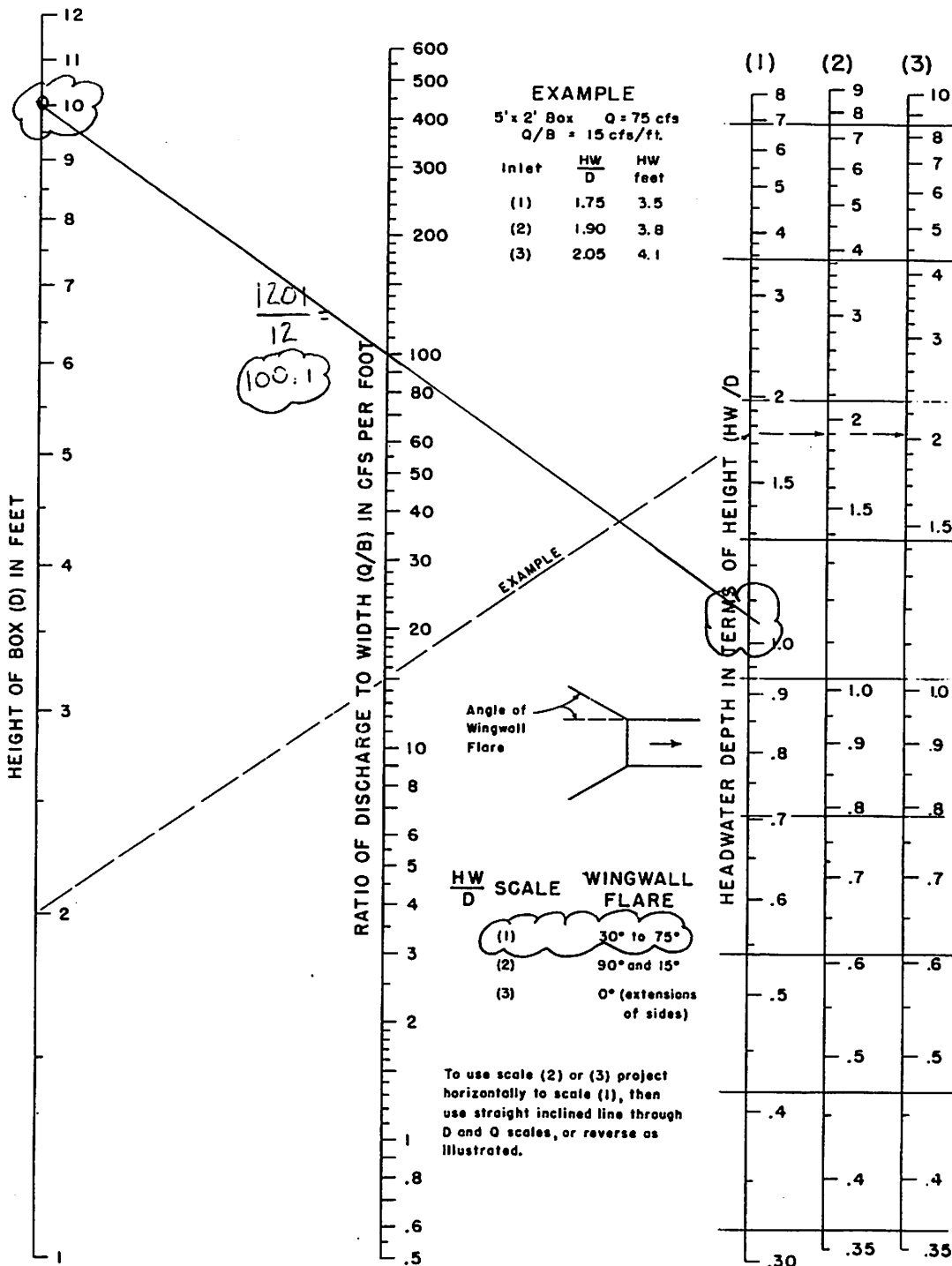
CULVERT DESCRIPTION (ENTRANCE TYPE)	Q	SIZE	HEADWATER COMPUTATION										CONTROLLING HW	OUTLET VELOCITY	COST	COMMENTS		
			INLET CONT.		OUTLET CONTROL HW=H+h ₀ -LS ₀						TW	h ₀					LS ₀	HW
			H/W D	HW	K _e	H	d _c	$\frac{d_c+D}{2}$										
1- 45° Wing Walls Box CULVERT	(1302) 1201	3- 12x10	1.05	10.5	0.4	2.4	6.8	8.4	5.0	8.4	.5	10.3	10.5					

SUMMARY & RECOMMENDATIONS:

$\frac{6.8+10}{2} = 8.4$ use > of $\frac{d_c+D}{2} + TW$



CHART 8



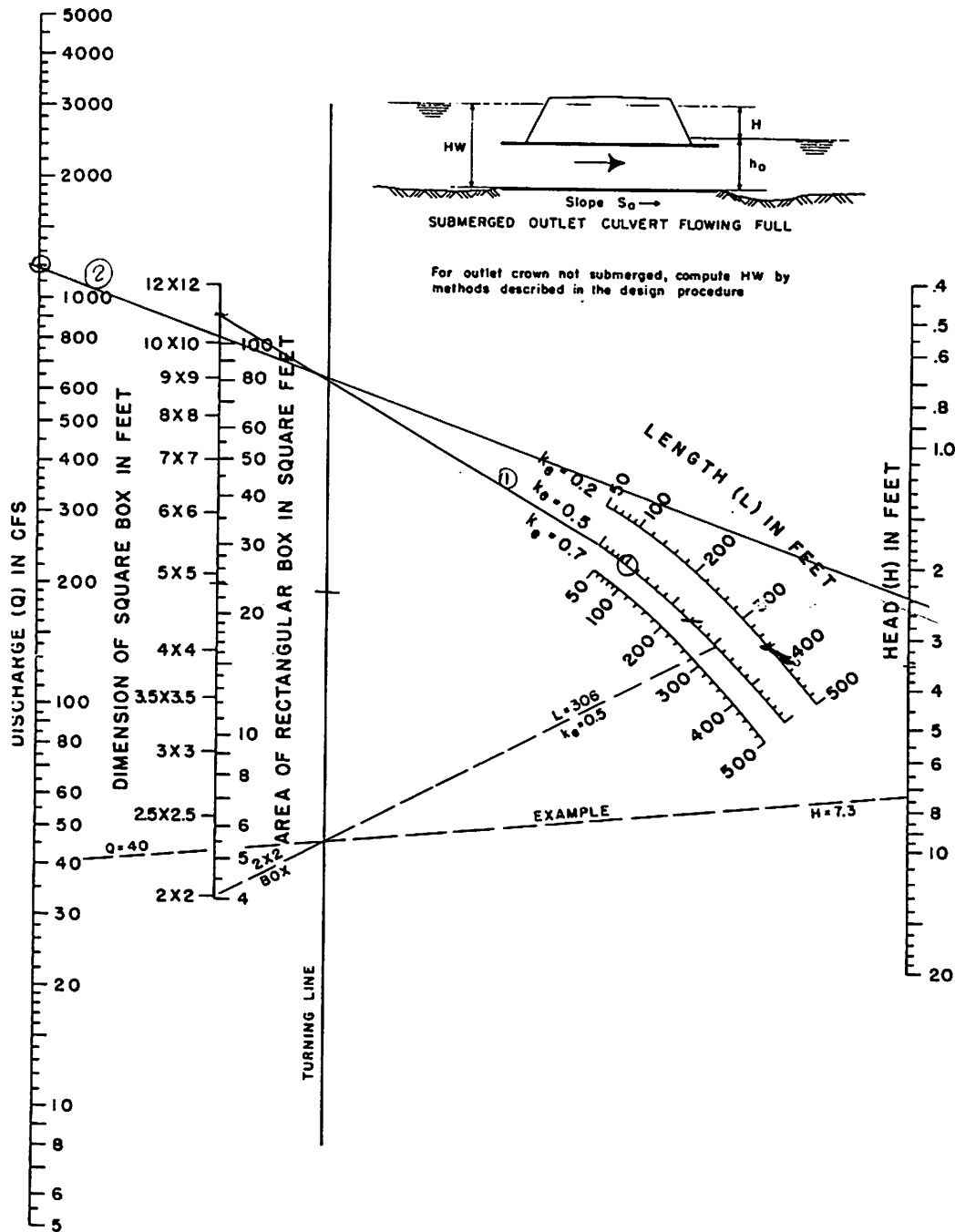
BUREAU OF PUBLIC ROADS JAN. 1963

HEADWATER DEPTH FOR BOX CULVERTS WITH INLET CONTROL

* 3 - 12' W x 10' H Box.
 $Q_{in} = 3602 \div 3 = 1201$ cfs/Box



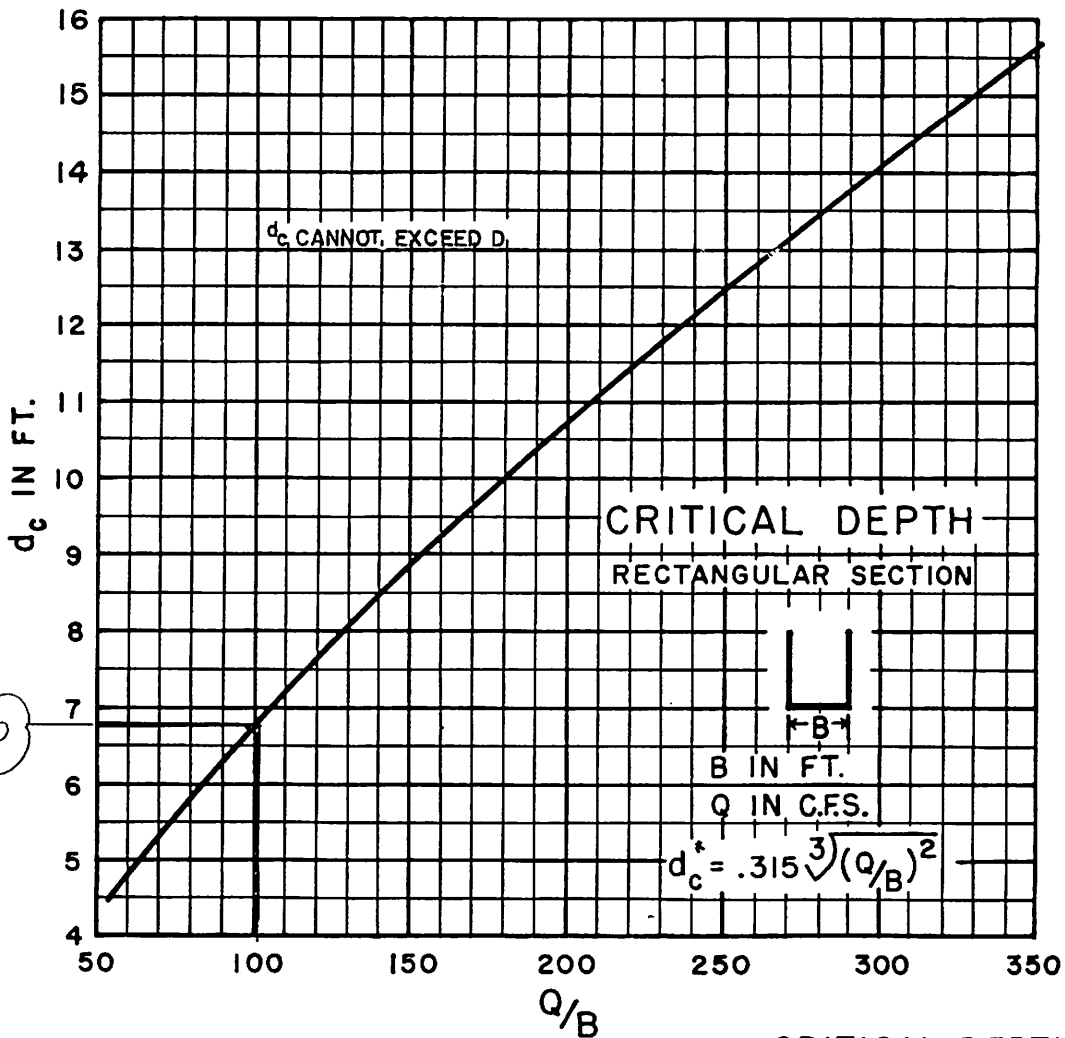
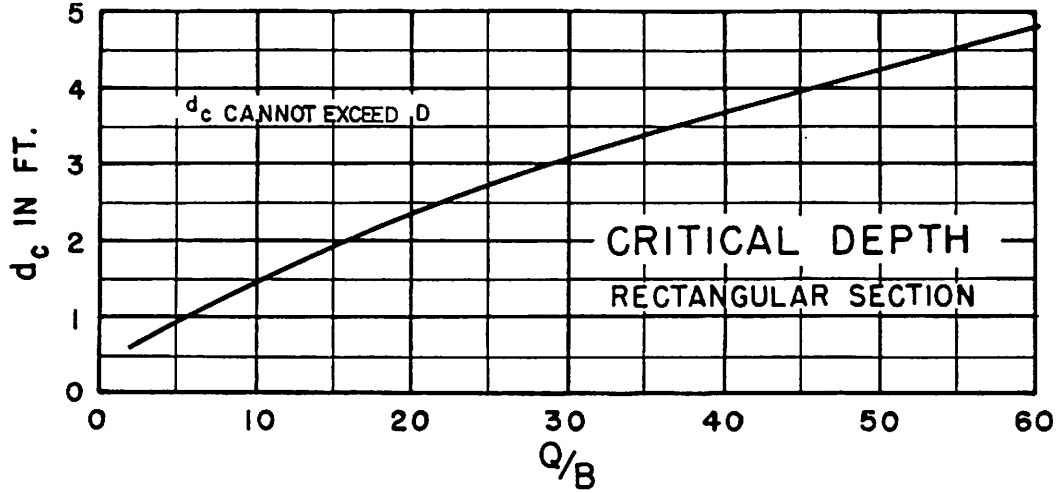
CHART 15



HEAD FOR
CONCRETE BOX CULVERTS
FLOWING FULL
 $n = 0.012$



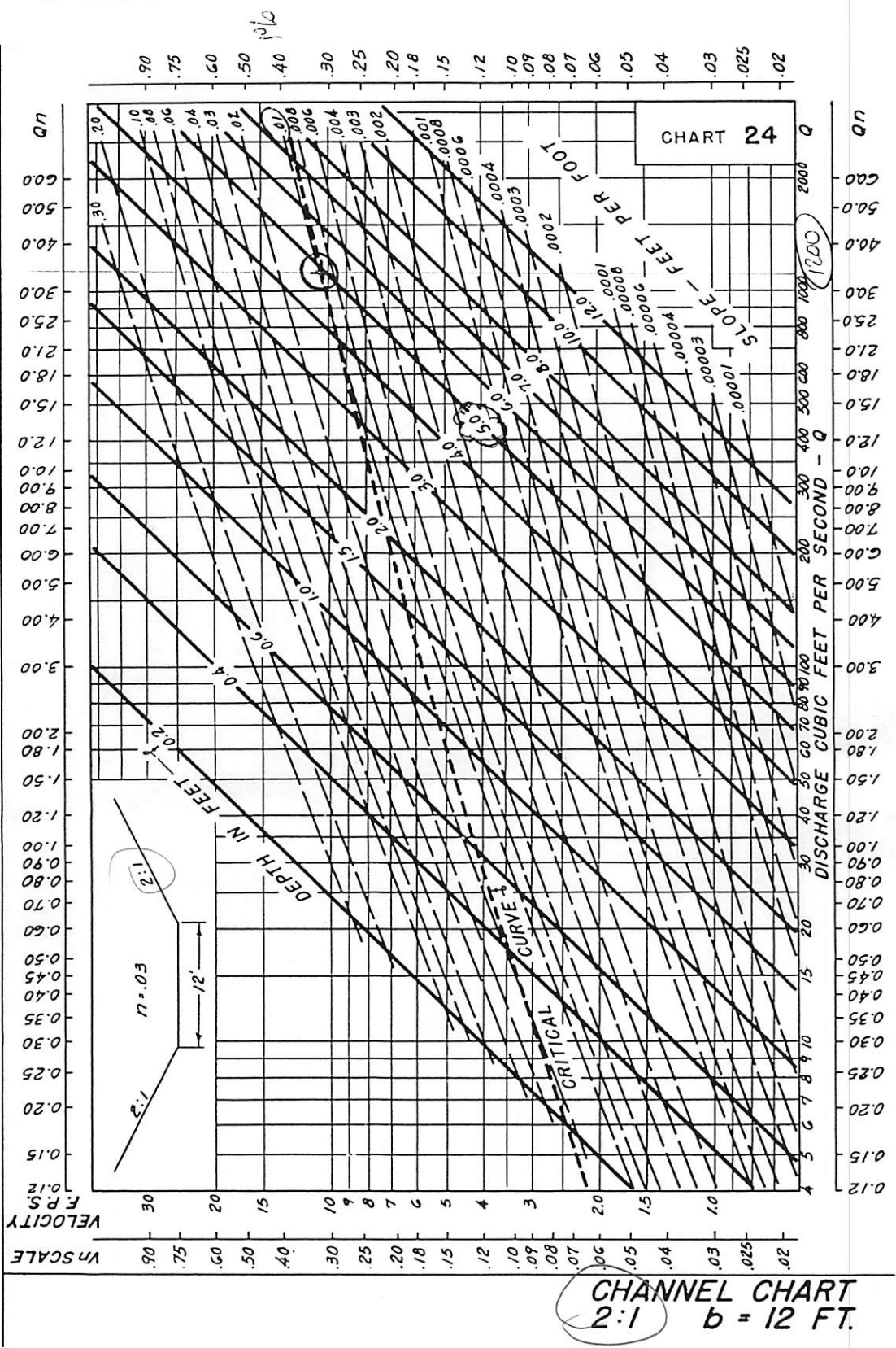
CHART 14



BUREAU OF PUBLIC ROADS JAN. 1963

5-38

CRITICAL DEPTH
RECTANGULAR SECTION



PICKETT RAY & SILVER

333 Mid Rivers Mall Dr
St. Peters, MO 63376

Civil Engineers
Planners
Land Surveyors

441-1211
278-1211

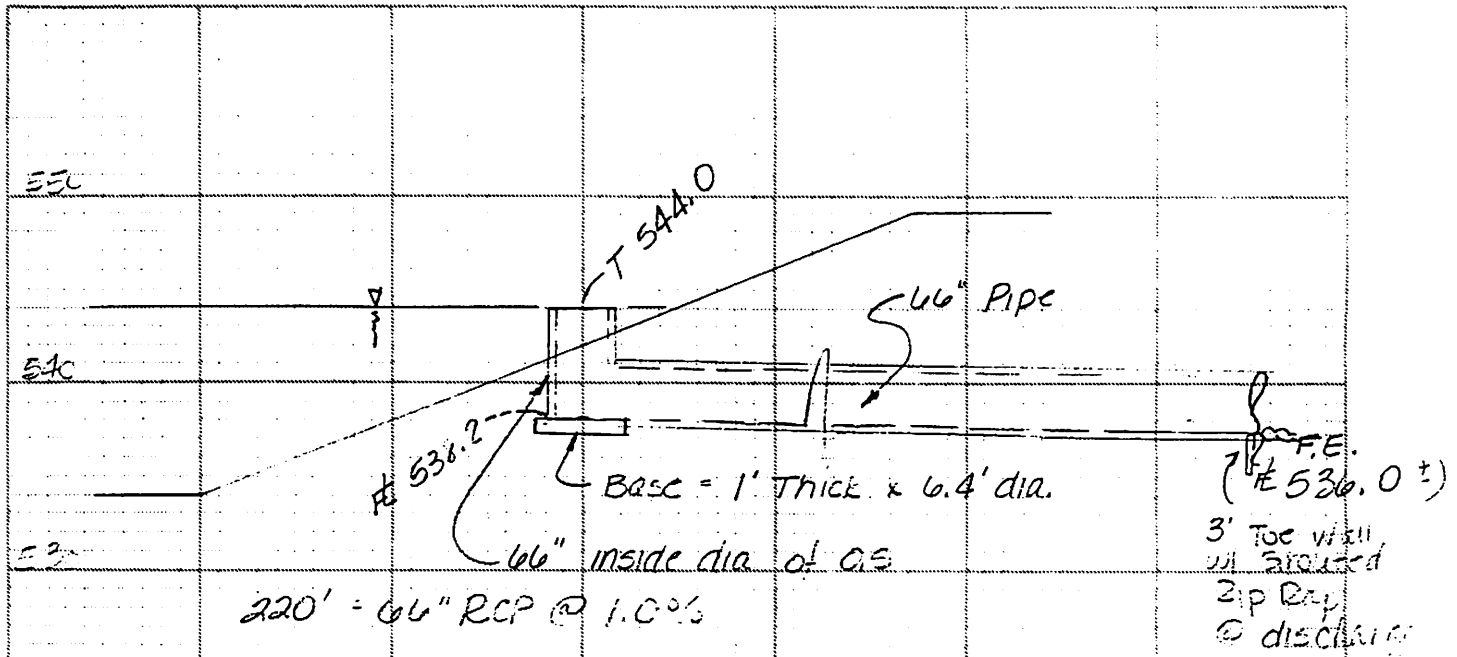
PROJECT NAME #5 OUTFALL STRUCTURE

PROJECT #/JOB ORDER # 29904

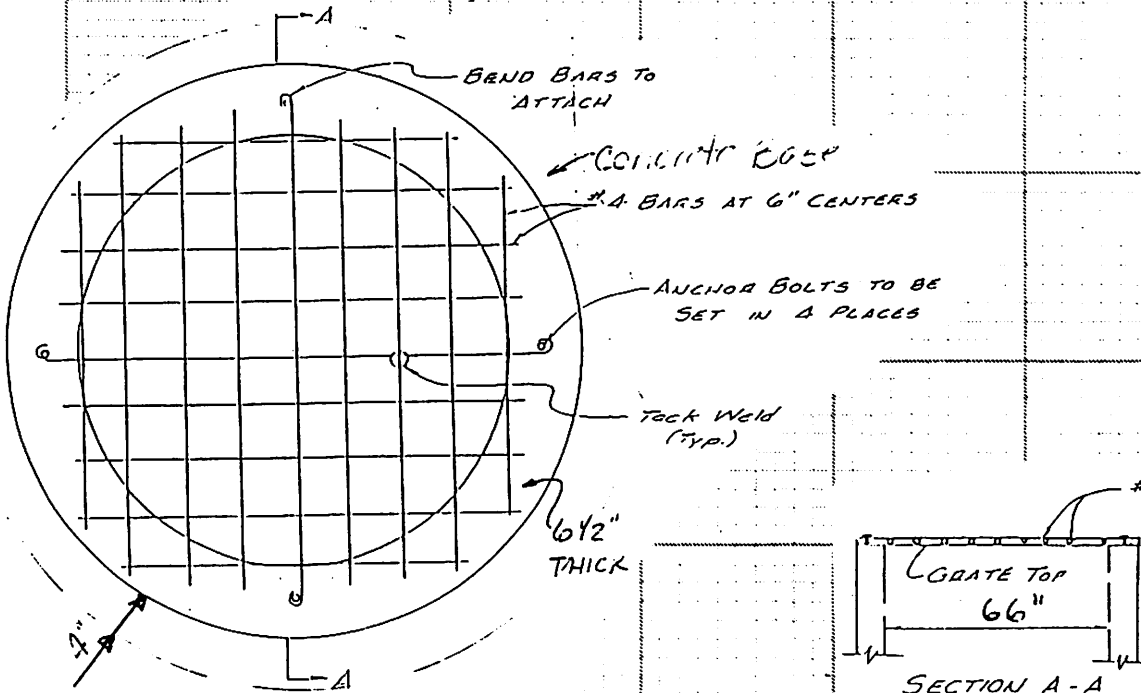
DATE 11-15-91

DESIGNER T. Detz

PAGE 1 of 2



Construct grate top - #4 rebar 6" apart.



TOP DETAIL
OVERFLOW STRUCTURE