

Pickett,
Ray &
Silver, Inc.

349 MID RIVERS DR.
ST. PETERS, MO. 63376

(314) 441-1211
(314) 278-1211

CIVIL ENGINEERS AND LAND SURVEYORS

PROJECT NAME GOLDEN OAKS PL 2
PROJECT #/JOB ORDER # 60-027
DATE 3/4/86 /REV 3/14/86
DESIGNER JTB
PAGE 1 of 5

CALCULATE RUNOFF

FROM USGS - OFFSITE

$$A = 70 \text{ AC } @ 1.7 \text{ CFS/AC} =$$

$$\times 2.4$$

$Q_{15} \quad Q_{100}$

$$119 \text{ CFS} \times 1.39 = 165 \text{ CFS}$$

$$168 \times 1.39 = 234 \text{ cfs}$$

AREA OF DEVELOPMENT = 14 AC

DIFFERENTIAL RUNOFF (14.0 AC) (2.4 CFS/AC - 1.7 CFS/AC) = 9.8 CFS

VOLUME REQUIRED = 9.8 CFS X 1800 SEC = 17,640 CF

PROVIDE REQUIRED VOLUME BY DISPLACEMENT OF
DIRT OUTSIDE NORMAL CHANNEL TO STORE DIFFERENTIAL
OF Q_{15} STORM. (119 CFS).

USING EXIST. CHANNEL C SEC C, & MANNING
EQUATION WATER LEVEL = 552.1

$Q = VA$

$= \frac{1.486 R^{2/3} S^{1/2} (A)}{n}$

$A = 22 \text{ SF}$

$WP = 13.7 \text{ FT}$

$= \frac{1.486 (1.60)^{2/3} (22)^{1/2} \times 22}{.04}$

$R = 22 / 13.7 = 1.60 \text{ FT}$

$S = \frac{552 - 548}{285} = 1.40 \%$

$Q = 134 \text{ CFS}$

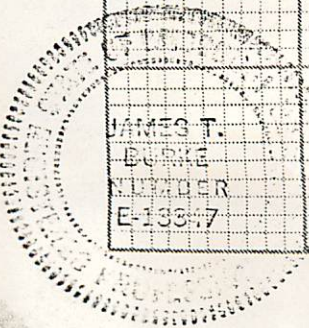
$n = .040$

FROM GRADING PLAN & USING AVERAGE END AREA, THE
VOLUME OF DISPLACEMENT WAS CALCULATED.
THE VOLUME IS 18,086 CF WHICH EXCEEDS REQUIRED.
SEE SECTIONS ON SHEET 3.

PROPOSED DISPLACEMENT SHOWN ON
GRADING PLAN

RECEIVED MAR 2 1 1986

James T. Burke



GOLDEN OAKS

80-027
2015

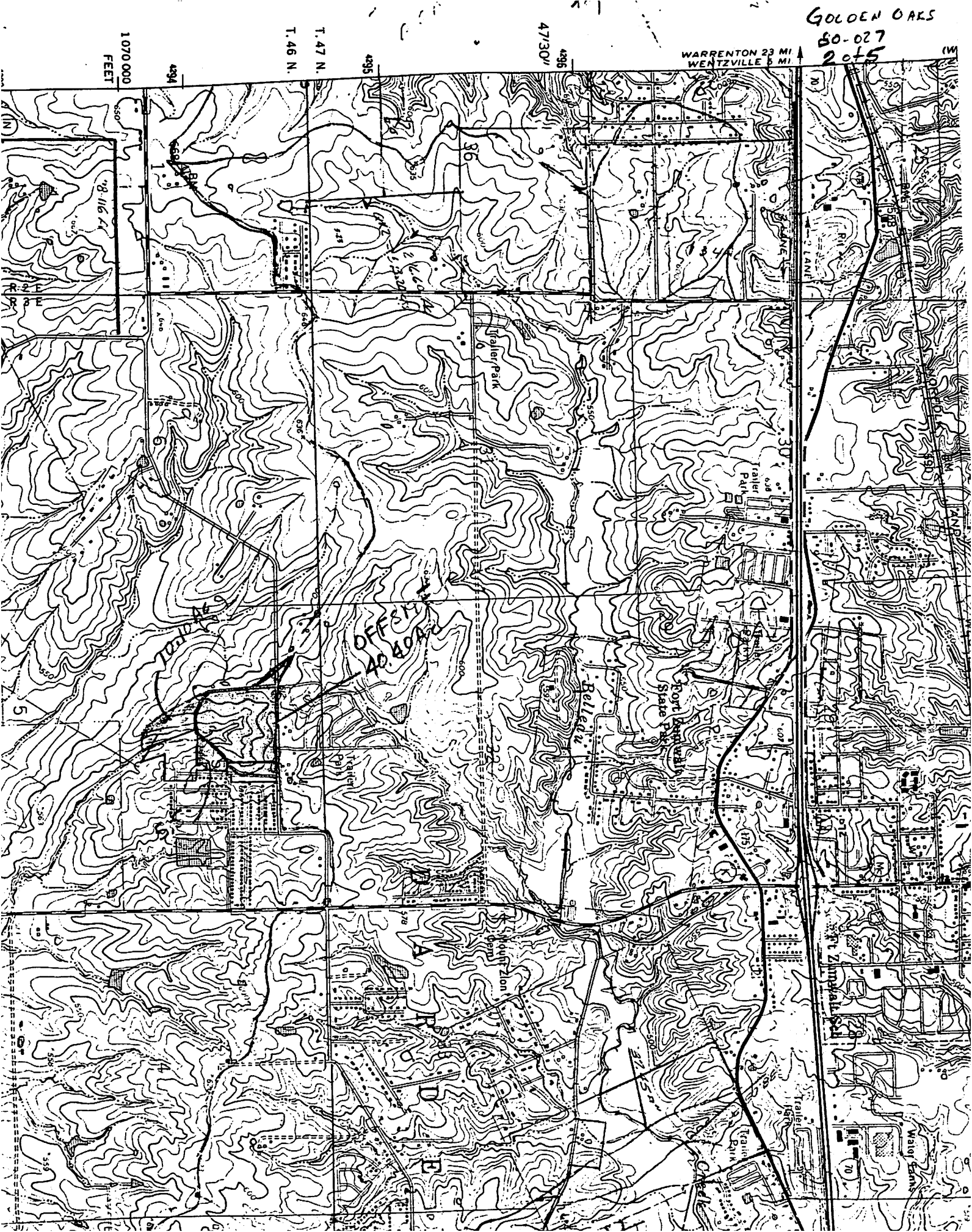
WARRENTON 23 MI
WENTZVILLE 8 MI

4730'

4935

T. 47 N.
T. 46 N.

1070 000
FEET



INSTALL WEIR IN CHANNEL TO HOLD BACK RUNOFF IN
DISPLACED VOLUME

SET WEIR ELEV @ 552.1

SIZE LOW FLOW PIPES

$Q = C A \sqrt{2gh}$ TRY 18 OPENINGS $Q =$ DISCHARGE, CFS

$Q = 0.6 \times 1.787 \times \sqrt{2 \times 32.2 \times 1.35}$

$Q = 10$ CFS PER OPENING

$C =$ CONSTANT 0.6

$A =$ AREA, SQ FT

$g =$ GRAVITY

$h =$ HEAD, FT

USE 8-OPENINGS TO PASS 90 CFS

ASSUMING 20' WEIR CALCULATE HEAD FOR REMAINING Q'S
& REMAINING Q₁₀₀. ASSUME FLOW THRU OPENINGS DOES
NOT INCREASE DUE TO ADDITIONAL HEIGHT.

$Q = C L H^{3/2}$

$(119-80) = 3 \times 20 \times H^{3/2}$

$\frac{59}{60} = H^{3/2}$

$H = 0.74'$

$Q =$ DISCHARGE CFS

$C =$ CONSTANT 3

$L =$ WEIR LENGTH = FT

$H =$ HEAD, FT

NORMAL HIGH WATER =	502.1	
	+ 0.74	
	502.84	
$Q_{100} = 165 \text{ CFS}$		
$(165 - 80) = 3 \times 20 \times H^{3/2}$		
$H^{3/2} = 1.42$		
$H = 1.27'$		
EXTREME HIGH WATER =	502.1	
	+ 1.27	
	503.37	