

**STORM WATER DETENTION
CALCULATIONS AND REPORT
FOR
MALLARD'S LANDING**

Prepared for:

DOMINION CONSTRUCTION, INC
1714 Deer Tracks Trail, 2nd Floor
Town & Country, Missouri 63131

Prepared by:

Colleen E. Young

The Clayton Engineering Company
12755 Olive Boulevard, Suite 100
St. Louis, Missouri 63141-6200

March 26, 1998

JOB: 97454 FB: BY: KRS 04/07/98
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 DOMINION PROPERTIES, INC.
 MALLARD'S LANDING

THE CLAYTON ENGINEERING COMPANY
 12755 OLIVE BLVD - SUITE 100
 ST. LOUIS, MO 63141
 (314) 542-0009

<<<< PIPE HYDRAULICS MODULE >>>>

Description =
 System Number = 1
 Return Period (yr) = 15
 Rainfall Duration (min) = 20
 Runoff Factor Multiplier = 1.00
 Starting HGL Elev. (ft) = 535.00
 Use St. Louis Co./MSD Losses? = Y

STRUCTURES UP/LOW	FLOWLINES UP/LOW	DIAM	LENGTH	n	AREA/PI	Qadd	Qtot/Qfull	CS/RS	Yn/Yc	PARTIAL	FULL	LOSSES		COND	HGL	UPPER STRUCT
										V/Y	V/Vhead	F/C	V/T	UP/LOW	UP/LOW	TOP/FREEBD
CB 1-28	556.29	12	63	0.013	0.29	0.94	0.94	2.00	0.29	1.20	1.20	0.47	0.50	ND	557.08	562.00
MH 1-27	555.03				3.30		5.05	0.07	0.41	1.08	0.02	0.00	0.00	FP	556.11	4.92
MH 1-27	555.03	12	115	0.013	0.23	0.78	1.72	5.00	0.31	2.19	2.19	5.33	0.08	OF	556.11	564.00
MH 1-11	549.28				3.30		7.99	0.23	0.56	1.42	0.07	0.00	0.00	FP	550.70	7.89
CB 1-13	555.23	12	135	0.013	0.04	0.13	0.13	3.00	0.10	0.17	0.17	2.98	0.21	ND	555.54	565.50
CB 1-12	551.18				3.30		6.19	0.00	0.15	1.17	0.00	0.00	0.00	FP	552.35	9.96
CB 1-12	551.18	12	95	0.013	0.64	2.11	2.24	2.00	0.47	2.85	2.85	1.48	0.17	OF	552.35	556.50
MH 1-11	549.28				3.30		5.05	0.39	0.64	1.42	0.13	0.00	0.00	FP	550.70	4.15
MH 1-11	549.28	12	38	0.013	0.23	1.72	3.96	3.15	0.57	5.04	5.04	0.73	0.39	OF	550.70	557.00
MH 1-10	548.08				3.30		6.34	1.23	0.84	1.47	0.39	0.00	0.03	FP	549.55	6.30
MH 1-10	548.08	12	88	0.013	0.23	0.76	4.73	3.15	0.64	6.02	6.02	2.50	0.31	OF	549.55	556.50
MH 1-9	545.31				3.30		6.34	1.75	0.90	1.27	0.56	0.00	0.16	FP	546.58	6.95
MH 1-9	545.31	12	32	0.013	0.00	0.00	4.73	3.15	0.64	6.02	6.02	0.70	0.00	OF	546.58	550.00
MH 1-8	544.30				3.30		6.34	1.75	0.90	1.31	0.56	0.00	0.27	FP	545.61	3.42
CB 1-8	544.30	18	132	0.013	0.62	2.05	6.78	1.00	0.88	3.96	3.84	0.80	0.31	ND	545.61	548.80
CB 1-7	542.98				3.30		10.53	0.41	1.01	1.39	0.23	0.00	0.12	OJ	544.37	3.19
CB 1-7	542.98	18	124	0.013	0.12	0.40	7.18	1.00	0.91	4.06	4.06	0.58	0.17	OC	544.37	553.50
CB 1-6	541.74				3.30		10.53	0.46	1.04	1.85	0.26	0.00	0.04	FP	543.59	9.13

STRUCTURES UP/LOW	FLOWLINES UP/LOW	DIAM	LENGTH	n	AREA/PI	Qadd	Qtot/Qfull	CS/RS	Yn/Yc	PARTIAL	FULL	LOSSES		COND	HGL	UPPER STRUCT.
										V/Y	V/Vhead	F/C	V/T	UP/LOW	UP/LOW	TOP/FREED
CB 1-6 CB 1-5	541.74 540.58	18	116	0.013	0.41 3.30	1.36	8.54 10.53	1.00 0.66	1.03 1.13	4.83 1.63	4.83 0.36	1.03 0.00	0.20 0.15	OF FP	543.59 542.21	548.80 5.21
CB 1-5 CB 1-4	540.58 535.61	18	145	0.013	0.11 3.30	0.36	8.90 19.51	3.43 0.71	0.71 1.15	5.04 3.01	5.04 0.39	3.46 0.00	0.06 0.07	OF FP	542.21 538.62	549.80 7.59
CB 1-4 MH 1-3	535.61 534.24	18	69	0.013	0.20 3.30	0.66	9.56 14.86	1.99 0.82	0.88 1.19	5.41 3.61	5.41 0.45	0.57 0.00	0.12 0.09	FP FP	538.62 537.85	543.80 5.18
CB 1-18 CB 1-17	549.59 546.89	12	90	0.013	0.51 3.30	1.68	1.68 6.19	3.00 0.22	0.36 0.55	2.14 1.09	2.14 0.07	2.61 0.00	0.09 0.00	OF FP	550.68 547.98	557.00 6.32
CB 1-17 CB 1-16	546.89 544.79	12	42	0.013	0.16 3.30	0.51	2.20 7.99	5.00 0.38	0.36 0.63	2.80 1.27	2.80 0.12	1.83 0.00	0.09 0.00	OF FP	547.98 546.06	551.50 3.52
CB 1-16 MH 1-15	544.79 543.24	12	31	0.013	0.35 3.30	1.16	3.36 7.99	5.00 0.88	0.45 0.78	4.28 1.15	4.28 0.28	1.40 0.00	0.27 0.00	OF FP	546.06 544.39	551.50 5.44
MH 1-15 CB 1-14	543.24 534.99	12	165	0.013	0.00 3.30	0.00	3.36 7.99	5.00 0.88	0.45 0.78	4.28 3.40	4.28 0.28	5.85 0.00	0.00 0.15	OF FP	544.39 538.39	549.00 4.61
CB 1-26 MH 1-19	539.00 538.10	12	90	0.013	0.65 3.30	2.16	2.16 3.57	1.00 0.37	0.56 0.63	2.75 1.97	2.75 0.12	0.33 0.00	0.16 0.00	FP FP	540.55 540.07	543.50 2.95
CB 1-25 MH 1-24	552.57 552.19	12	38	0.013	0.55 3.30	1.82	1.82 3.57	1.00 0.26	0.51 0.57	4.56 0.51	2.32 0.08	0.38 0.00	0.43 0.00	OC ND	553.51 552.70	561.00 7.49
MH 1-24 MH 1-23	552.19 551.29	12	90	0.013	0.00 3.30	0.00	1.82 3.57	1.00 0.26	0.51 0.57	2.32 1.06	2.32 0.08	0.35 0.00	0.00 0.00	ND FP	552.70 552.35	562.00 9.30
MH 1-23 CB 1-22	551.29 542.32	12	69	0.013	0.00 3.30	0.00	1.82 12.88	13.00 0.26	0.25 0.57	2.32 1.58	2.32 0.08	8.39 0.00	0.00 0.06	OF FP	552.35 543.90	556.20 3.85
CB 1-22 MH 1-21	542.32 540.92	15	70	0.013	0.99 3.30	3.27	5.10 9.16	2.00 0.62	0.67 0.92	4.16 1.35	4.16 0.27	1.30 0.00	0.32 0.01	OF FP	543.90 542.27	553.00 9.10
MH 1-21 CB 1-20	540.92 538.31	15	87	0.013	0.00 3.30	0.00	5.10 11.22	3.00 0.62	0.59 0.92	4.16 2.49	4.16 0.27	1.37 0.00	0.00 0.10	OF FP	542.27 540.80	555.00 12.73
CB 1-20 MH 1-19	538.31 536.57	15	87	0.013	0.09 3.30	0.28	5.38 9.16	2.00 0.69	0.69 0.94	4.38 3.50	4.38 0.30	0.60 0.00	0.06 0.07	FP FP	540.80 540.07	554.50 13.70

STRUCTURES UP/LOW	FLOWLINES UP/LOW	DIAM	LENGTH	n	AREA/PI	Qadd	Qtot/Qfull	CS/RS	Yn/Yc	PARTIAL	FULL	LOSSES		COND	HGL	UPPER STRUCT.
										V/Y	V/Vhead	F/C	V/T	UP/LOW	UP/LOW	TOP/FREED
MH 1-19	536.57	15	79	0.013	0.00	2.16	7.54	2.00	0.86	6.14	6.14	1.07	0.50	FP	540.07	543.00
CB 1-14	534.99				3.30		9.16	1.36	1.09	3.40	0.59	0.00	0.10	FP	538.39	2.93
CB 1-14	534.99	24	111	0.013	1.06	11.04	14.40	1.00	1.16	4.58	4.58	0.45	0.00	FP	538.39	540.00
MH 1-3	533.88				3.30		22.68	0.40	1.37	3.97	0.33	0.00	0.10	FP	537.85	1.61
MH 1-3	534.24	24	144	0.013	0.24	15.20	24.76	1.25	1.60	7.88	7.88	1.72	0.80	FP	537.85	545.20
CB 1-2	532.44				3.30		25.36	1.19	1.76	2.81	0.96	0.00	0.08	FP	535.25	7.35
CB 1-2	532.44	36	24	0.013	0.32	1.06	25.82	1.00	1.29	3.76	3.65	0.04	0.00	DC	535.25	538.00
FES 1-1	532.20				3.30		66.88	0.15	1.64	2.80	0.21	0.00	0.22	OJ	535.00	2.75

JOB: 97454 FB: BY: CEY 03/25/98
 ID: 002 6:09P

DOMINION PROPERTIES, INC.
 MALLARD'S LANDING

THE CLAYTON ENGINEERING COMPANY
 12755 OLIVE BLVD - SUITE 100
 ST. LOUIS, MO 63141
 (314) 542-0009

<<<< PIPE HYDRAULICS MODULE >>>>

Description =
 System Number = 2
 Return Period (yr) = 15
 Rainfall Duration (min) = 20
 Runoff Factor Multiplier = 1.00
 Starting HGL Elev. (ft) = 535.00
 Use St. Louis Co./MSD Losses? = Y

STRUCTURES UP/LOW	FLOWLINES UP/LOW	DIAM	LENGTH	n	AREA/PI	Qadd	Qtot/Qfull	CS/RS	Yn/Yc	PARTIAL	FULL	LOSSES		COND	HGL	UPPER STRUCT
										V/Y	V/Vhead	F/C	V/T	UP/LOW	UP/LOW	TOP/FREEBD
CB 2-8 CB 2-7	542.74 542.38	12	8	0.013	0.17 3.30	0.57	0.57 7.58	4.50 0.03	0.19 0.31	0.73 1.05	0.73 0.01	0.00 0.00	0.02 0.00	OC FP	543.46 543.43	548.80 5.34
CB 2-7 CB 2-4	542.38 535.72	12	148	0.013	0.23 3.30	0.76	1.33 7.58	4.50 0.14	0.28 0.49	1.69 2.19	1.69 0.04	5.47 0.00	0.05 0.00	OF FP	543.43 537.91	548.50 5.07
CB 2-9 MH 2-5	544.57 539.05	12	69	0.013	0.89 3.30	2.94	2.94 10.10	8.00 0.68	0.37 0.73	3.74 2.97	3.74 0.22	3.55 0.00	0.29 0.00	OF FP	545.86 542.02	551.50 5.64
FES 2-6 MH 2-5	541.00 539.05	18	65	0.013	8.26 1.70	14.04	14.04 18.24	3.00 1.78	0.99 1.38	7.95 2.97	7.95 0.98	1.16 0.00	1.31 0.00	FP FP	544.49 542.02	541.00 -3.49
MH 2-5 CB 2-4	539.05 535.72	18	111	0.013	0.21 3.30	3.64	17.68 18.24	3.00 2.82	1.19 0.00	10.00 2.19	10.00 1.55	3.13 0.00	0.99 0.00	FP FP	542.02 537.91	553.00 10.98
CB 2-4 MH 2-3	535.72 532.57	24	105	0.013	0.91 3.30	4.34	22.02 39.29	3.00 0.94	1.07 1.68	7.01 3.30	7.01 0.76	1.85 0.00	0.00 0.19	OF FP	537.91 535.87	546.00 8.09
MH 2-3 CB 2-2	532.57 532.07	24	50	0.013	0.00 3.30	0.00	22.02 22.68	1.00 0.94	1.59 1.68	7.01 3.27	7.01 0.76	0.47 0.00	0.00 0.06	FP FP	535.87 535.34	539.70 3.83
CB 2-2 FES 2-1	532.07 531.77	30	30	0.013	0.30 3.30	0.99	23.01 41.13	1.00 0.31	1.34 1.63	4.69 3.23	4.69 0.34	0.09 0.00	0.00 0.25	FP FP	535.34 535.00	539.00 3.66

JOB: 97454
ID: 003

FR:

BY: CEY

03/25/98

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DOMINION PROPERTIES, INC.
MALLARD'S LANDING

THE CLAYTON ENGINEERING COMPANY
12755 OLIVE BLVD - SUITE 100
ST. LOUIS, MO 63141
(314) 542-0009

<<<< PIPE HYDRAULICS MODULE >>>>

Description =
System Number = 3
Return Period (yr) = 15
Rainfall Duration (min) = 20
Runoff Factor Multiplier = 1.00
Starting HGL Elev. (ft) = 535.00
Use St. Louis Co./MSD Losses? = Y

STRUCTURES UP/LOW	FLOWLINES UP/LOW	DIAM	LENGTH	n	AREA/PI	Qadd	Qtot/Qfull	CS/RS	Yn/Yc	PARTIAL V/Y	FULL V/Vhead	LOSSES F/C V/T		COND UP/LOW	HGL UP/LOW	UPPER STRUCT TOP/FREEBD
CB 3-5 CB 3-2	534.58 530.78	12	76	0.013	0.06 3.30	0.20	0.20 7.99	5.00 0.00	0.11 0.18	0.25 4.51	0.25 0.00	0.00 0.00	0.00 0.00	OC FP	535.29 535.29	542.50 7.21
CB 3-4 MH 3-3	536.78 535.18	12	80	0.013	0.44 3.30	1.45	1.45 5.05	2.00 0.17	0.37 0.51	1.85 1.10	1.85 0.05	1.50 0.00	0.07 0.00	OF FP	537.85 536.28	542.00 4.15
MH 3-3 CB 3-2	535.18 530.78	12	110	0.013	0.20 3.30	0.67	2.12 7.14	4.00 0.35	0.37 0.62	2.70 4.51	2.70 0.11	0.89 0.00	0.10 0.00	OF FP	536.28 535.29	545.00 8.72
CB 3-2 FES 3-1	530.78 529.50	15	32	0.013	0.52 3.30	1.92	4.04 12.95	4.00 0.39	0.48 0.81	3.29 5.50	3.29 0.17	0.12 0.00	0.15 0.02	FP FP	535.29 535.00	537.50 2.21

LEGEND

UP - At upper end of pipe LW - At lower end of pipe DIAM - Pipe diameter (in) LENGTH - Pipe length (ft) n - Manning's roughness factor AREA - Upper drainage area (ac) PI - Runoff factor (cfs/ac) Qadd - Added flowrate (cfs) Qtot - Total flowrate (cfs) Qfull - Pipe full capacity (cfs) CS - Construction slope of pipe (%) RS - Minimum required slope (%)	Yn - Normal depth (ft) Yc - Critical depth (ft) PARTIAL - Conditions at lower end of pipe FULL - Conditions assuming full pipe flow V - Velocity (fps) Y - Depth (ft) Vhead - Velocity head (ft) LOSSES - Major and minor head losses F - Friction in pipe (ft) C - Curve in pipe (ft) V - Velocities in upper structure (ft) T - Turns in upper structure (ft)	COND - Flow condition code at each end of pipe: FP - full pipe flow OC - open channel flow ND - set to normal depth CD - set to critical depth OJ - open channel flow but hydraulic jump will occur downstream OF - initially set to open channel depth then set to full pipe flow HGL - Hydraulic grade line elevation (ft) TOP - Elevation of top of upper structure (ft) FREEBD - Difference btwn upper HGL and TOP (ft)
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- Notes:
1. Friction losses computed with Manning's formula if full pipe flow or back-calculated if open channel flow (simulating flow profile)
 2. HGL at upper structure includes structure losses calculated with actual inflowing velocities using iterative procedure
 3. Velocity and turn structure loss components only computed for incoming pipes with invert elevations below outlet crown elevation

THE CLAYTON ENGINEERING COMPANY



Project No. 97454 Proj. Name MALLARD'S LANDING
Subject Detention Calculations (2 yr. Frequency)
By CEY Date 3/23/98 Checked _____ Date _____

INFLOW HYDROGRAPH PARAMETERS

Offsite Area	8.26 Ac. @	1.09 c.f.s./Ac.	=	9.00 c.f.s.
Onsite Area	11.76 Ac. @	2.26 c.f.s./Ac.	=	26.58 c.f.s.
TOTAL PROPOSED INFLOW				= 35.58 c.f.s.

UNDETAINED RUNOFF FROM SITE

Developed Onsite Area	1.38 Ac. @	2.26 c.f.s./Ac.	=	3.12 c.f.s.
TOTAL UNDETAINED RUNOFF FROM SITE				= 3.12 c.f.s.

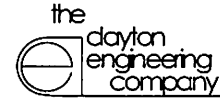
ALLOWABLE OUTFLOW

Offsite Area	8.26 Ac. @	1.09 c.f.s./Ac.	=	9.00 c.f.s.
Onsite Area	11.76 Ac. @	1.09 c.f.s./Ac.	=	12.82 c.f.s.
ALLOWABLE OUTFLOW				= 21.82 c.f.s.
Less Undetained Runoff				= 3.12 c.f.s.
TOTAL ALLOWABLE OUTFLOW				= 18.70 c.f.s.

REQUIRED DETENTION

$$13.14 \text{ Ac.} * (2.26 - 1.09) * 30 \text{ min.} * 60 \text{ sec.} = 27,673 \text{ cu. ft.}$$

THE CLAYTON ENGINEERING COMPANY



Project No. 97454 Proj. Name MALLARD'S LANDING
 Subject Detention Calculations (5 yr. Frequency)
 By CEY Date 3/23/98 Checked _____ Date _____

INFLOW HYDROGRAPH PARAMETERS

Offsite Area	8.26 Ac. @	1.33 c.f.s./Ac.	=	10.99 c.f.s.
Onsite Area	11.76 Ac. @	2.77 c.f.s./Ac.	=	32.58 c.f.s.

TOTAL PROPOSED INFLOW = 43.56 c.f.s.

UNDETAINED RUNOFF FROM SITE

Developed Onsite Area	1.38 Ac. @	2.77 c.f.s./Ac.	=	3.82 c.f.s.
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TOTAL UNDETAINED RUNOFF FROM SITE = 3.82 c.f.s.

ALLOWABLE OUTFLOW

Offsite Area	8.26 Ac. @	1.33 c.f.s./Ac.	=	10.99 c.f.s.
Onsite Area	11.76 Ac. @	1.33 c.f.s./Ac.	=	15.64 c.f.s.

ALLOWABLE OUTFLOW = 26.63 c.f.s.

Less Undetained Runoff = 3.82 c.f.s.

TOTAL ALLOWABLE OUTFLOW = 22.81 c.f.s.

REQUIRED DETENTION

13.14 Ac. * (2.77 - 1.33) * 30 min. * 60 sec. = 34,059 cu. ft.

THE CLAYTON ENGINEERING COMPANY



Project No. 97454 Proj. Name MALLARD'S LANDING
 Subject Detention Calculations (15 yr. Frequency)
 By CEY Date 3/23/98 Checked _____ Date _____

INFLOW HYDROGRAPH PARAMETERS

Offsite Area	8.26 Ac. @	1.87 c.f.s./Ac.	=	15.45 c.f.s.
Onsite Area	11.76 Ac. @	3.30 c.f.s./Ac.	=	38.81 c.f.s.
TOTAL PROPOSED INFLOW				= 54.25 c.f.s.

UNDETAINED RUNOFF FROM SITE

Developed Onsite Area	1.38 Ac. @	3.30 c.f.s./Ac.	=	4.55 c.f.s.
TOTAL UNDETAINED RUNOFF FROM SITE				= 4.55 c.f.s.

ALLOWABLE OUTFLOW

Offsite Area	8.26 Ac. @	1.87 c.f.s./Ac.	=	15.45 c.f.s.
Onsite Area	11.76 Ac. @	1.87 c.f.s./Ac.	=	21.99 c.f.s.
ALLOWABLE OUTFLOW				= 37.44 c.f.s.
Less Undetained Runoff				= 4.55 c.f.s.
TOTAL ALLOWABLE OUTFLOW				= 32.89 c.f.s. ✓

REQUIRED DETENTION

13.14 Ac. * (3.30 - 1.87) * 30 min. * 60 sec. = **33,822 cu. ft.**

THE CLAYTON ENGINEERING COMPANY



Project No. 97454 Proj. Name MALLARD'S LANDING
Subject Detention Calculations (25 yr. Frequency)
By CEY Date 3/23/98 Checked _____ Date _____

INFLOW HYDROGRAPH PARAMETERS

Offsite Area	8.26 Ac. @	2.31 c.f.s./Ac.	=	19.08 c.f.s.
Onsite Area	11.76 Ac. @	4.07 c.f.s./Ac.	=	47.86 c.f.s.

TOTAL PROPOSED INFLOW = 66.94 c.f.s.

UNDETAINED RUNOFF FROM SITE

Developed Onsite Area	1.38 Ac. @	4.07 c.f.s./Ac.	=	5.61 c.f.s.
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TOTAL UNDETAINED RUNOFF FROM SITE = 5.61 c.f.s.

ALLOWABLE OUTFLOW

Offsite Area	8.26 Ac. @	2.31 c.f.s./Ac.	=	19.08 c.f.s.
Onsite Area	11.76 Ac. @	2.31 c.f.s./Ac.	=	27.17 c.f.s.

ALLOWABLE OUTFLOW = 46.25 c.f.s.

Less Undetained Runoff = 5.61 c.f.s.

TOTAL ALLOWABLE OUTFLOW = 40.64 c.f.s.

REQUIRED DETENTION

13.14 Ac. * (4.07 - 2.31) * 30 min. * 60 sec. = 41,628 cu. ft.

THE CLAYTON ENGINEERING COMPANY



Project No. 97454 Proj. Name MALLARD'S LANDING
Subject Detention Calculations (100 yr. Frequency)
By CEY Date 3/23/98 Checked _____ Date _____

INFLOW HYDROGRAPH PARAMETERS

Offsite Area	8.26 Ac. @	2.95 c.f.s./Ac.	=	24.37 c.f.s.
Onsite Area	11.76 Ac. @	5.21 c.f.s./Ac.	=	61.27 c.f.s.

TOTAL PROPOSED INFLOW = 85.64 c.f.s.

THE CLAYTON ENGINEERING COMPANY



Project No. 97454 Proj. Name MALLARD'S LANDING
 Subject Detention Calculations
 By CEY Date 3/25/98 Checked _____ Date _____

SUMMARY

Storm Frequency (yrs)	Allow. Outflow c.f.s.	Required Volume c.f.	Design Outflow c.f.s.	Volume Provided c.f.
2	18.7	27,673	17.82	28,864
5	22.81	34,059	20.04	36,200
15	32.89	33,822	32.34	44,641
25	40.63	41,628	40.64	53,372
100	XX	XX	85.62	61,378

POND-2 Version: 5.17
S/N:

Mallard's Landing
Detention Calculations
Basin Parameters

CALCULATED 03-25-1998 09:22:16
DISK FILE: n:\97454\deten\97454VOL.VOL

Planimeter scale: 1 inch = 1 ft.

Elevation (ft)	Planimeter (sq.in.)	Area (sq.ft)	$A1+A2+\text{sqr}(A1*A2)$ (sq.ft)	* Volume (cubic-ft)	Volume Sum (cubic-ft)
530.00	6,721.00	6,721	0	0	0
531.00	8,016.00	8,016	22,077	7,359	7,359
532.00	9,391.00	9,391	26,083	8,694	16,053
532.50	10,104.00	10,104	29,236	4,873	20,926
533.00	10,834.00	10,834	31,401	5,233	26,160
533.50	11,579.00	11,579	33,613	5,602	31,762
534.00	12,338.00	12,338	35,869	5,978	37,740
534.50	13,112.00	13,112	38,169	6,362	44,102
535.00	13,900.00	13,900	40,512	6,752	50,854
536.00	15,520.00	15,520	44,108	14,703	65,556

* Incremental volume computed by the Conic Method for Reservoir Volumes.

Outlet Structure File: OUTFLOW .STR

POND-2 Version: 5.17

S/N:

Date Executed:

Time Executed:

MALLARD'S LANDING
Detention Calculations

***** COMPOSITE OUTFLOW SUMMARY *****

Elevation (ft)	Q (cfs)	Contributing Structures
530.00	0.0	1
530.50	1.2	1
531.00	4.1	1
531.50	7.6	1
532.00	12.3	2
532.50	14.8	2
533.00	16.9	2
533.50	18.8	2
534.00	20.5	2 +3
534.50	31.6	2 +4 +5
535.00	40.8	2 +4 +5
535.50	39.6	2 +4
536.00	0.0	

Outlet Structure File: OUTFLOW .STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations

Outlet Structure File: n:\97454\deten\OUTFLOW .STR
Planimeter Input File: n:\97454\deten\97454VOL.VOL
Rating Table Output File: n:\97454\deten\POND .PND

Min. Elev.(ft) = 530 Max. Elev.(ft) = 536 Incr.(ft) = .5

Additional elevations (ft) to be included in table:
* * * * *

SYSTEM CONNECTIVITY

Structure	No.	Q Table	Q Table
-----	---	-----	-----
CULVERT-CR	1	->	1
ORIFICE	2	->	2
WEIR-VR	3	->	3
ORIFICE	4	->	4
WEIR-VR	5	->	5

Outflow rating table summary was stored in file:
n:\97454\deten\POND .PND

Outlet Structure File: OUTFLOW .STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations

>>>>> Structure No. 1 <<<<<<
(Input Data) *marsh*

CULVERT-CR
Circular Culvert (With Inlet Control)

E1 elev. (ft)?	530
E2 elev. (ft)?	531.75
Diam. (ft)?	1.75 - 21"
Inv. el. (ft)?	530
Slope (ft/ft)?	.05
T1 ratio?	
T2 ratio?	
K Coeff.?	.0098
M Coeff.?	2
c Coeff.?	.0398
Y Coeff.?	.67
Form 1 or 2?	1
Slope factor?	-.5

Outlet Structure File: OUTFLOW .STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations

>>>>> Structure No. 2 <<<<<<
(Input Data)

ORIFICE
Orifice - Based on Area and Datum Elevation

E1 elev.(ft)?	531.75
E2 elev.(ft)?	536.00
Orifice coeff.?	0.6
Invert elev.(ft)?	530
Datum elev.(ft) ?	530.875
Orifice area (sq ft)?	2.405

Outlet Structure File: OUTFLOW .STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations

>>>>> Structure No. 3 <<<<<<
(Input Data)

WEIR-VR
Weir - Vertical Rectangular

E1 elev.(ft)?	533.75
E2 elev.(ft)?	534.25
Weir coefficient?	3.0
Weir elev.(ft)?	534.00
Length (ft)?	1.00
Contracted/Suppressed (C/S)?	S

Outlet Structure File: OUTFLOW .STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations

>>>>> Structure No. 4 <<<<<<
(Input Data)

ORIFICE
Orifice - Based on Area and Datum Elevation

E1 elev.(ft)?	534.25
E2 elev.(ft)?	536
Orifice coeff.?	3
Invert elev.(ft)?	533.75
Datum elev.(ft) ?	534.00
Orifice area (sq ft)?	0.50

Outlet Structure File: OUTFLOW .STR

POND-2 Version: 5.17

S/N:

Date Executed:

Time Executed:

MALLARD'S LANDING
Detention Calculations

>>>>> Structure No. 5 / <<<<<<
(Input Data)

WEIR-VR
Weir - Vertical Rectangular

E1 elev. (ft)?	534.25
E2 elev. (ft)?	535.25
Weir coefficient?	3
Weir elev. (ft)?	534.25
Length (ft)?	2.67
Contracted/Suppressed (C/S)?	S

Outlet Structure File: OUTFLOW .STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations

Outflow Rating Table for Structure #1
CULVERT-CR Circular Culvert (With Inlet Control)

***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation	Messages
530.00	0.0	No headwater	
530.50	1.2	Equ.1: HW =.5	dc=.398 Ac=.412
531.00	4.1	Equ.1: HW =1.0	dc=.74 Ac=.968
531.50	7.6	Equ.1: HW =1.5	dc=1.023 Ac=1.46
532.00	0.0	E = or > E2=531.75	
532.50	0.0	E = or > E2=531.75	
533.00	0.0	E = or > E2=531.75	
533.50	0.0	E = or > E2=531.75	
534.00	0.0	E = or > E2=531.75	
534.50	0.0	E = or > E2=531.75	
535.00	0.0	E = or > E2=531.75	
535.50	0.0	E = or > E2=531.75	
536.00	0.0	E = or > E2=531.75	

Used Unsubmerged Equ. Form (1) for elev. less than 531.99 ft
Used Submerged Equation for elevations greater than 532.24 ft
HW=Headwater (ft) dc=Critical depth (ft) Ac=Area (sq.ft) at dc

Transition flows interpolated from the following values:
E1=531.99 ft; Q1=11.14 cfs; Dc=1.24 ft; E2=532.24 ft; Q2=12.73 cfs

Outlet Structure File: OUTFLOW .STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations

Outflow Rating Table for Structure #2
ORIFICE Orifice - Based on Area and Datum Elevation

Elevation (ft)	Q (cfs)	Computation Messages
530.00	0.0	E < E1=531.75
530.50	0.0	E < E1=531.75
531.00	0.0	E < E1=531.75
531.50	0.0	E < E1=531.75
532.00	12.3	H =1.125
532.50	14.8	H =1.625
533.00	16.9	H =2.125
533.50	18.8	H =2.625
534.00	20.5	H =3.125
534.50	22.0	H =3.625
535.00	23.5	H =4.125
535.50	24.9	H =4.625
536.00	0.0	E = or > E2=536.00

C = .6 A = 2.405 sq.ft.
H (ft) = Table elev. - Datum elev. (530.875 ft)
Q (cfs) = C * A * sqr(2g * H)

Outlet Structure File: OUTFLOW .STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations

Outflow Rating Table for Structure #3
WEIR-VR Weir - Vertical Rectangular

***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation Messages
530.00	0.0	E < Inv.El. = 534
530.50	0.0	E < Inv.El. = 534
531.00	0.0	E < Inv.El. = 534
531.50	0.0	E < Inv.El. = 534
532.00	0.0	E < Inv.El. = 534
532.50	0.0	E < Inv.El. = 534
533.00	0.0	E < Inv.El. = 534
533.50	0.0	E < Inv.El. = 534
534.00	0.0	H = 0.0
534.50	0.0	E = or > E2=534.25
535.00	0.0	E = or > E2=534.25
535.50	0.0	E = or > E2=534.25
536.00	0.0	E = or > E2=534.25

C = 3 L (ft) = 1
H (ft) = Table elev. - Invert elev. (534 ft)
Q (cfs) = C * L * (H**1.5) -- Suppressed Weir

Outlet Structure File: OUTFLOW .STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations

Outflow Rating Table for Structure #4
ORIFICE Orifice - Based on Area and Datum Elevation

Elevation (ft)	Q (cfs)	Computation Messages
530.00	0.0	E < E1=534.25
530.50	0.0	E < E1=534.25
531.00	0.0	E < E1=534.25
531.50	0.0	E < E1=534.25
532.00	0.0	E < E1=534.25
532.50	0.0	E < E1=534.25
533.00	0.0	E < E1=534.25
533.50	0.0	E < E1=534.25
534.00	0.0	E < E1=534.25
534.50	8.5	H =.5
535.00	12.0	H =1.0
535.50	14.7	H =1.5
536.00	0.0	E = or > E2=536

C = 3 A = .5 sq.ft.

H (ft) = Table elev. - Datum elev. (534 ft)

Q (cfs) = C * A * sqr(2g * H)

Outlet Structure File: OUTFLOW .STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations

Outflow Rating Table for Structure #5
WEIR-VR Weir - Vertical Rectangular

***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation Messages
530.00	0.0	E < Inv.El.= 534.25
530.50	0.0	E < Inv.El.= 534.25
531.00	0.0	E < Inv.El.= 534.25
531.50	0.0	E < Inv.El.= 534.25
532.00	0.0	E < Inv.El.= 534.25
532.50	0.0	E < Inv.El.= 534.25
533.00	0.0	E < Inv.El.= 534.25
533.50	0.0	E < Inv.El.= 534.25
534.00	0.0	E < Inv.El.= 534.25
534.50	1.0	H =.25
535.00	5.2	H =.750
535.50	0.0	E = or > E2=535.25
536.00	0.0	E = or > E2=535.25

C = 3 L (ft) = 2.67

H (ft) = Table elev. - Invert elev. (534.25 ft)

Q (cfs) = C * L * (H**1.5) -- Suppressed Weir.

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*****
*
*      MALLARD'S LANDING      *
*  Detention Calculations    *
*      2 Year Storm          *
*
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*
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Inflow Hydrograph: n:\97454\deten\002-IN .HYD
 Rating Table file: n:\97454\deten\POND .PND

----INITIAL CONDITIONS----
 Elevation = 530.00 ft
 Outflow = 0.00 cfs
 Storage = 0 cu-ft

GIVEN POND DATA

ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (cu-ft)
530.00	0.0	0
530.50	1.2	3,517
531.00	4.1	7,359
531.50	7.6	11,534
532.00	12.3	16,054
532.50	14.8	20,926
533.00	16.9	26,160
533.50	18.8	31,762
533.75	19.6	34,703
534.00	20.5	37,740
534.25	27.3	40,872
534.50	31.6	44,101
535.00	40.8	50,854
535.50	50.8	58,002

INTERMEDIATE ROUTING
 COMPUTATIONS

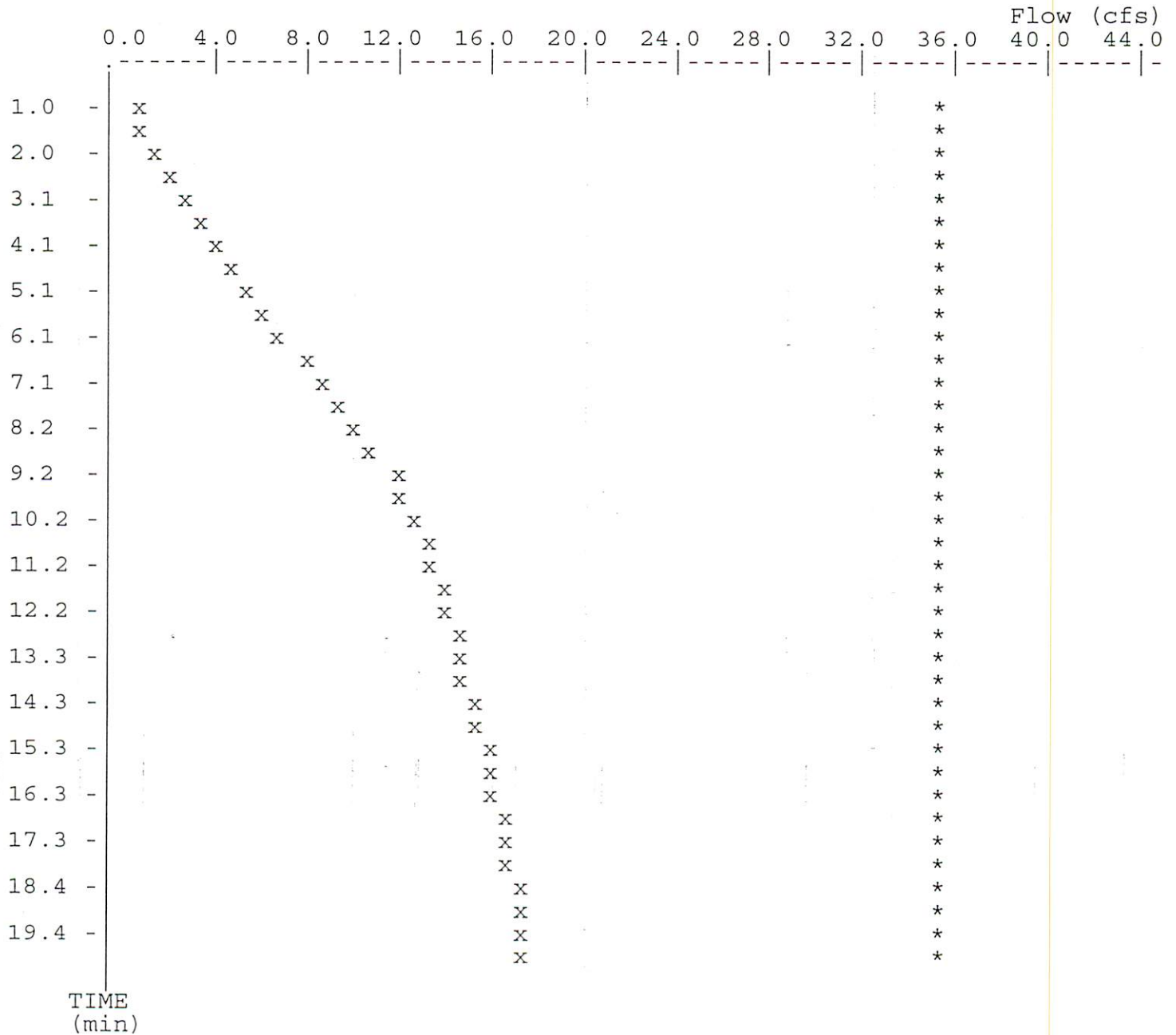
2S/t (cfs)	2S/t + 0 (cfs)
0.0	0.0
117.2	118.4
245.3	249.4
384.5	392.1
535.1	547.4
697.5	712.3
872.0	888.9
1058.7	1077.5
1156.8	1176.4
1258.0	1278.5
1362.4	1389.7
1470.0	1501.6
1695.1	1735.9
1933.4	1984.2

Time increment (t) = 1.0 min.

Pond File: n:\97454\deten\POND .PND
 Inflow Hydrograph: n:\97454\deten\002-IN .HYD
 Outflow Hydrograph: n:\97454\deten\OUT .HYD

EXECUTED: 03-24-1998
 17:39:41

Peak Inflow = 35.58 cfs
 Peak Outflow = 17.82 cfs
 Peak Elevation = 533.24 ft



x File: n:\97454\deten\002-IN .HYD Qmax = 17.8 cfs
 * File: n:\97454\deten\OUT .HYD Qmax = 35.6 cfs

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*****
*
* MALLARD'S LANDING *
* Detention Calculations *
* 5 Year Storm *
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Inflow Hydrograph: n:\97454\deten\005-IN .HYD
 Rating Table file: n:\97454\deten\POND .PND

----INITIAL CONDITIONS----
 Elevation = 530.00 ft
 Outflow = 0.00 cfs
 Storage = 0 cu-ft

GIVEN POND DATA

INTERMEDIATE ROUTING
 COMPUTATIONS

ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (cu-ft)	2S/t (cfs)	2S/t + 0 (cfs)
530.00	0.0	0	0.0	0.0
530.50	1.2	3,517	117.2	118.4
531.00	4.1	7,359	245.3	249.4
531.50	7.6	11,534	384.5	392.1
532.00	12.3	16,054	535.1	547.4
532.50	14.8	20,926	697.5	712.3
533.00	16.9	26,160	872.0	888.9
533.50	18.8	31,762	1058.7	1077.5
533.75	19.6	34,703	1156.8	1176.4
534.00	20.5	37,740	1258.0	1278.5
534.25	27.3	40,872	1362.4	1389.7
534.50	31.6	44,101	1470.0	1501.6
535.00	40.8	50,854	1695.1	1735.9
535.50	50.8	58,002	1933.4	1984.2

Time increment (t) = 1.0 min.

Pond File: n:\97454\deten\POND .PND
 Inflow Hydrograph: n:\97454\deten\005-IN .HYD
 Outflow Hydrograph: n:\97454\deten\OUT .HYD

INFLOW HYDROGRAPH

ROUTING COMPUTATIONS

TIME (min)	INFLOW (cfs)	I1+I2 (cfs)	2S/t - 0 (cfs)	2S/t + 0 (cfs)	OUTFLOW (cfs)	ELEVATION (ft)
0.0	0.00	----	0.0	0.0	0.00	530.00
1.0	43.56	43.6	42.7	43.6	0.44	530.18
2.0	43.56	87.1	126.9	129.8	1.45	530.54
3.0	43.56	87.1	207.4	214.0	3.32	530.86
4.0	43.56	87.1	284.1	294.5	5.21	531.16
5.0	43.56	87.1	357.0	371.2	7.09	531.43
6.0	43.56	87.1	425.8	444.2	9.18	531.67
7.0	43.56	87.1	490.4	512.9	11.26	531.89
8.0	43.56	87.1	552.0	577.5	12.76	532.09
9.0	43.56	87.1	611.8	639.1	13.69	532.28
10.0	43.56	87.1	669.7	698.9	14.60	532.46
11.0	43.56	87.1	726.1	756.8	15.33	532.63
12.0	43.56	87.1	781.3	813.3	16.00	532.79
13.0	43.56	87.1	835.1	868.4	16.66	532.94
14.0	43.56	87.1	887.7	922.2	17.24	533.09
15.0	43.56	87.1	939.3	974.8	17.77	533.23
16.0	43.56	87.1	989.9	1026.4	18.29	533.36
17.0	43.56	87.1	1039.4	1077.0	18.79	533.50
18.0	43.56	87.1	1088.1	1126.5	19.20	533.62
19.0	43.56	87.1	1136.1	1175.2	19.59	533.75
20.0	43.56	87.1	1183.2	1223.2	20.01	533.86
21.0	0.00	43.6	1186.6	1226.7	20.04	533.87
22.0	0.00	0.0	1147.2	1186.6	19.69	533.78
23.0	0.00	0.0	1108.5	1147.2	19.36	533.68
24.0	0.00	0.0	1070.4	1108.5	19.05	533.58
25.0	0.00	0.0	1033.0	1070.4	18.73	533.48
26.0	0.00	0.0	996.3	1033.0	18.35	533.38
27.0	0.00	0.0	960.3	996.3	17.98	533.28
28.0	0.00	0.0	925.1	960.3	17.62	533.19
29.0	0.00	0.0	890.5	925.1	17.26	533.10
30.0	0.00	0.0	856.7	890.5	16.92	533.00
31.0	0.00	0.0	823.7	856.7	16.52	532.91
32.0	0.00	0.0	791.4	823.7	16.12	532.82
33.0	0.00	0.0	759.9	791.4	15.74	532.72
34.0	0.00	0.0	729.2	759.9	15.37	532.63
35.0	0.00	0.0	699.2	729.2	15.00	532.55
36.0	0.00	0.0	670.0	699.2	14.60	532.46
37.0	0.00	0.0	641.7	670.0	14.16	532.37
38.0	0.00	0.0	614.2	641.7	13.73	532.29
39.0	0.00	0.0	587.6	614.2	13.31	532.20
40.0	0.00	0.0	561.8	587.6	12.91	532.12

***** SUMMARY OF ROUTING COMPUTATIONS *****

Pond File: n:\97454\deten\POND .PND
Inflow Hydrograph: n:\97454\deten\005-IN .HYD
Outflow Hydrograph: n:\97454\deten\OUT .HYD

Starting Pond W.S. Elevation = 530.00 ft

***** Summary of Peak Outflow and Peak Elevation *****

Peak Inflow = 43.56 cfs
Peak Outflow = 20.04 cfs
Peak Elevation = 533.87 ft

***** Summary of Approximate Peak Storage *****

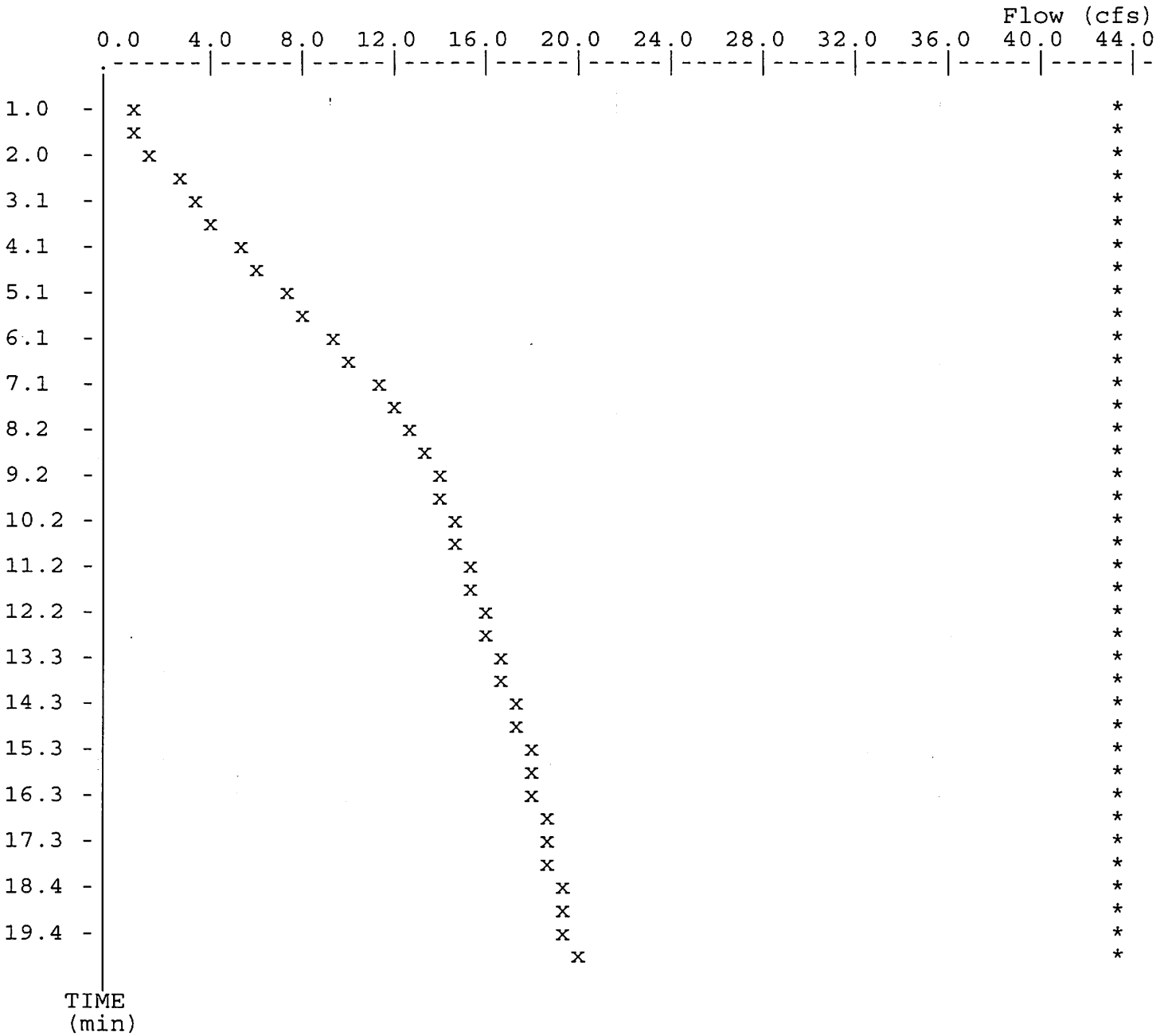
Initial Storage = 0 cu-ft
Peak Storage From Storm = 36,200 cu-ft

Total Storage in Pond = 36,200 cu-ft

Pond File: n:\97454\deten\POND .PND
 Inflow Hydrograph: n:\97454\deten\005-IN .HYD
 Outflow Hydrograph: n:\97454\deten\OUT .HYD

EXECUTED: 03-24-1998
 17:40:31

Peak Inflow = 43.56 cfs
 Peak Outflow = 20.04 cfs
 Peak Elevation = 533.87 ft



x File: n:\97454\deten\005-IN .HYD Qmax = 20.0 cfs
 * File: n:\97454\deten\OUT .HYD Qmax = 43.6 cfs


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*****
*
*      MALLARD'S LANDING      *
*  Detention Calculations    *
*      15 Year Storm         *
*
*
*
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Inflow Hydrograph: n:\97454\deten\015-IN .HYD
 Rating Table file: n:\97454\deten\POND .PND

----INITIAL CONDITIONS----
 Elevation = 530.00 ft
 Outflow = 0.00 cfs
 Storage = 0 cu-ft

GIVEN POND DATA

INTERMEDIATE ROUTING
 COMPUTATIONS

ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (cu-ft)	2S/t (cfs)	2S/t + 0 (cfs)
530.00	0.0	0	0.0	0.0
530.50	1.2	3,517	117.2	118.4
531.00	4.1	7,359	245.3	249.4
531.50	7.6	11,534	384.5	392.1
532.00	12.3	16,054	535.1	547.4
532.50	14.8	20,926	697.5	712.3
533.00	16.9	26,160	872.0	888.9
533.50	18.8	31,762	1058.7	1077.5
533.75	19.6	34,703	1156.8	1176.4
534.00	20.5	37,740	1258.0	1278.5
534.25	27.3	40,872	1362.4	1389.7
534.50	31.6	44,101	1470.0	1501.6
535.00	40.8	50,854	1695.1	1735.9
535.50	50.8	58,002	1933.4	1984.2

Time increment (t) = 1.0 min.

Pond File: n:\97454\deten\POND .PND
 Inflow Hydrograph: n:\97454\deten\015-IN .HYD
 Outflow Hydrograph: n:\97454\deten\OUT .HYD

INFLOW HYDROGRAPH

ROUTING COMPUTATIONS

TIME (min)	INFLOW (cfs)	I1+I2 (cfs)	2S/t - 0 (cfs)	2S/t + 0 (cfs)	OUTFLOW (cfs)	ELEVATION (ft)
0.0	0.00	-----	0.0	0.0	0.00	530.00
1.0	54.25	54.3	53.2	54.3	0.55	530.23
2.0	54.25	108.5	157.3	161.7	2.16	530.66
3.0	54.25	108.5	256.8	265.8	4.50	531.06
4.0	54.25	108.5	351.4	365.3	6.94	531.41
5.0	54.25	108.5	440.6	459.9	9.65	531.72
6.0	54.25	108.5	524.5	549.1	12.33	532.01
7.0	54.25	108.5	605.8	633.0	13.60	532.26
8.0	54.25	108.5	684.6	714.3	14.82	532.51
9.0	54.25	108.5	761.6	793.1	15.76	532.73
10.0	54.25	108.5	836.8	870.1	16.68	532.95
11.0	54.25	108.5	910.3	945.3	17.47	533.15
12.0	54.25	108.5	982.4	1018.8	18.21	533.34
13.0	54.25	108.5	1053.1	1090.9	18.91	533.53
14.0	54.25	108.5	1122.6	1161.6	19.48	533.71
15.0	54.25	108.5	1191.0	1231.1	20.08	533.88
16.0	54.25	108.5	1255.9	1299.5	21.78	534.05
17.0	54.25	108.5	1312.9	1364.4	25.75	534.19
18.0	54.25	108.5	1364.4	1421.4	28.52	534.32
19.0	54.25	108.5	1411.9	1472.9	30.49	534.44
20.0	54.25	108.5	1455.7	1520.4	32.34	534.54
21.0	0.00	54.3	1446.1	1510.0	31.93	534.52
22.0	0.00	0.0	1387.2	1446.1	29.47	534.38
23.0	0.00	0.0	1332.9	1387.2	27.14	534.24
24.0	0.00	0.0	1285.2	1332.9	23.83	534.12
25.0	0.00	0.0	1243.4	1285.2	20.91	534.02
26.0	0.00	0.0	1203.0	1243.4	20.19	533.91
27.0	0.00	0.0	1163.4	1203.0	19.83	533.82
28.0	0.00	0.0	1124.4	1163.4	19.49	533.72
29.0	0.00	0.0	1086.0	1124.4	19.18	533.62
30.0	0.00	0.0	1048.3	1086.0	18.87	533.52
31.0	0.00	0.0	1011.3	1048.3	18.51	533.42
32.0	0.00	0.0	975.0	1011.3	18.13	533.32
33.0	0.00	0.0	939.5	975.0	17.77	533.23
34.0	0.00	0.0	904.6	939.5	17.41	533.13
35.0	0.00	0.0	870.5	904.6	17.06	533.04
36.0	0.00	0.0	837.2	870.5	16.68	532.95
37.0	0.00	0.0	804.6	837.2	16.28	532.85
38.0	0.00	0.0	772.8	804.6	15.90	532.76
39.0	0.00	0.0	741.8	772.8	15.52	532.67
40.0	0.00	0.0	711.5	741.8	15.15	532.58

***** SUMMARY OF ROUTING COMPUTATIONS *****

Pond File: n:\97454\deten\POND .PND
Inflow Hydrograph: n:\97454\deten\015-IN .HYD
Outflow Hydrograph: n:\97454\deten\OUT .HYD

Starting Pond W.S. Elevation = 530.00 ft

***** Summary of Peak Outflow and Peak Elevation *****

Peak Inflow = 54.25 cfs
Peak Outflow = 32.34 cfs
Peak Elevation = 534.54 ft

***** Summary of Approximate Peak Storage *****

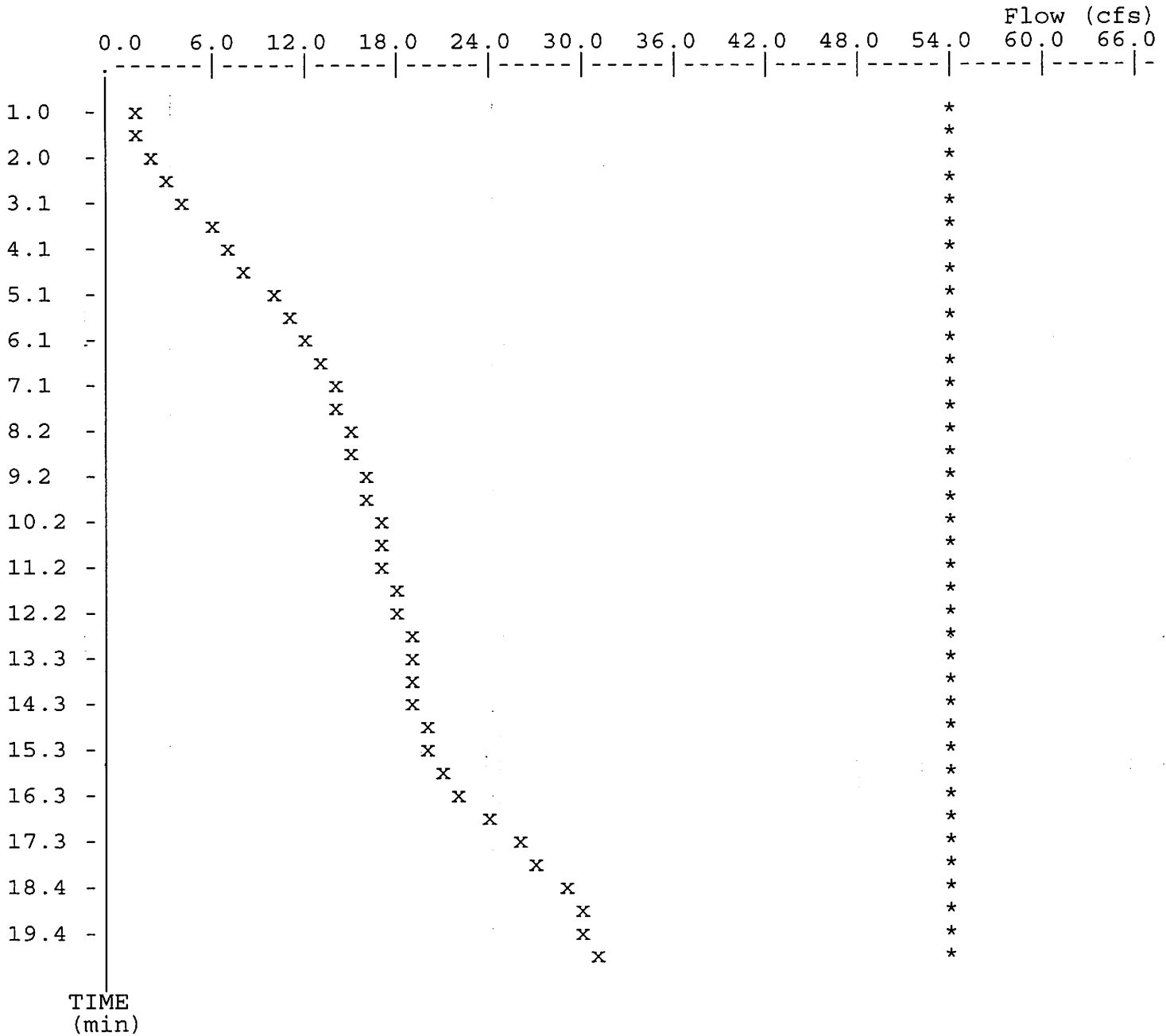
Initial Storage = 0 cu-ft
Peak Storage From Storm = 44,641 cu-ft

Total Storage in Pond = 44,641 cu-ft

Pond File: n:\97454\deten\POND .PND
 Inflow Hydrograph: n:\97454\deten\015-IN .HYD
 Outflow Hydrograph: n:\97454\deten\OUT .HYD

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 17:41:11

Peak Inflow = 54.25 cfs
 Peak Outflow = 32.34 cfs
 Peak Elevation = 534.54 ft



x File: n:\97454\deten\015-IN .HYD Qmax = 32.3 cfs
 * File: n:\97454\deten\OUT .HYD Qmax = 54.3 cfs

```

*****
*
*           MALLARD'S LANDING           *
*   Detention Calculations             *
*           25 Year Storm               *
*                                       *
*                                       *
*****
  
```

Inflow Hydrograph: n:\97454\deten\025-IN .HYD
 Rating Table file: n:\97454\deten\POND .PND

----INITIAL CONDITIONS----
 Elevation = 530.00 ft
 Outflow = 0.00 cfs
 Storage = 0 cu-ft

GIVEN POND DATA

INTERMEDIATE ROUTING
 COMPUTATIONS

ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (cu-ft)	2S/t (cfs)	2S/t + 0 (cfs)
530.00	0.0	0	0.0	0.0
530.50	1.2	3,517	117.2	118.4
531.00	4.1	7,359	245.3	249.4
531.50	7.6	11,534	384.5	392.1
532.00	12.3	16,054	535.1	547.4
532.50	14.8	20,926	697.5	712.3
533.00	16.9	26,160	872.0	888.9
533.50	18.8	31,762	1058.7	1077.5
533.75	19.6	34,703	1156.8	1176.4
534.00	20.5	37,740	1258.0	1278.5
534.25	27.3	40,872	1362.4	1389.7
534.50	31.6	44,101	1470.0	1501.6
535.00	40.8	50,854	1695.1	1735.9
535.50	39.6	58,002	1933.4	1973.0

Time increment (t) = 1.0 min.

Pond File: n:\97454\deten\POND .PND
 Inflow Hydrograph: n:\97454\deten\025-IN .HYD
 Outflow Hydrograph: n:\97454\deten\OUT .HYD

INFLOW HYDROGRAPH

ROUTING COMPUTATIONS

TIME (min)	INFLOW (cfs)	I1+I2 (cfs)	2S/t - 0 (cfs)	2S/t + 0 (cfs)	OUTFLOW (cfs)	ELEVATION (ft)
0.0	0.00	----	0.0	0.0	0.00	530.00
1.0	66.94	66.9	65.6	66.9	0.68	530.28
2.0	66.94	133.9	193.5	199.5	2.99	530.81
3.0	66.94	133.9	315.3	327.4	6.01	531.27
4.0	66.94	133.9	430.6	449.2	9.33	531.68
5.0	66.94	133.9	539.3	564.4	12.56	532.05
6.0	66.94	133.9	644.8	673.2	14.21	532.38
7.0	66.94	133.9	747.5	778.7	15.59	532.69
8.0	66.94	133.9	847.7	881.4	16.81	532.98
9.0	66.94	133.9	946.0	981.6	17.83	533.25
10.0	66.94	133.9	1042.2	1079.8	18.82	533.51
11.0	66.94	133.9	1136.9	1176.1	19.60	533.75
12.0	66.94	133.9	1229.9	1270.8	20.43	533.98
13.0	66.94	133.9	1312.4	1363.8	25.71	534.19
14.0	66.94	133.9	1387.3	1446.2	29.47	534.38
15.0	66.94	133.9	1456.4	1521.2	32.37	534.54
16.0	66.94	133.9	1520.2	1590.3	35.08	534.69
17.0	66.94	133.9	1578.9	1654.0	37.58	534.83
18.0	66.94	133.9	1633.0	1712.7	39.89	534.95
19.0	66.94	133.9	1685.6	1766.8	40.64	535.07
20.0	66.94	133.9	1738.7	1819.4	40.38	535.18
21.0	0.00	66.9	1724.7	1805.6	40.45	535.15
22.0	0.00	0.0	1644.0	1724.7	40.36	534.98
23.0	0.00	0.0	1569.6	1644.0	37.19	534.80
24.0	0.00	0.0	1501.1	1569.6	34.27	534.65
25.0	0.00	0.0	1437.9	1501.1	31.58	534.50
26.0	0.00	0.0	1379.6	1437.9	29.15	534.36
27.0	0.00	0.0	1326.3	1379.6	26.68	534.23
28.0	0.00	0.0	1279.4	1326.3	23.42	534.11
29.0	0.00	0.0	1238.3	1279.4	20.56	534.00
30.0	0.00	0.0	1198.0	1238.3	20.15	533.90
31.0	0.00	0.0	1158.4	1198.0	19.79	533.80
32.0	0.00	0.0	1119.5	1158.4	19.45	533.70
33.0	0.00	0.0	1081.2	1119.5	19.14	533.61
34.0	0.00	0.0	1043.6	1081.2	18.83	533.51
35.0	0.00	0.0	1006.7	1043.6	18.46	533.41
36.0	0.00	0.0	970.5	1006.7	18.09	533.31
37.0	0.00	0.0	935.0	970.5	17.72	533.22
38.0	0.00	0.0	900.3	935.0	17.36	533.12
39.0	0.00	0.0	866.3	900.3	17.02	533.03
40.0	0.00	0.0	833.0	866.3	16.63	532.94

***** SUMMARY OF ROUTING COMPUTATIONS *****

Pond File: n:\97454\deten\POND .PND
Inflow Hydrograph: n:\97454\deten\025-IN .HYD
Outflow Hydrograph: n:\97454\deten\OUT .HYD

Starting Pond W.S. Elevation = 530.00 ft

***** Summary of Peak Outflow and Peak Elevation *****

Peak Inflow = 66.94 cfs
Peak Outflow = 40.64 cfs
Peak Elevation = 535.18 ft

***** Summary of Approximate Peak Storage *****

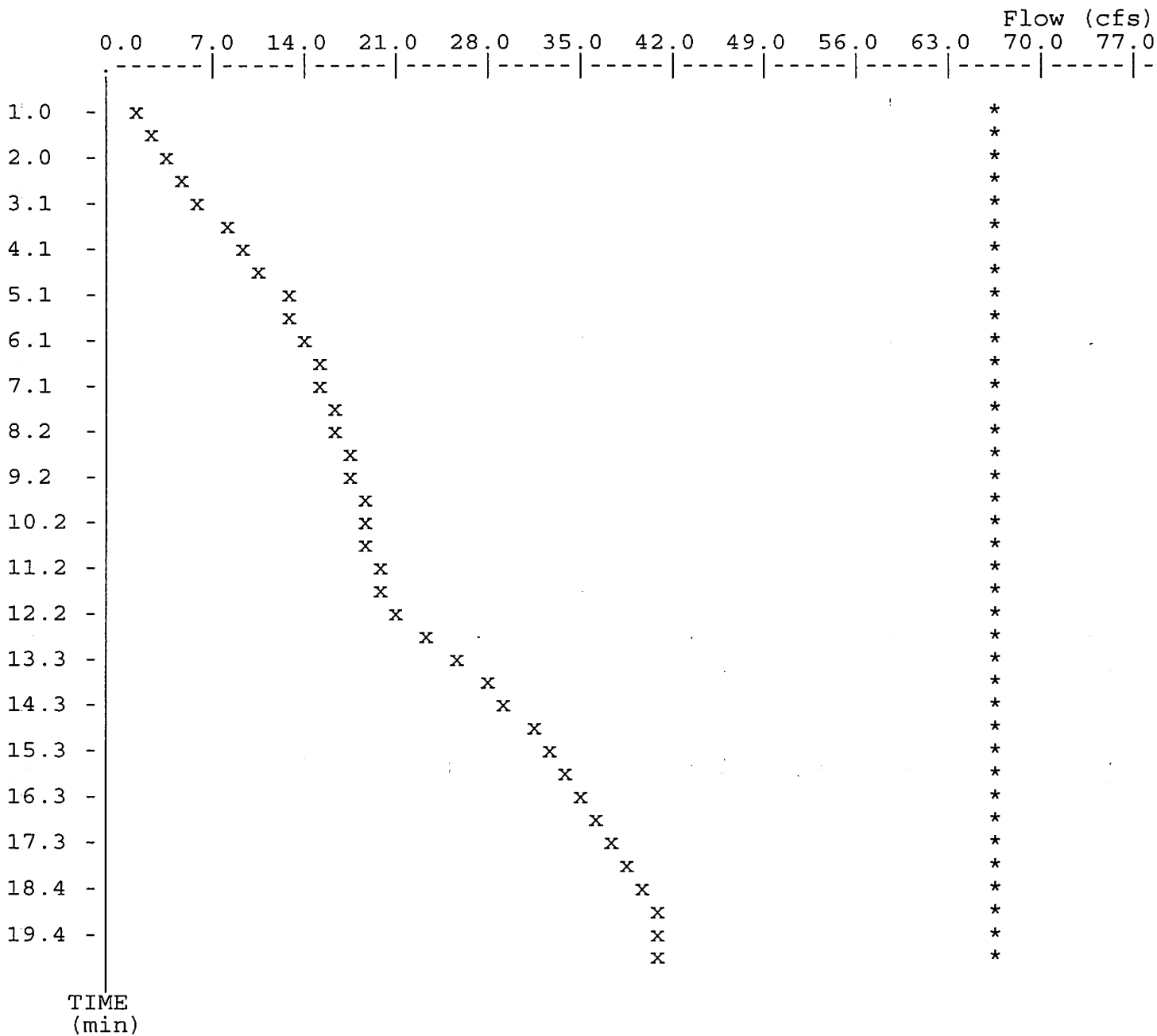
Initial Storage = 0 cu-ft
Peak Storage From Storm = 53,372 cu-ft

Total Storage in Pond = 53,372 cu-ft

Pond File: n:\97454\deten\POND .PND
 Inflow Hydrograph: n:\97454\deten\025-IN .HYD
 Outflow Hydrograph: n:\97454\deten\OUT .HYD

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 17:48:58

Peak Inflow = 66.94 cfs
 Peak Outflow = 40.64 cfs
 Peak Elevation = 535.18 ft



x File: n:\97454\deten\025-IN .HYD Qmax = 40.6 cfs
 * File: n:\97454\deten\OUT .HYD Qmax = 66.9 cfs

Outlet Structure File: 100YRLF.B.STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations for the 100 yr Storm Low Flow Blocked

***** COMPOSITE OUTFLOW SUMMARY *****

Elevation (ft)	Q (cfs)	Contributing Structures
-----	-----	-----
533.75	0.0	1
534.25	6.0	2 +3
534.75	13.3	2 +3
535.18	20.3	2 +3 +4
535.25	24.0	2 +3 +4
535.75	88.7	2 +3 +4
536.00	0.0	

Outlet Structure File: 100YRLF.B.STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations for the 100 yr Storm Low Flow Blocked

Outlet Structure File: n:\97454\deten\100YRLF.B.STR
Planimeter Input File: n:\97454\deten\97454VOL.VOL
Rating Table Output File: n:\97454\deten\POND2 .PND

Min. Elev.(ft) = 533.75 Max. Elev.(ft) = 536 Incr.(ft) = .5

Additional elevations (ft) to be included in table:
* * * * *
535.18

SYSTEM CONNECTIVITY

Structure	No.	Q Table	Q Table
WEIR-VR	1	->	1
ORIFICE	2	->	2
WEIR-VR	3	->	3
WEIR-VR	4	->	4

Outflow rating table summary was stored in file:
n:\97454\deten\POND2 .PND

Outlet Structure File: 100YRLF.B.STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING

Detention Calculations for the 100 yr Storm Low Flow Blocked

>>>>> Structure No. 1 <<<<<<
(Input Data)

WEIR-VR
Weir - Vertical Rectangular

E1 elev.(ft)?	533.75
E2 elev.(ft)?	534.25
Weir coefficient?	3
Weir elev.(ft)?	533.75
Length (ft)?	1
Contracted/Suppressed (C/S)?	S

Outlet Structure File: 100YRLFB.STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING

Detention Calculations for the 100 yr Storm Low Flow Blocked

>>>>> Structure No. 2 <<<<<<
(Input Data)

ORIFICE

Orifice - Based on Area and Datum Elevation

E1 elev.(ft)?	534.25
E2 elev.(ft)?	536
Orifice coeff.?	3
Invert elev.(ft)?	533.75
Datum elev.(ft) ?	534
Orifice area (sq ft)?	.5

Outlet Structure File: 100YRLF.B.STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations for the 100 yr Storm Low Flow Blocked

>>>>> Structure No. 3 <<<<<<
(Input Data)

WEIR-VR
Weir - Vertical Rectangular

E1 elev.(ft)?	534.25
E2 elev.(ft)?	536.00
Weir coefficient?	3
Weir elev.(ft)?	534.25
Length (ft)?	2.67
Contracted/Suppressed (C/S)?	S

Outlet Structure File: 100YRLFB.STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations for the 100 yr Storm Low Flow Blocked

>>>>> Structure No. 4 <<<<<<
(Input Data)

WEIR-VR
Weir - Vertical Rectangular

E1 elev.(ft)?	535.18
E2 elev.(ft)?	536
Weir coefficient?	3
Weir elev.(ft)?	535.18
Length (ft)?	45.00
Contracted/Suppressed (C/S)?	S

Outlet Structure File: 100YRLFB.STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations for the 100 yr Storm Low Flow Blocked

Outflow Rating Table for Structure #1
WEIR-VR Weir - Vertical Rectangular

***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation Messages
533.75	0.0	H =0.0
534.25	0.0	E = or > E2=534.25
534.75	0.0	E = or > E2=534.25
535.18	0.0	E = or > E2=534.25
535.25	0.0	E = or > E2=534.25
535.75	0.0	E = or > E2=534.25
536.00	0.0	E = or > E2=534.25

C = 3 L (ft) = 1
H (ft) = Table elev. - Invert elev. (533.75 ft)
Q (cfs) = C * L * (H**1.5) -- Suppressed Weir

Outlet Structure File: 100YRLFB.STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations for the 100 yr Storm Low Flow Blocked

Outflow Rating Table for Structure #2
ORIFICE Orifice - Based on Area and Datum Elevation

Elevation (ft)	Q (cfs)	Computation Messages
533.75	0.0	E < E1=534.25
534.25	6.0	H =.25
534.75	10.4	H =.750
535.18	13.1	H =1.18
535.25	13.5	H =1.25
535.75	15.9	H =1.75
536.00	0.0	E = or > E2=536

C = 3 A = .5 sq.ft.
H (ft) = Table elev. - Datum elev. (534 ft)
Q (cfs) = C * A * sqr(2g * H)

Outlet Structure File: 100YRLFB.STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations for the 100 yr Storm Low Flow Blocked

Outflow Rating Table for Structure #3
WEIR-VR Weir - Vertical Rectangular

***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation Messages
533.75	0.0	E < Inv.El.= 534.25
534.25	0.0	H =0.0
534.75	2.8	H =.5
535.18	7.2	H =.930
535.25	8.0	H =1.0
535.75	14.7	H =1.5
536.00	0.0	E = or > E2=536.00

C = 3 L (ft) = 2.67
H (ft) = Table elev. - Invert elev. (534.25 ft)
Q (cfs) = C * L * (H**1.5) -- Suppressed Weir

Outlet Structure File: 100YRLFB.STR

POND-2 Version: 5.17
Date Executed:

S/N:
Time Executed:

MALLARD'S LANDING
Detention Calculations for the 100 yr Storm Low Flow Blocked

Outflow Rating Table for Structure #4
WEIR-VR Weir - Vertical Rectangular

***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation Messages
533.75	0.0	E < Inv.El.= 535.18
534.25	0.0	E < Inv.El.= 535.18
534.75	0.0	E < Inv.El.= 535.18
535.18	0.0	H =0.0
535.25	2.5	H =.07
535.75	58.1	H =.570
536.00	0.0	E = or > E2=536

C = 3 L (ft) = 45
H (ft) = Table elev. - Invert elev. (535.18 ft)
Q (cfs) = C * L * (H**1.5) -- Suppressed Weir

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*****
*
*           MALLARD'S LANDING           *
*       Detention Calculations         *
* 100 Year Storm With Low Flow Blocked *
*
*
*****
  
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Inflow Hydrograph: n:\97454\deten\100-IN .HYD
 Rating Table file: n:\97454\deten\POND2 .PND

----INITIAL CONDITIONS----
 Elevation = 533.75 ft
 Outflow = 0.00 cfs
 Storage = 34,703 cu-ft

GIVEN POND DATA

ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (cu-ft)
533.75	0.0	34,703
534.25	6.0	40,872
534.75	13.3	47,428
535.18	20.3	53,381
535.25	24.0	54,378
535.75	88.7	61,728

INTERMEDIATE ROUTING
 COMPUTATIONS

2S/t (cfs)	2S/t + 0 (cfs)
1156.8	1156.8
1362.4	1368.4
1580.9	1594.2
1779.4	1799.7
1812.6	1836.6
2057.6	2146.3

Time increment (t) = 1.0 min.

Pond File: n:\97454\deten\POND2 .PND
 Inflow Hydrograph: n:\97454\deten\100-IN .HYD
 Outflow Hydrograph: n:\97454\deten\OUT .HYD

INFLOW HYDROGRAPH

ROUTING COMPUTATIONS

TIME (min)	INFLOW (cfs)	I1+I2 (cfs)	2S/t - 0 (cfs)	2S/t + 0 (cfs)	OUTFLOW (cfs)	ELEVATION (ft)
0.0	0.00	-----	1156.8	1156.8	0.00	533.75
1.0	85.64	85.6	1237.6	1242.4	2.43	533.95
2.0	85.64	171.3	1394.2	1408.8	7.31	534.34
3.0	85.64	171.3	1540.8	1565.5	12.37	534.69
4.0	85.64	171.3	1677.4	1712.0	17.31	535.00
5.0	85.64	171.3	1795.6	1848.7	26.53	535.27
6.0	85.64	171.3	1864.5	1966.9	51.23	535.46
7.0	85.64	171.3	1904.5	2035.7	65.61	535.57
8.0	85.64	171.3	1927.9	2075.8	73.98	535.64
9.0	85.64	171.3	1941.4	2099.1	78.85	535.67
10.0	85.64	171.3	1949.4	2112.7	81.69	535.70
11.0	85.64	171.3	1954.0	2120.6	83.34	535.71
12.0	85.64	171.3	1956.6	2125.2	84.30	535.72
13.0	85.64	171.3	1958.2	2127.9	84.86	535.72
14.0	85.64	171.3	1959.1	2129.5	85.19	535.72
15.0	85.64	171.3	1959.6	2130.4	85.38	535.72
16.0	85.64	171.3	1959.9	2130.9	85.49	535.73
17.0	85.64	171.3	1960.1	2131.2	85.55	535.73
18.0	85.64	171.3	1960.2	2131.4	85.59	535.73
19.0	85.64	171.3	1960.3	2131.5	85.61	535.73
20.0	85.64	171.3	1960.3	2131.6	85.62	535.73
21.0	0.00	85.6	1910.5	2046.0	67.74	535.59
22.0	0.00	0.0	1831.6	1910.5	39.44	535.37
23.0	0.00	0.0	1784.6	1831.6	23.50	535.24
24.0	0.00	0.0	1745.0	1784.6	19.79	535.15
25.0	0.00	0.0	1708.2	1745.0	18.44	535.07
26.0	0.00	0.0	1673.8	1708.2	17.18	534.99
27.0	0.00	0.0	1641.8	1673.8	16.01	534.92
28.0	0.00	0.0	1611.9	1641.8	14.92	534.85
29.0	0.00	0.0	1584.1	1611.9	13.90	534.79
30.0	0.00	0.0	1558.2	1584.1	12.97	534.73
31.0	0.00	0.0	1533.9	1558.2	12.13	534.67
32.0	0.00	0.0	1511.2	1533.9	11.35	534.62
33.0	0.00	0.0	1490.0	1511.2	10.62	534.57
34.0	0.00	0.0	1470.1	1490.0	9.93	534.52
35.0	0.00	0.0	1451.5	1470.1	9.29	534.48
36.0	0.00	0.0	1434.2	1451.5	8.69	534.43
37.0	0.00	0.0	1417.9	1434.2	8.13	534.40
38.0	0.00	0.0	1402.7	1417.9	7.60	534.36
39.0	0.00	0.0	1388.5	1402.7	7.11	534.33
40.0	0.00	0.0	1375.2	1388.5	6.65	534.29

***** SUMMARY OF ROUTING COMPUTATIONS *****

Pond File: n:\97454\deten\POND2 .PND
Inflow Hydrograph: n:\97454\deten\100-IN .HYD
Outflow Hydrograph: n:\97454\deten\OUT .HYD

Starting Pond W.S. Elevation = 533.75 ft

***** Summary of Peak Outflow and Peak Elevation *****

Peak Inflow = 85.64 cfs
Peak Outflow = 85.62 cfs
Peak Elevation = 535.73 ft

***** Summary of Approximate Peak Storage *****

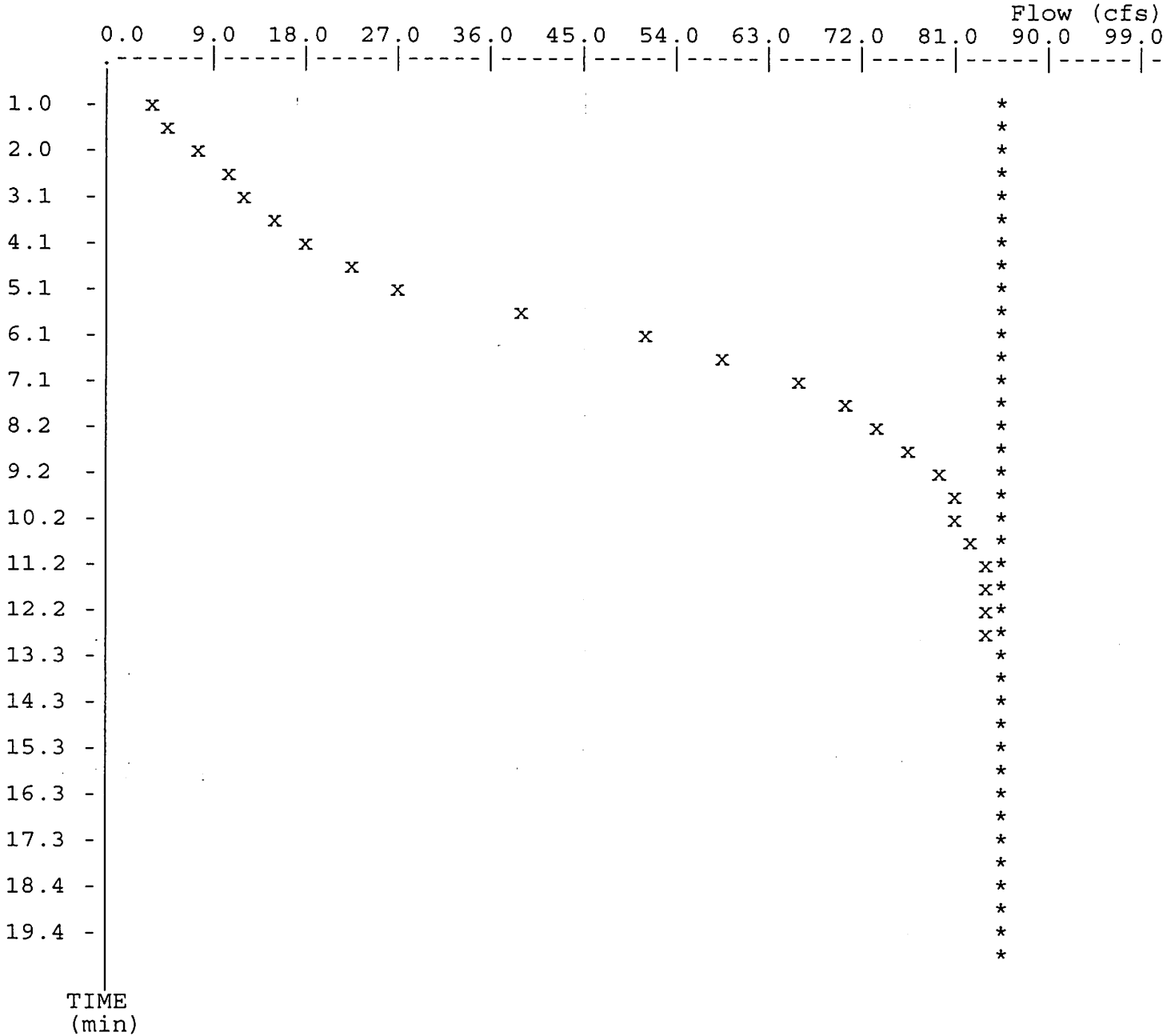
Initial Storage = 34,703 cu-ft
Peak Storage From Storm = 26,675 cu-ft

Total Storage in Pond = 61,378 cu-ft

Pond File: n:\97454\deten\POND2 .PND
 Inflow Hydrograph: n:\97454\deten\100-IN .HYD
 Outflow Hydrograph: n:\97454\deten\OUT .HYD

EXECUTED: 03-24-1998
 17:29:45

Peak Inflow = 85.64 cfs
 Peak Outflow = 85.62 cfs
 Peak Elevation = 535.73 ft



x File: n:\97454\deten\100-IN .HYD Qmax = 85.6 cfs
 * File: n:\97454\deten\OUT .HYD Qmax = 85.6 cfs

JOB: 97454 FB: 1221 BY: CEY 05/28/98
 ID: 004
 DOMINION PROPERTIES, INC.
 MALLARD'S LANDING 6:35P

THE CLAYTON ENGINEERING COMPANY
 12755 OLIVE BLVD - SUITE 100
 ST. LOUIS, MO 63141
 (314) 542-0009

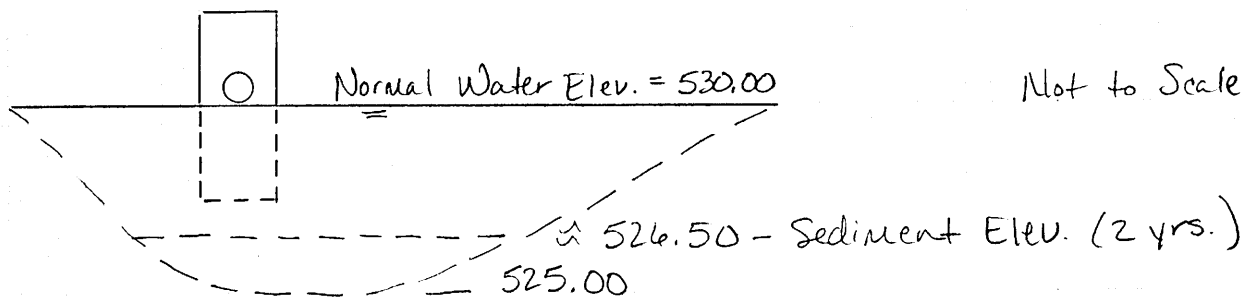
<<<< PIPE HYDRAULICS MODULE >>>>

Description = OUTFALL
 System Number = 4
 Return Period (yr) = 15
 Rainfall Duration (min) = 20
 Runoff Factor Multiplier = 1.00
 Starting HGL Elev. (ft) = 0.00
 Use St. Louis Co./MSD Losses? = Y

STRUCTURES UP/LOW	FLOWLINES UP/LOW	DIAM	LENGTH	n	AREA/PI	Qadd	Qtot/Qfull	CS/RS	Yn/Yc	PARTIAL	FULL	LOSSES		COND	HGL	UPPER STRUCT
										V/Y	V/Vhead	F/C	V/T	UP/LOW	UP/LOW	TOP/FREED
OS 4-2	527.00	36	46	0.013	32.34	32.34	32.34	1.00	1.47	9.38	4.58	1.99	0.43	OF	530.43	535.67
FES 4-1	526.54				1.00		66.88	0.23	1.85	1.47	0.33	0.00	0.00	ND	528.01	5.24

THE CLAYTON ENGINEERING CO.

Project No. 97454 Project Name Mallard's Landing
 Computation for Sediment Storage in Detention Basin
 By CEY Date 5/28/98 Checked _____ Date _____ Sheet _____ of _____



Volume of Pond at Normal Depth = 19,950 cu. ft.

Annual Sediment Storage = $125 \times 13.1 = 1,640$ cu. ft.

For 2 years = $1,640 \times 2 = 3280$ cu. ft.

(See attached Figure 6)

* Since this is a wet basin, the sediment does not effect the Storage capacity. The low flow outlet remains constant and therefore the normal water elevation remains constant. This in turn means the detention volume remains constant with or without sediment

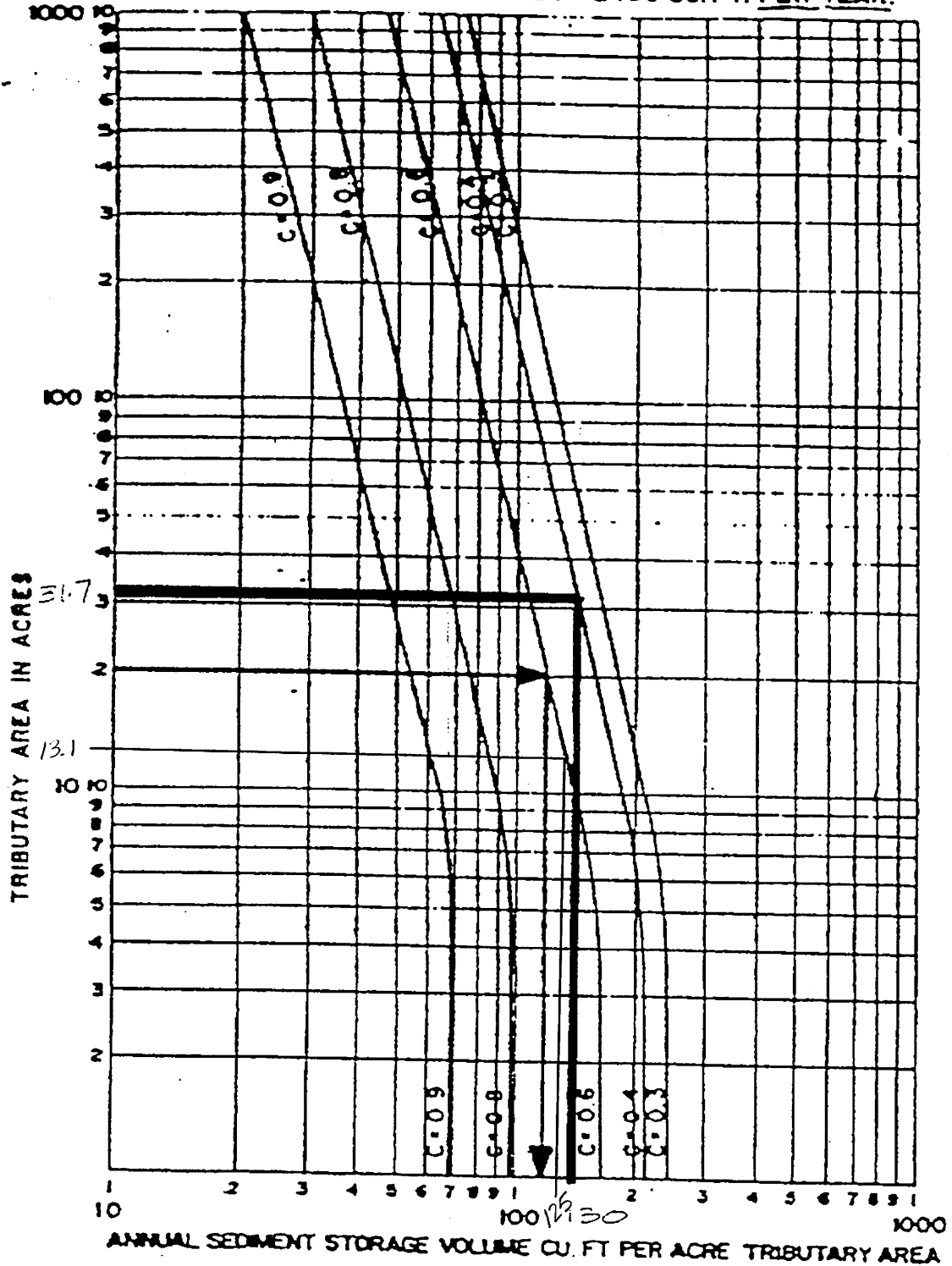
EXAMPLE:

TRIBUTARY AREA = 20 ACRES

RATIONAL METHOD RUNOFF COEFFICIENT "C" = 0.6

SEDIMENT STORAGE = 120 CU. FT. PER ACRE PER YEAR

TOTAL SEDIMENT STORAGE = 120 X 20 = 2400 CU. FT. PER YEAR.



$13.1 \times 2 = 26.2$

ANNUAL SEDIMENT STORAGE

$125 \times 13.1 \times 2 = 3275 \text{ cu. ft.}$

FIG. 6

POND-2 Version: 5.17
S/N:

Volume of Pond Below the Normal Water Elevation
Mallard's Landing

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Planimeter scale: 1 inch = 1 ft.

Elevation (ft)	Planimeter (sq.in.)	Area (sq.ft)	$A1+A2+\text{sqr}(A1*A2)$ (sq.ft)	* Volume (cubic-ft)	Volume Sum (cubic-ft)
525.00	1,659.92	1,660	0	0	0
526.00	2,488.87	2,489	6,181	2,060	2,060
527.00	3,402.32	3,402	8,801	2,934	4,994
528.00	4,409.62	4,410	11,685	3,895	8,889
529.00	5,513.66	5,514	14,854	4,951	13,841
530.00	6,721.40	6,721	18,323	6,108	19,948

* Incremental volume computed by the Conic Method for Reservoir Volumes.