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**TRANSLAND TRUCKING
EAST TERRA LANE
O'FALLON, MO**

DETENION BASIN REPORT

**PREPARED FOR:
MW REAL ESTATE
1601 WEST OLD ROUTE
STAFFORD, MISSOURI 65757**

**PREPARED BY:
ST. CHARLES ENGINEERING & SURVEYING, INC.**

November 2022

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SITE INFORMATION AND PROPOSED DETENTION

Transland Trucking is 4.47-acre site on East Terra Lane at Commerce Drive in the City of O'Fallon, Missouri. The site is located between State Highway 79 on the east side and East Terra Lane on the west side. The site was previously graded but has not been developed. This report provide the hydraulic analysis to provide the storm water detention for the Proposed Transland Trucking site.

The site is less than 10 acres, so this report was prepared using the Rational Method of storm water analysis for the storms that are a 20-minute duration. The travel time is mainly sheet flow over pavement then a short length of storm sewer travel time and 2-minute durations has been assigned for the maximum travel time to the detention basin.

As shown on the Existing Drainage Area Map 0.45 acres of the site drains off-site to the south to a ditch along the Highway 79 exit ramp to a 36-inch reinforced concrete pipe on the west side of the Highway 79 pavement. An additional .05 acres of off-site land on East Terra Lane drain onto the side and travels with the above mentioned 0.45 acres draining off-site to the south. The north 4.02 acres of the site drains to a low point along the east property line to a ditch along Highway 79 that drains to the above mentioned 36-inch reinforced concrete pipe on the west side of Highway 79. An additional 3.03 acres off-site land to the west, north and east drains onto the site draining to the low point on the east property line.

As shown on the Proposed Drainage Area Map all of the site proposed pavement and buildings area of 3.83 acres will drain to the proposed detention basin located in the southeast corner of the site plus 0.07 acres of East Terra Lane that drains into the site at the entrance. The total area draining to the detention basin is 3.9 acres and that area is 87 percent impervious. As shown at the south end of the site 0.26 acres will by-pass the detention basin draining to the ditch to the south. As shown at the north end of the site 0.38 acres will by-pass the detention basin and drain to a proposed on-site swale along the east property line to the site low point along Highway 79. The total site area by-passing the basin will be 0.64 acres. An additional 3.02 acres of off-site land to the west, north, and east will also drain in the proposed on-site swale along the east property line to the site low point along Highway 79.

The allowable release from the detention basin was determined using the differential runoff for the predeveloped 4.47 acres of the site less the post developed site area of 0.64 acres of pervious area by-passes the detention basin as indicated above and as shown on the drainage area map.

The detention report was prepared using Hydraflow Hydrographs Extension for Autodesk Civil 3D software. The combined discharge of the basin routing and the basin by-pass is shown as the actual release from the site which are all below the allowable release.

The low flow openings blocked analysis for the 100-year storm assumes the lower opening on the Outfall Structure to be blocked and then routing the storm water through be basin. The top of the detention basin dam provides at least 1.0 feet of freeboard to the top of the detention basin dam.

To filter the storm water runoff all of the curb inlets catch basins include a catch basin inlet filter per the detail shown on the plans (or equal product).

Catch Basin Inlet Filters shall be serviced on a recurring basis to remove pollutant loading and interference from debris (leaves, vegetation, or trash). Each insert filter installation shall be serviced a minimum of three times per year with a change of filter medium pouches once per year. Timing of insert filter maintenance shall be March 1st, July 1st, and November 1st being prior to and just after the winter snow season and during the summer.

Service Procedures:

1. Remove catch basin top or grate and set to one side. The catch basin shall be visually inspected for defects and possible illegal dumping. If illegal dumping has occurred, the proper authorities shall be notified as soon as practicable.
2. Using an individual vacuum, the collected materials shall be removed from the liner.
3. When all collect material have been removed, the filter medium pouches shall be removed per the manufacture's recommendations. The filter liner, gaskets, frame, and mounting brackets shall be inspected for continued serviceability. Minor damage or defects found shall be correct on-the-spot and a notation made on the Maintenance Records. More extensive deficiencies that affect the efficiency of the filter (torn liner, etc.) shall be correct and notation made in the Maintenance Record.
4. The filter medium pouches shall be inspected for defects and continued serviceability and replaced as necessary, at least once per year.
5. The top or grate shall be replaced.
6. Used pouches and removed debris shall be disposed of in accordance with local, state, and federal requirements.

DETENTION SUMMARY
Site Area = 4.47 Acres

Pre-Development PIs, 5% Impervious, 20-Minute Storm

Pre-Developed 2-Year PI =	1.15 cfs/acres
Pre-Developed 15-Year PI =	1.7 cfs/acre
Pre-Developed 25-Year PI =	1.81 cfs/acre
Pre-Developed 100-Year PI =	2.29 cfs/acre

Pre-Development 4.47 Acres Discharge, 5% Impervious, 20-Minute Storm

Pre-Developed 2-Year $4.47 \times 1.15 =$	5.14 cfs
Pre-Developed 15-Year $4.47 \times 1.7 =$	7.60 cfs
Pre-Developed 25-Year $4.47 \times 1.81 =$	9.09 cfs
Pre-Developed 100-Year $4.47 \times 2.29 =$	10.24 cfs

Less Basin By-Pass, 5% Impervious 0.64 Acres, 20-Minute Storm

		Difference
Post-Developed 2-Year $0.64 \times 1.15 =$	0.74 cfs	4.4 cfs
Post-Developed 15-Year $0.64 \times 1.7 =$	1.09 cfs	6.51 cfs
Post-Developed 25-Year $0.64 \times 1.81 =$	1.16 cfs	7.93 cfs
Post-Developed 100-Year $0.64 \times 2.29 =$	1.47 cfs	8.77 cfs

Plus, Off-Site to Basin 0.07 Acres, 100% Impervious, 20 Min Storm

		Allowable Release
Pre-Developed 2-Year $0.07 \times 2.39 =$	0.17 cfs	4.64 cfs
Pre-Developed 15-Year $0.07 \times 3.54 =$	0.25 cfs	6.76 cfs
Pre-Developed 25-Year $0.07 \times 3.77 =$	0.26 cfs	8.19 cfs
Pre-Developed 100-Year $0.07 \times 4.77 =$	0.33 cfs	9.1 cfs

Post-Development, 87% Impervious, 20- Minute Storm

Post-Developed 2-Year PI =	2.22 cfs/acre
Post-Developed 15-Year PI =	3.29 cfs/acre
Post-Developed 25-Year PI =	3.52 cfs/acre
Post-Developed 100-Year PI =	4.43 cfs/acre

Discharge with 3.9 Acres to Basin

Allowable 2-Year $3.9 \times 2.22 =$	8.66 cfs
Allowable 15-Year $3.9 \times 3.29 =$	12.83 cfs
Allowable 25-Year $3.9 \times 3.52 =$	13.73 cfs
Allowable 100-Year $3.9 \times 4.43 =$	17.28 cfs

Actual Detention Basin Release

Actual 2-Year, 20-Minute Release =	4.15 cfs
Actual 15-Year, 20-Minute Release =	5.35 cfs
Actual 25-Year, 20-Minute Release =	5.63 cfs
Actual 100-Year, 20-Minute Release =	8.53 cfs

Actual All Below Allowable

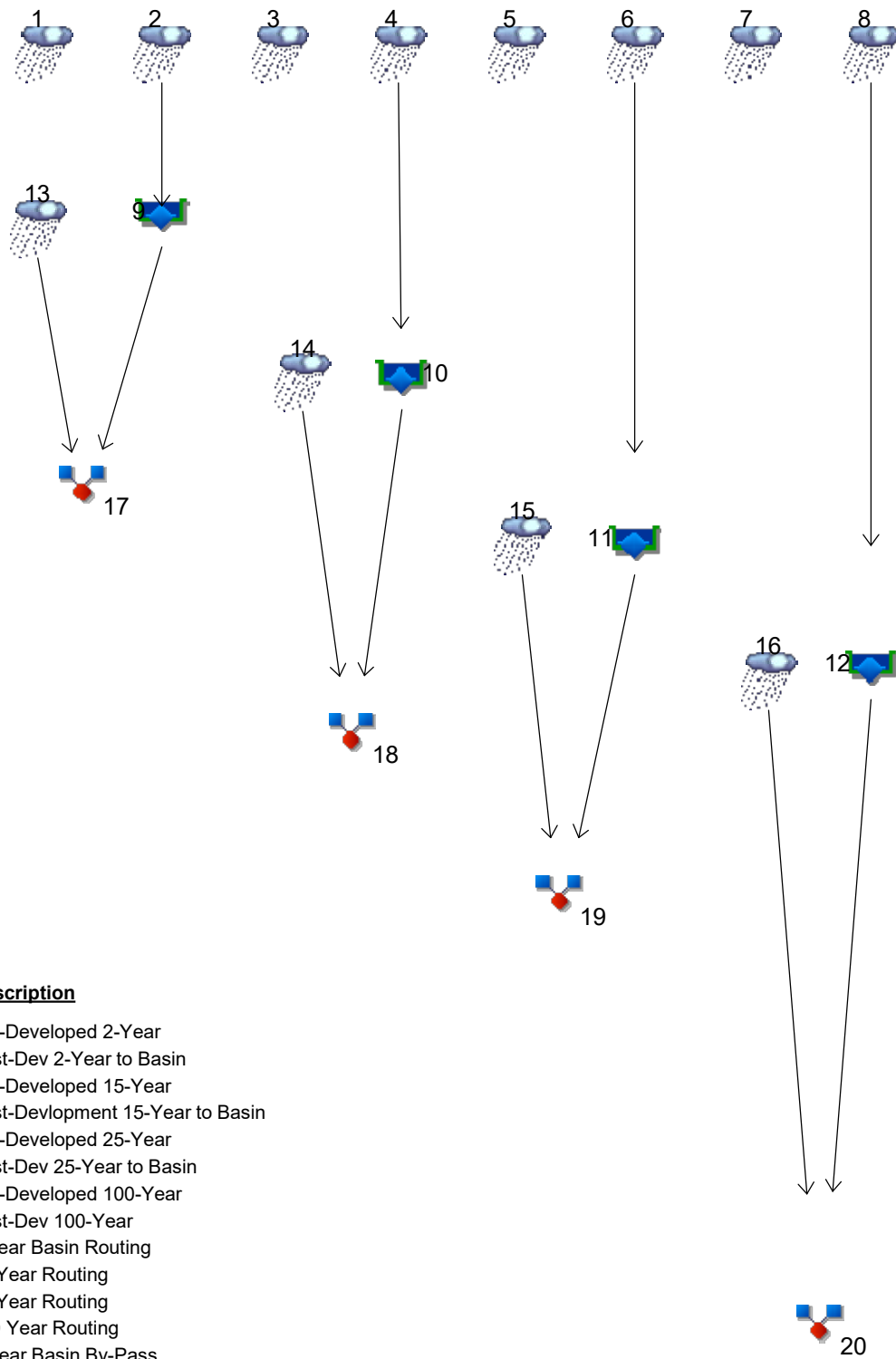
100-Year Low Flow Blocked

Post-Development 100-year low flows blocked
Peak Elevation = 489.40
Top of Dam = 491.00

1.60 Feet of Freeboard

Watershed Model Schematic

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020



Legend

Hyd. Origin	Description
1 Manual	Pre-Developed 2-Year
2 Manual	Post-Dev 2-Year to Basin
3 Manual	Pre-Developed 15-Year
4 Manual	Post-Development 15-Year to Basin
5 Manual	Pre-Developed 25-Year
6 Manual	Post-Dev 25-Year to Basin
7 Manual	Pre-Developed 100-Year
8 Manual	Post-Dev 100-Year
9 Reservoir	2 Year Basin Routing
10 Reservoir	15 Year Routing
11 Reservoir	25 Year Routing
12 Reservoir	100 Year Routing
13 Manual	2-Year Basin By-Pass
14 Manual	15-Year Basin By-Pass
15 Manual	25-Year Basin By-Pass
16 Manual	100-Year Basin By-Pass
17 Combine	2 Year Combined
18 Combine	15-Year Combined
19 Combine	25-Year Combined
20 Combine	100-Year Combined

Hydrograph Return Period Recap

Hydroflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Inflow hyd(s)	Peak Outflow (cfs)								Hydrograph Description
			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
1	Manual	-----	-----	5.140	-----	-----	0.000	0.000	-----	0.000	Pre-Developed 2-Year
2	Manual	-----	-----	8.660	-----	-----	0.000	0.000	-----	0.000	Post-Dev 2-Year to Basin
3	Manual	-----	-----	0.000	-----	-----	7.600	0.000	-----	0.000	Pre-Developed 15-Year
4	Manual	-----	-----	0.000	-----	-----	12.83	0.000	-----	0.000	Post-Development 15-Year to Basin
5	Manual	-----	-----	0.000	-----	-----	0.000	8.090	-----	0.000	Pre-Developed 25-Year
6	Manual	-----	-----	0.000	-----	-----	0.000	13.73	-----	0.000	Post-Dev 25-Year to Basin
7	Manual	-----	-----	0.000	-----	-----	0.000	0.000	-----	10.24	Pre-Developed 100-Year
8	Manual	-----	-----	0.000	-----	-----	0.000	0.000	-----	17.28	Post-Dev 100-Year
9	Reservoir	2	-----	3.443	-----	-----	0.000	0.000	-----	0.000	2 Year Basin Routing
10	Reservoir	4	-----	0.000	-----	-----	4.338	0.000	-----	0.000	15 Year Routing
11	Reservoir	6	-----	0.000	-----	-----	0.000	4.544	-----	0.000	25 Year Routing
12	Reservoir	8	-----	0.000	-----	-----	0.000	0.000	-----	7.589	100 Year Routing
13	Manual	-----	-----	0.740	-----	-----	0.000	0.000	-----	0.000	2-Year Basin By-Pass
14	Manual	-----	-----	0.000	-----	-----	1.090	0.000	-----	0.000	15-Year Basin By-Pass
15	Manual	-----	-----	0.000	-----	-----	0.000	1.160	-----	0.000	25-Year Basin By-Pass
16	Manual	-----	-----	0.000	-----	-----	0.000	0.000	-----	1.470	100-Year Basin By-Pass
17	Combine	9, 13,	-----	4.154	-----	-----	0.000	0.000	-----	0.000	2 Year Combined
18	Combine	10, 14,	-----	0.000	-----	-----	5.353	0.000	-----	0.000	15-Year Combined
19	Combine	11, 15,	-----	0.000	-----	-----	0.000	5.629	-----	0.000	25-Year Combined
20	Combine	12, 16,	-----	0.000	-----	-----	0.000	0.000	-----	8.527	100-Year Combined

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Manual	5.140	1	2	6,168	-----	-----	-----	Pre-Developed 2-Year
2	Manual	8.660	1	2	10,392	-----	-----	-----	Post-Dev 2-Year to Basin
3	Manual	0.000	1	n/a	0	-----	-----	-----	Pre-Developed 15-Year
4	Manual	0.000	1	n/a	0	-----	-----	-----	Post-Development 15-Year to Basin
5	Manual	0.000	6	n/a	0	-----	-----	-----	Pre-Developed 25-Year
6	Manual	0.000	1	n/a	0	-----	-----	-----	Post-Dev 25-Year to Basin
7	Manual	0.000	1	n/a	0	-----	-----	-----	Pre-Developed 100-Year
8	Manual	0.000	1	n/a	0	-----	-----	-----	Post-Dev 100-Year
9	Reservoir	3.443	1	21	10,388	2	488.29	7,226	2 Year Basin Routing
10	Reservoir	0.000	1	n/a	0	4	486.57	0.000	15 Year Routing
11	Reservoir	0.000	1	n/a	0	6	486.57	0.000	25 Year Routing
12	Reservoir	0.000	1	n/a	0	8	486.57	0.000	100 Year Routing
13	Manual	0.740	1	2	888	-----	-----	-----	2-Year Basin By-Pass
14	Manual	0.000	1	n/a	0	-----	-----	-----	15-Year Basin By-Pass
15	Manual	0.000	1	n/a	0	-----	-----	-----	25-Year Basin By-Pass
16	Manual	0.000	1	n/a	0	-----	-----	-----	100-Year Basin By-Pass
17	Combine	4.154	1	20	11,276	9, 13,	-----	-----	2 Year Combined
18	Combine	0.000	1	n/a	0	10, 14,	-----	-----	15-Year Combined
19	Combine	0.000	1	n/a	0	11, 15,	-----	-----	25-Year Combined
20	Combine	0.000	1	n/a	0	12, 16,	-----	-----	100-Year Combined
Basin Hydraulics.gpw					Return Period: 2 Year			Friday, 09 / 30 / 2022	

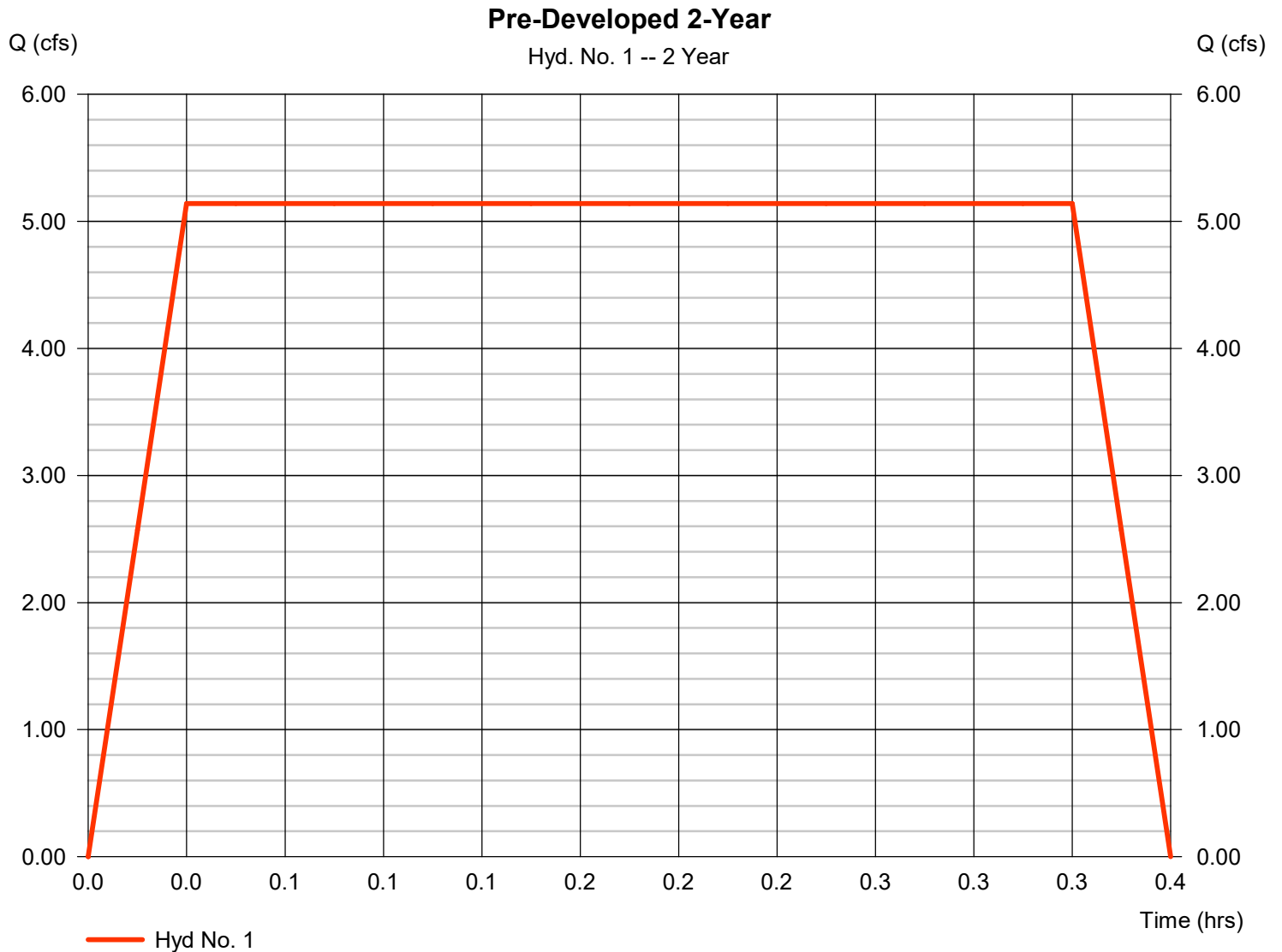
Hydrograph Report

Hyd. No. 1

Pre-Developed 2-Year

Hydrograph type = Manual
Storm frequency = 2 yrs
Time interval = 1 min

Peak discharge = 5.140 cfs
Time to peak = 0.03 hrs
Hyd. volume = 6,168 cuft



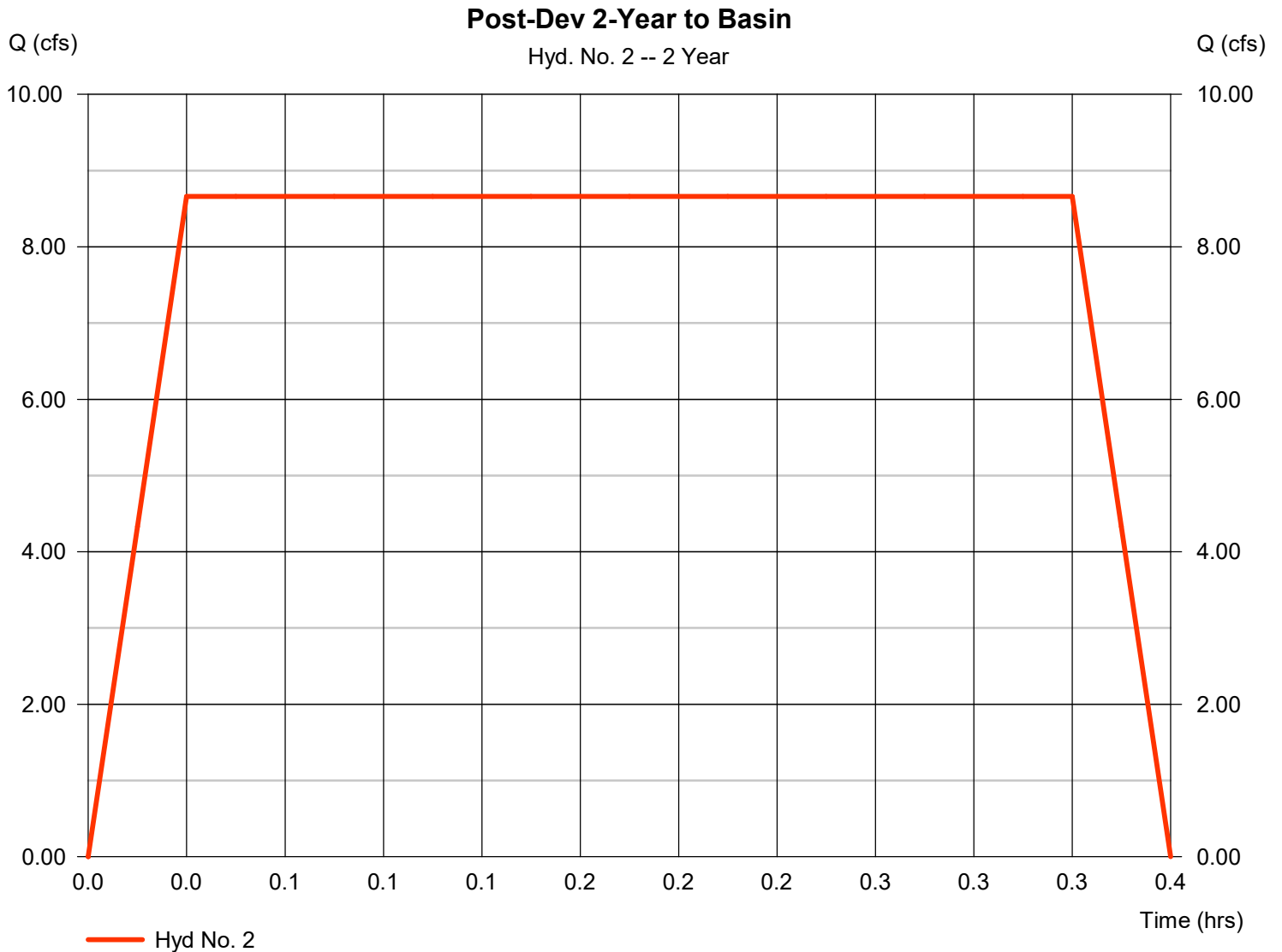
Hydrograph Report

Hyd. No. 2

Post-Dev 2-Year to Basin

Hydrograph type = Manual
Storm frequency = 2 yrs
Time interval = 1 min

Peak discharge = 8.660 cfs
Time to peak = 0.03 hrs
Hyd. volume = 10,392 cuft



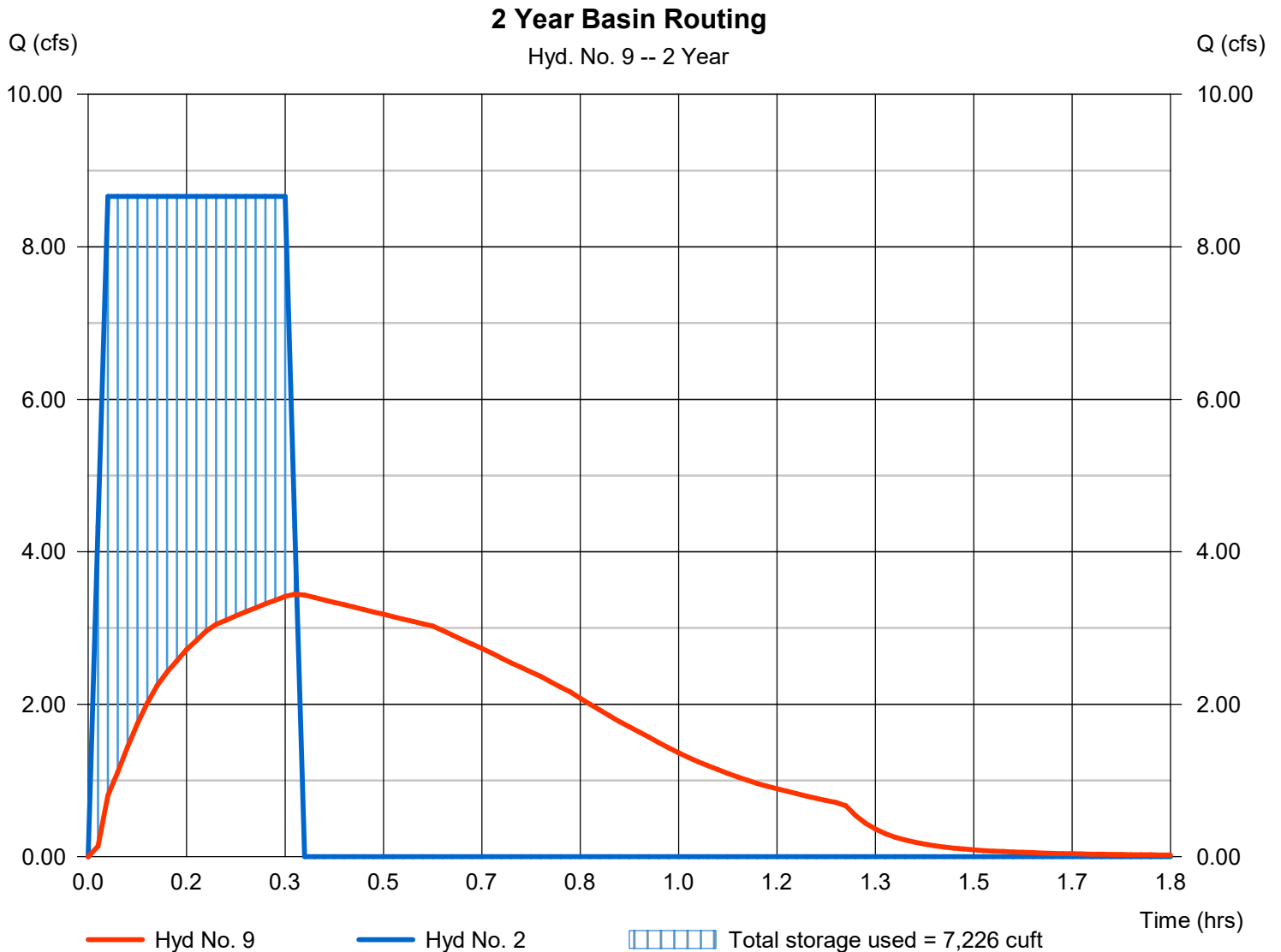
Hydrograph Report

Hyd. No. 9

2 Year Basin Routing

Hydrograph type	= Reservoir	Peak discharge	= 3.443 cfs
Storm frequency	= 2 yrs	Time to peak	= 0.35 hrs
Time interval	= 1 min	Hyd. volume	= 10,388 cuft
Inflow hyd. No.	= 2 - Post-Dev 2-Year to Basin	Max. Elevation	= 488.29 ft
Reservoir name	= Basin	Max. Storage	= 7,226 cuft

Storage Indication method used.



Pond Report

Pond No. 1 - Basin

Pond Data

Contours -User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 486.57 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	486.57	00	0	0
0.43	487.00	2,147	308	308
1.43	488.00	6,841	4,273	4,581
2.43	489.00	11,475	9,058	13,639
3.43	490.00	13,199	12,326	25,964
4.43	491.00	14,090	13,641	39,605

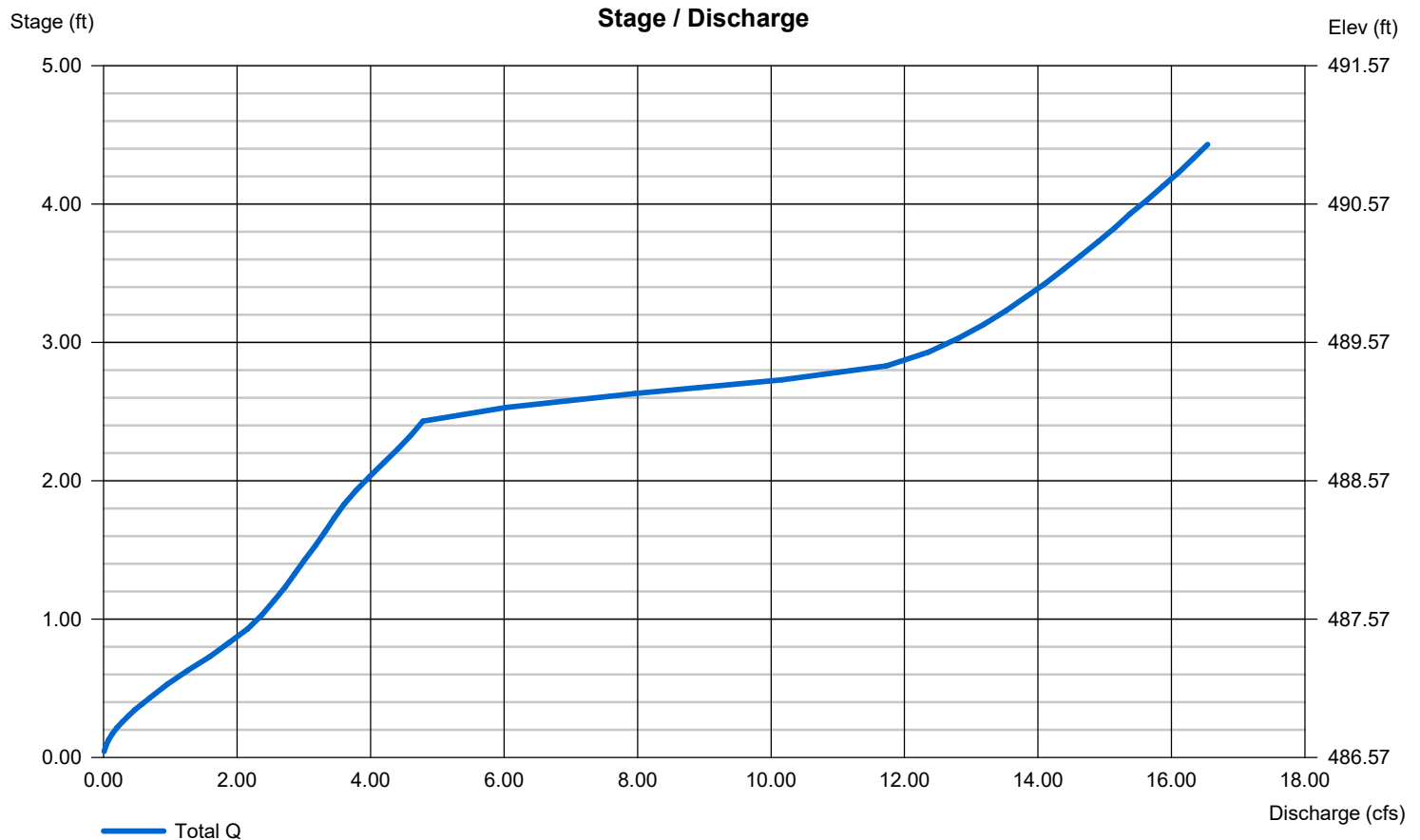
Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 18.00	12.00	8.00	Inactive
Span (in)	= 18.00	12.00	6.00	0.00
No. Barrels	= 1	1	1	0
Invert El. (ft)	= 486.45	486.57	488.33	0.00
Length (ft)	= 20.00	1.00	1.00	0.00
Slope (%)	= 1.00	1.00	1.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	Yes	Yes	No

Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 11.00	0.00	0.00	0.00
Crest El. (ft)	= 489.00	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	---	---	---
Multi-Stage	= Yes	No	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Pond Report

Pond No. 1 - Basin

Pond Data

Contours -User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 486.57 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
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0.43	487.00	2,147	308	308
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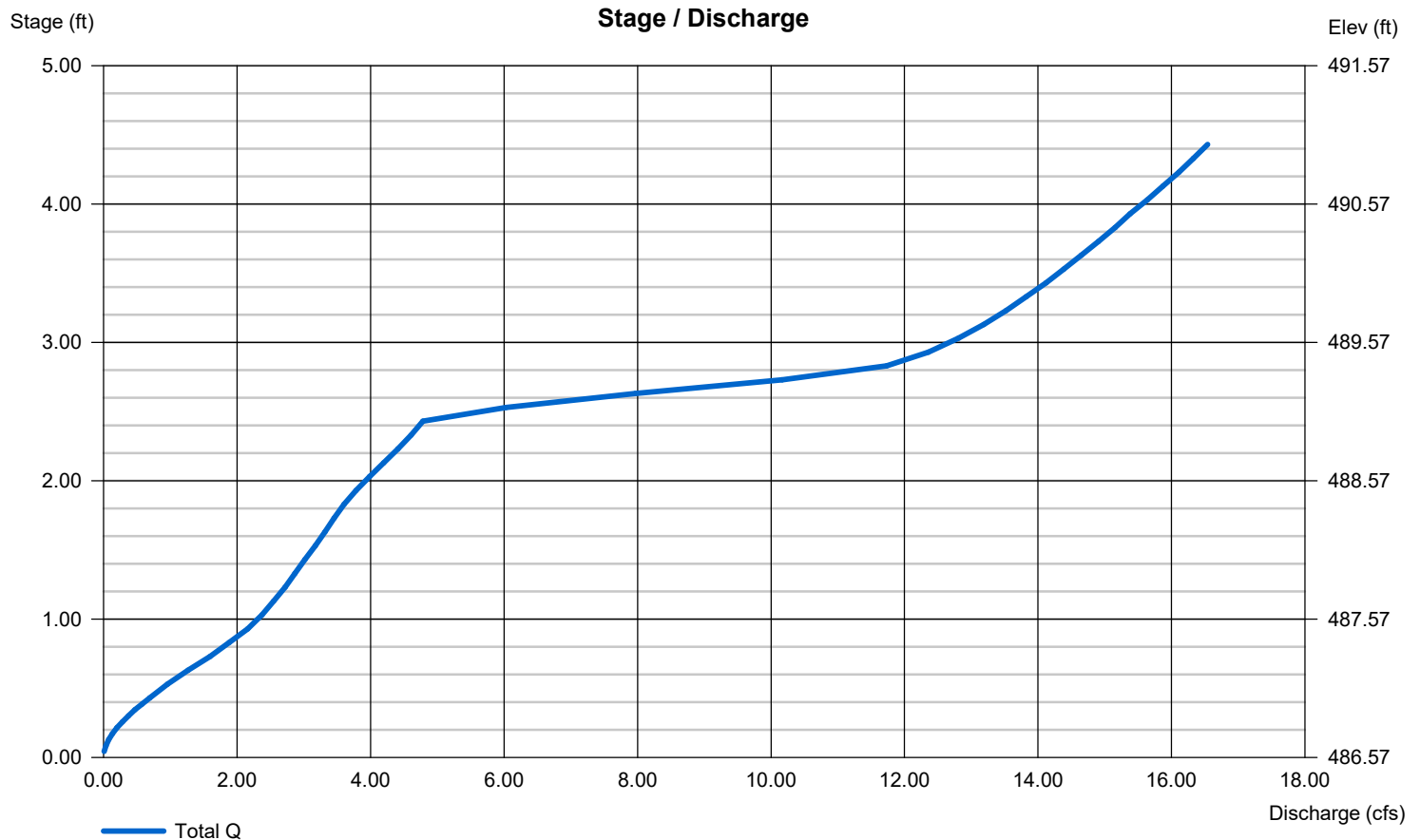
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Slope (%)	= 1.00	1.00	1.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	Yes	Yes	No

Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 11.00	0.00	0.00	0.00
Crest El. (ft)	= 489.00	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	---	---	---
Multi-Stage	= Yes	No	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Pond Report

Pond No. 1 - Basin

Pond Data

Contours -User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 486.57 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	486.57	00	0	0
0.43	487.00	2,147	308	308
1.43	488.00	6,841	4,273	4,581
2.43	489.00	11,475	9,058	13,639
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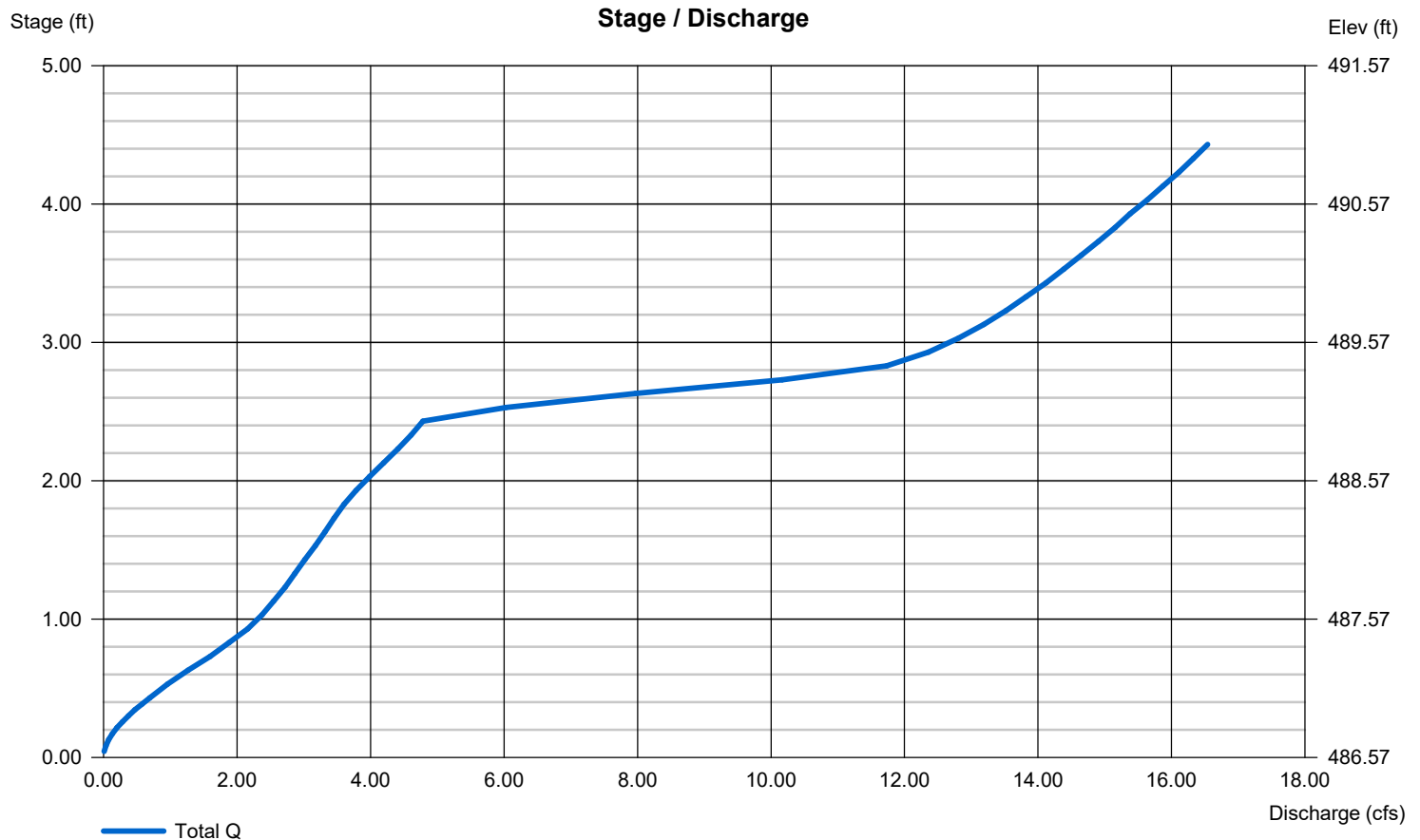
Culvert / Orifice Structures

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Span (in)	= 18.00	12.00	6.00	0.00
No. Barrels	= 1	1	1	0
Invert El. (ft)	= 486.45	486.57	488.33	0.00
Length (ft)	= 20.00	1.00	1.00	0.00
Slope (%)	= 1.00	1.00	1.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	Yes	Yes	No

Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 11.00	0.00	0.00	0.00
Crest El. (ft)	= 489.00	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	---	---	---
Multi-Stage	= Yes	No	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Pond Report

Pond No. 1 - Basin

Pond Data

Contours -User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 486.57 ft

Stage / Storage Table

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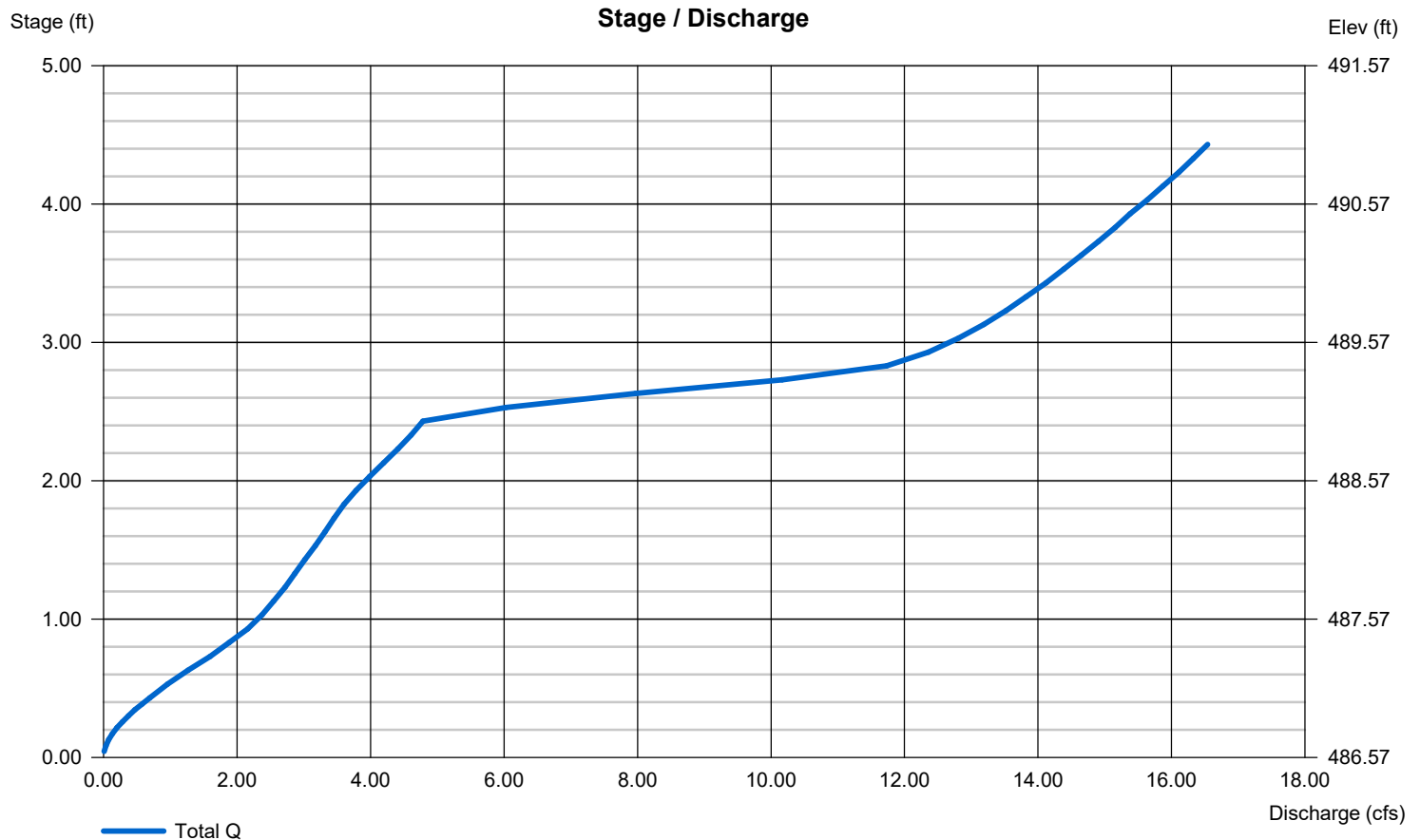
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Length (ft)	= 20.00	1.00	1.00	0.00
Slope (%)	= 1.00	1.00	1.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	Yes	Yes	No

Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 11.00	0.00	0.00	0.00
Crest El. (ft)	= 489.00	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	---	---	---
Multi-Stage	= Yes	No	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

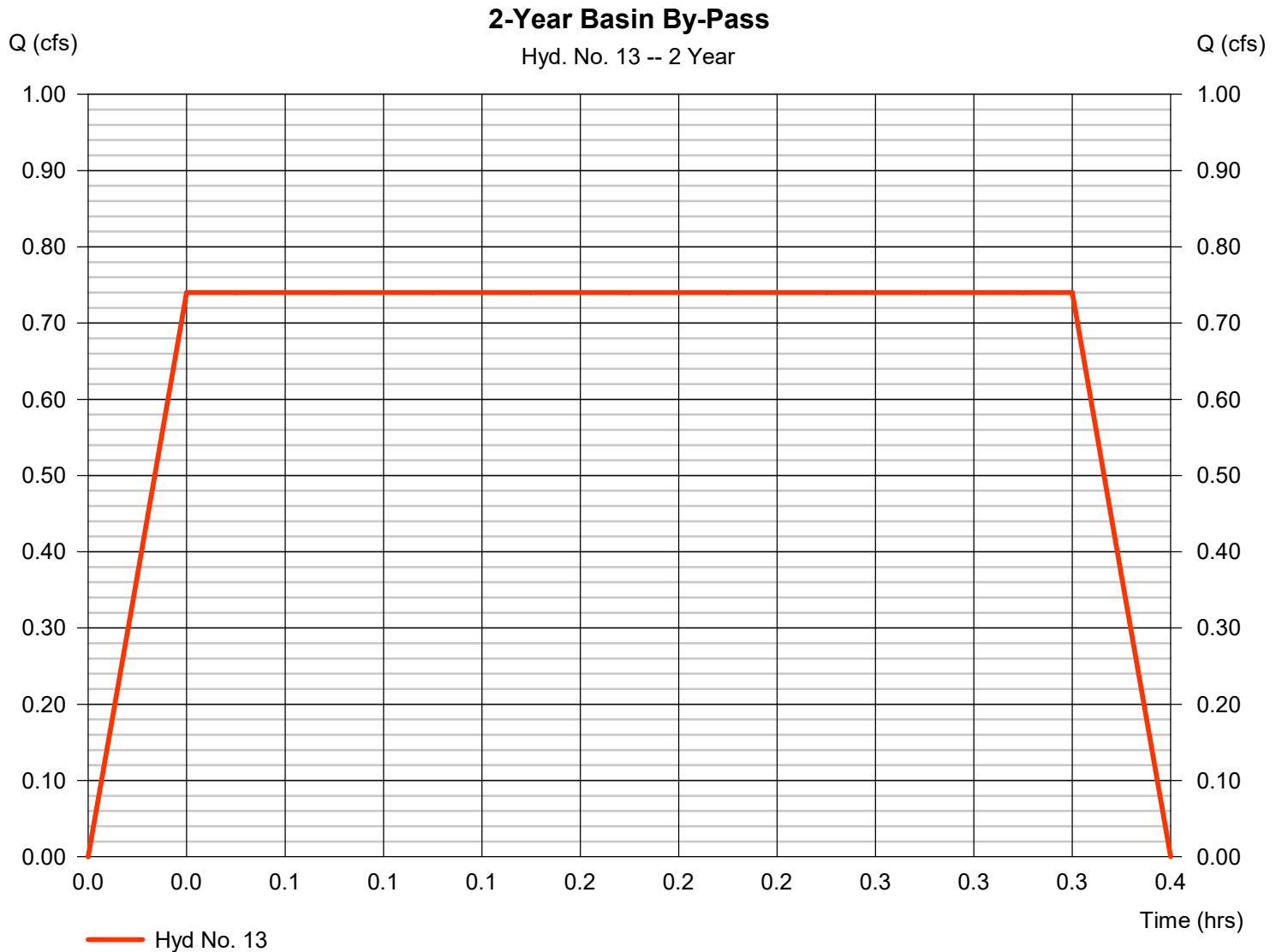


Hydrograph Report

Hyd. No. 13

2-Year Basin By-Pass

Hydrograph type	= Manual	Peak discharge	= 0.740 cfs
Storm frequency	= 2 yrs	Time to peak	= 0.03 hrs
Time interval	= 1 min	Hyd. volume	= 888 cuft



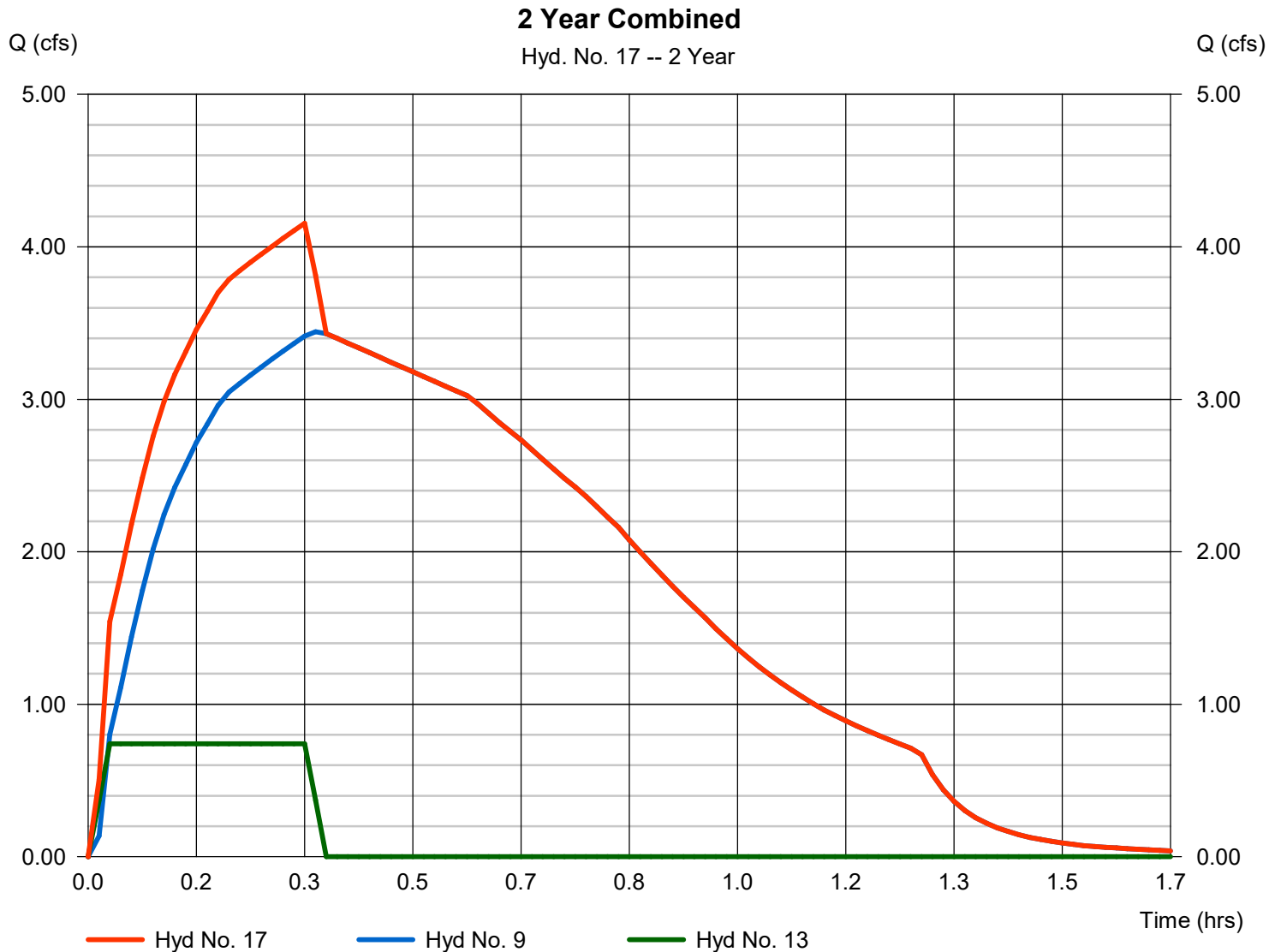
Hydrograph Report

Hyd. No. 17

2 Year Combined

Hydrograph type = Combine
Storm frequency = 2 yrs
Time interval = 1 min
Inflow hyds. = 9, 13

Peak discharge = 4.154 cfs
Time to peak = 0.33 hrs
Hyd. volume = 11,276 cuft
Contrib. drain. area = 0.000 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Manual	0.000	1	n/a	0	----	----	----	Pre-Developed 2-Year
2	Manual	0.000	1	n/a	0	----	----	----	Post-Dev 2-Year to Basin
3	Manual	7.600	1	2	9,120	----	----	----	Pre-Developed 15-Year
4	Manual	12.83	1	2	15,396	----	----	----	Post-Development 15-Year to Basin
5	Manual	0.000	6	n/a	0	----	----	----	Pre-Developed 25-Year
6	Manual	0.000	1	n/a	0	----	----	----	Post-Dev 25-Year to Basin
7	Manual	0.000	1	n/a	0	----	----	----	Pre-Developed 100-Year
8	Manual	0.000	1	n/a	0	----	----	----	Post-Dev 100-Year
9	Reservoir	0.000	1	n/a	0	2	486.57	0.000	2 Year Basin Routing
10	Reservoir	4.338	1	21	15,392	4	488.77	11,526	15 Year Routing
11	Reservoir	0.000	1	n/a	0	6	486.57	0.000	25 Year Routing
12	Reservoir	0.000	1	n/a	0	8	486.57	0.000	100 Year Routing
13	Manual	0.000	1	n/a	0	----	----	----	2-Year Basin By-Pass
14	Manual	1.090	1	2	1,308	----	----	----	15-Year Basin By-Pass
15	Manual	0.000	1	n/a	0	----	----	----	25-Year Basin By-Pass
16	Manual	0.000	1	n/a	0	----	----	----	100-Year Basin By-Pass
17	Combine	0.000	1	n/a	0	9, 13,	----	----	2 Year Combined
18	Combine	5.353	1	20	16,700	10, 14,	----	----	15-Year Combined
19	Combine	0.000	1	n/a	0	11, 15,	----	----	25-Year Combined
20	Combine	0.000	1	n/a	0	12, 16,	----	----	100-Year Combined
Basin Hydraulics.gpw					Return Period: 10 Year			Friday, 09 / 30 / 2022	

Hydrograph Report

Hyd. No. 3

Pre-Developed 15-Year

Hydrograph type = Manual
Storm frequency = 10 yrs
Time interval = 1 min

Peak discharge = 7.600 cfs
Time to peak = 0.03 hrs
Hyd. volume = 9,120 cuft



Hydrograph Report

Hyd. No. 4

Post-Development 15-Year to Basin

Hydrograph type = Manual
Storm frequency = 10 yrs
Time interval = 1 min

Peak discharge = 12.83 cfs
Time to peak = 0.03 hrs
Hyd. volume = 15,396 cuft



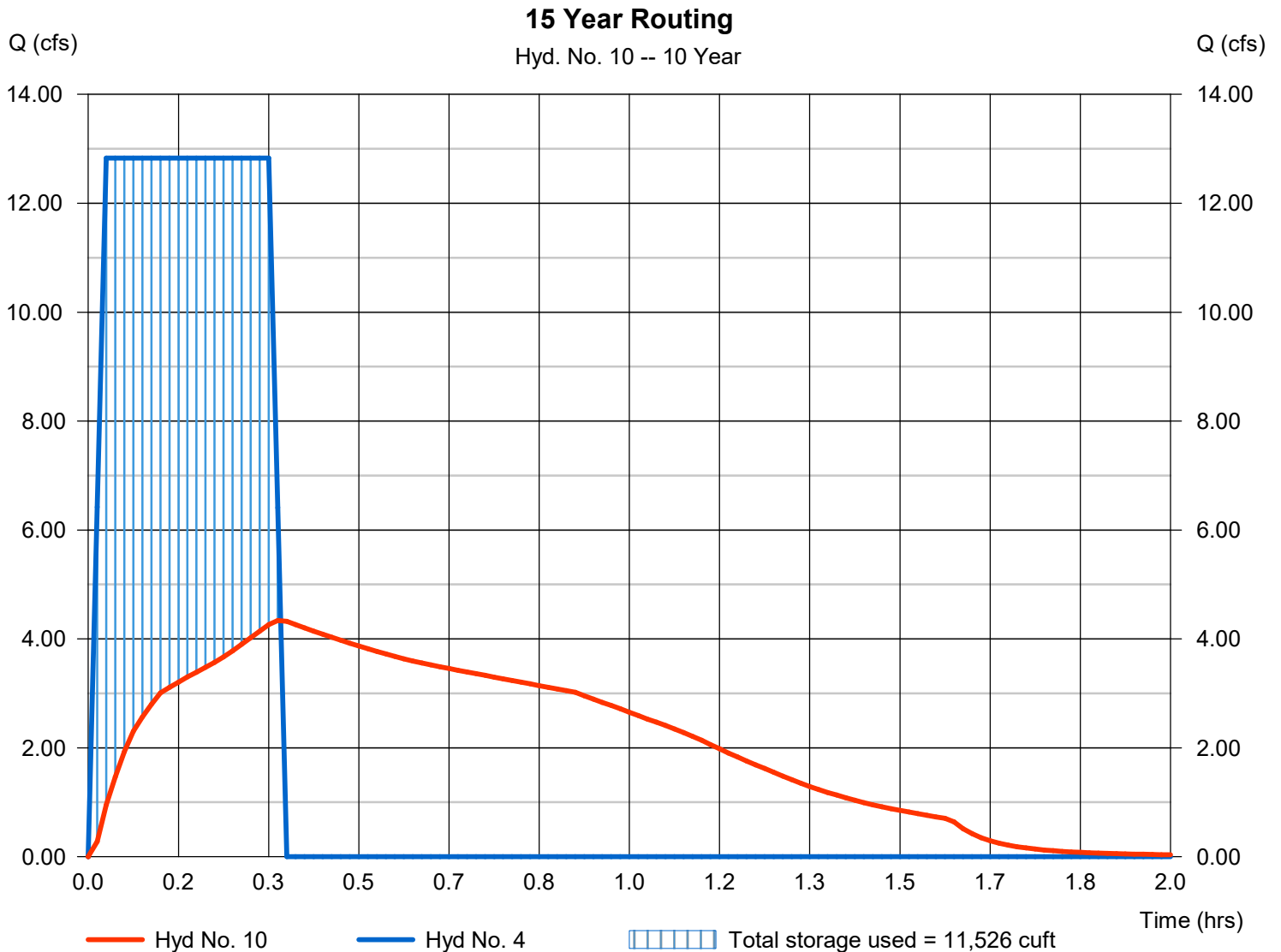
Hydrograph Report

Hyd. No. 10

15 Year Routing

Hydrograph type	= Reservoir	Peak discharge	= 4.338 cfs
Storm frequency	= 10 yrs	Time to peak	= 0.35 hrs
Time interval	= 1 min	Hyd. volume	= 15,392 cuft
Inflow hyd. No.	= 4 - Post-Development 15-Year to Basin	Base Elevation	= 488.77 ft
Reservoir name	= Basin	Max. Storage	= 11,526 cuft

Storage Indication method used.



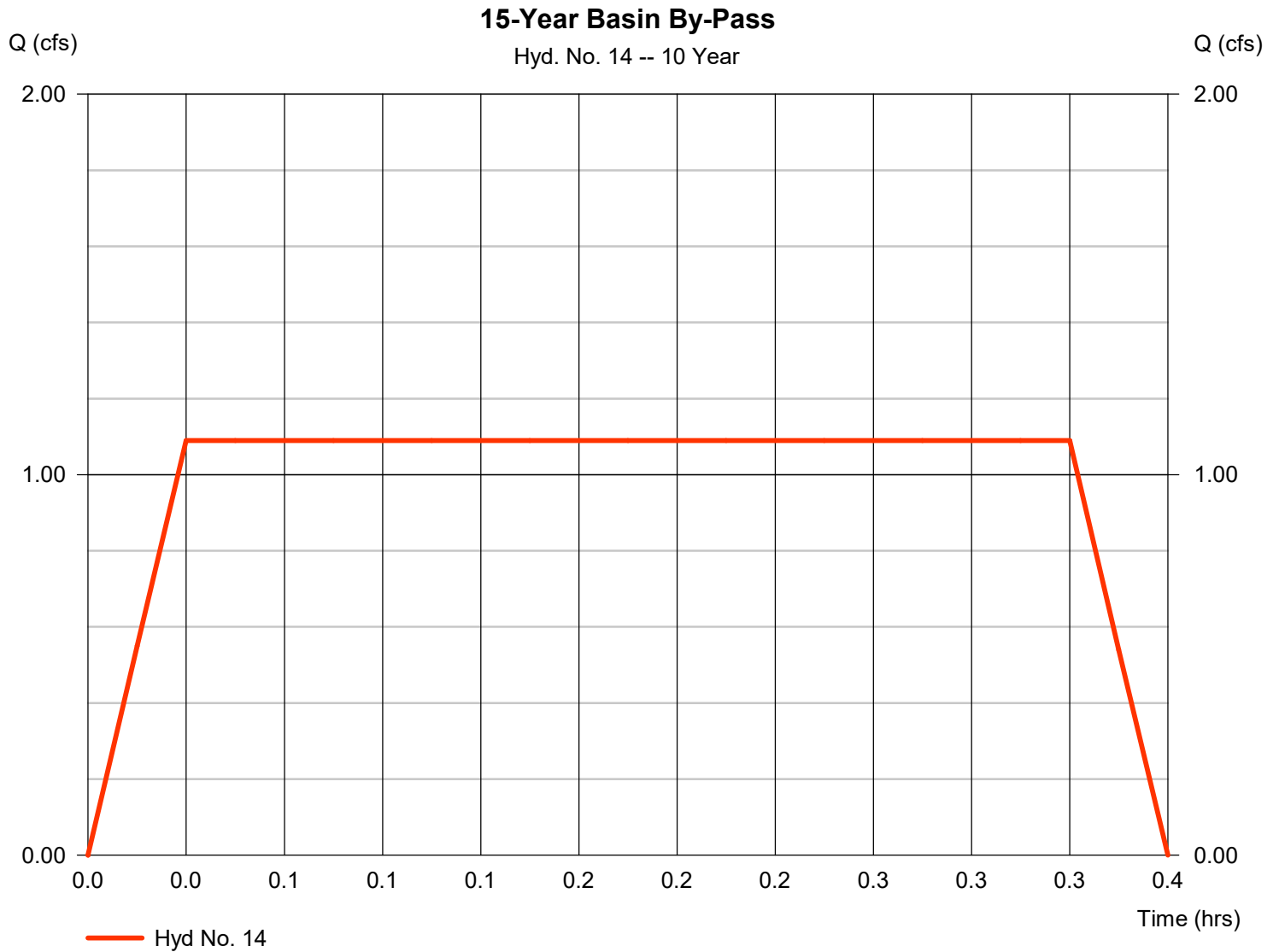
Hydrograph Report

Hyd. No. 14

15-Year Basin By-Pass

Hydrograph type = Manual
Storm frequency = 10 yrs
Time interval = 1 min

Peak discharge = 1.090 cfs
Time to peak = 0.03 hrs
Hyd. volume = 1,308 cuft



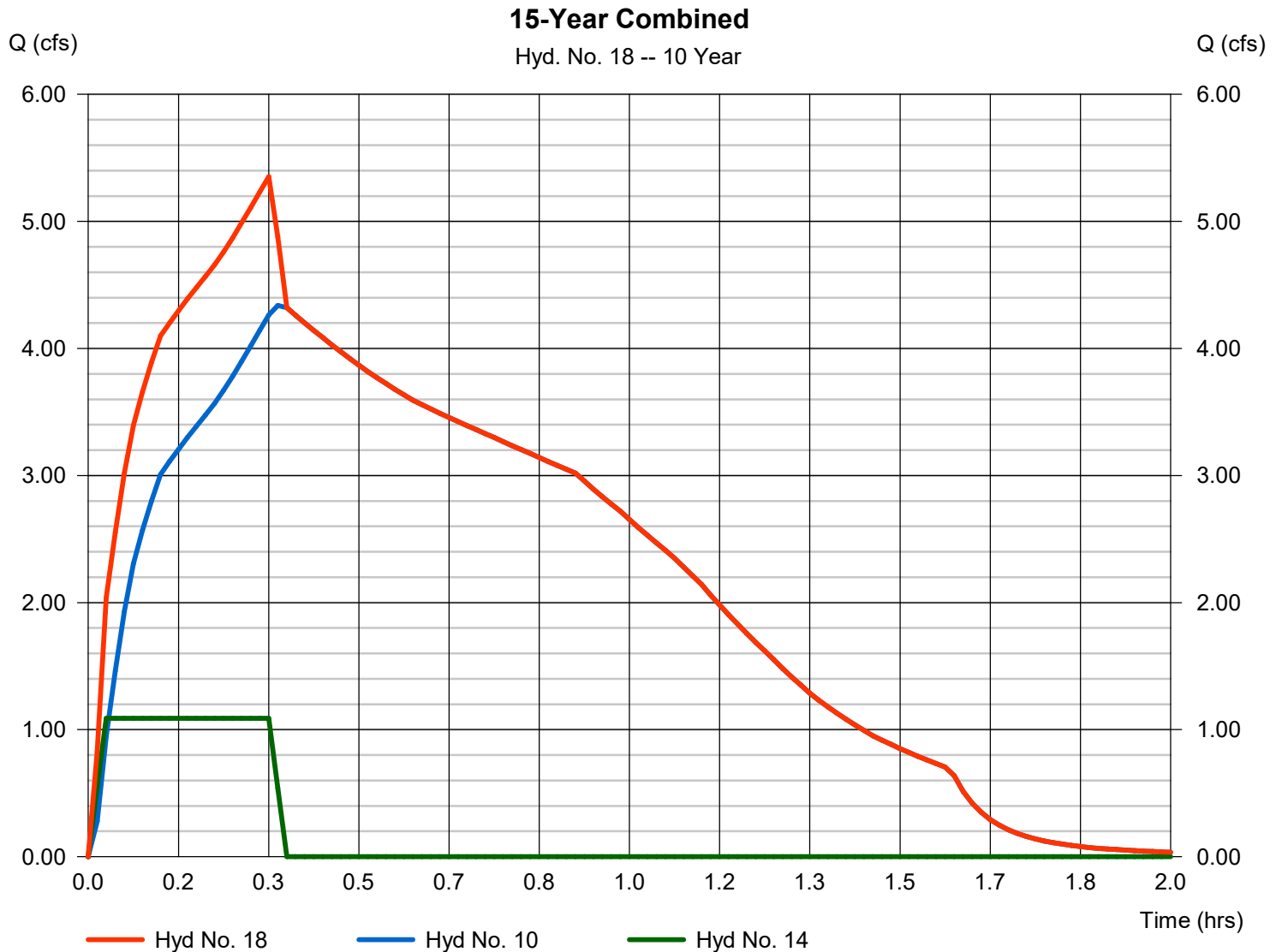
Hydrograph Report

Hyd. No. 18

15-Year Combined

Hydrograph type = Combine
Storm frequency = 10 yrs
Time interval = 1 min
Inflow hyds. = 10, 14

Peak discharge = 5.353 cfs
Time to peak = 0.33 hrs
Hyd. volume = 16,700 cuft
Contrib. drain. area = 0.000 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Manual	0.000	1	n/a	0	----	----	----	Pre-Developed 2-Year
2	Manual	0.000	1	n/a	0	----	----	----	Post-Dev 2-Year to Basin
3	Manual	0.000	1	n/a	0	----	----	----	Pre-Developed 15-Year
4	Manual	0.000	1	n/a	0	----	----	----	Post-Development 15-Year to Basin
5	Manual	8.090	6	12	58,248	----	----	----	Pre-Developed 25-Year
6	Manual	13.73	1	2	16,476	----	----	----	Post-Dev 25-Year to Basin
7	Manual	0.000	1	n/a	0	----	----	----	Pre-Developed 100-Year
8	Manual	0.000	1	n/a	0	----	----	----	Post-Dev 100-Year
9	Reservoir	0.000	1	n/a	0	2	486.57	0.000	2 Year Basin Routing
10	Reservoir	0.000	1	n/a	0	4	486.57	0.000	15 Year Routing
11	Reservoir	4.544	1	21	16,472	6	488.88	12,455	25 Year Routing
12	Reservoir	0.000	1	n/a	0	8	486.57	0.000	100 Year Routing
13	Manual	0.000	1	n/a	0	----	----	----	2-Year Basin By-Pass
14	Manual	0.000	1	n/a	0	----	----	----	15-Year Basin By-Pass
15	Manual	1.160	1	2	1,392	----	----	----	25-Year Basin By-Pass
16	Manual	0.000	1	n/a	0	----	----	----	100-Year Basin By-Pass
17	Combine	0.000	1	n/a	0	9, 13,	----	----	2 Year Combined
18	Combine	0.000	1	n/a	0	10, 14,	----	----	15-Year Combined
19	Combine	5.629	1	20	17,864	11, 15,	----	----	25-Year Combined
20	Combine	0.000	1	n/a	0	12, 16,	----	----	100-Year Combined
Basin Hydraulics.gpw					Return Period: 25 Year			Friday, 09 / 30 / 2022	

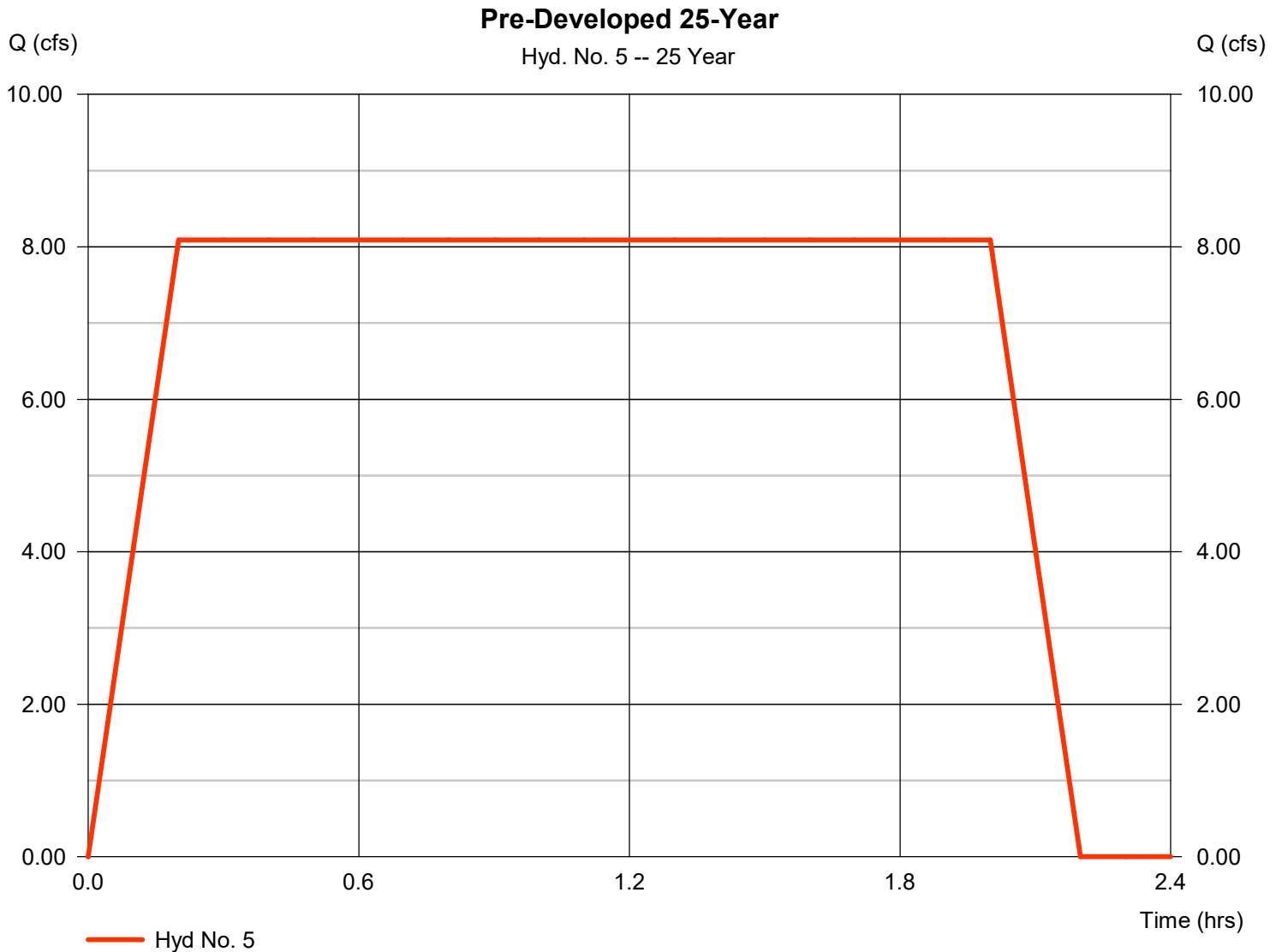
Hydrograph Report

Hyd. No. 5

Pre-Developed 25-Year

Hydrograph type = Manual
Storm frequency = 25 yrs
Time interval = 6 min

Peak discharge = 8.090 cfs
Time to peak = 0.20 hrs
Hyd. volume = 58,248 cuft



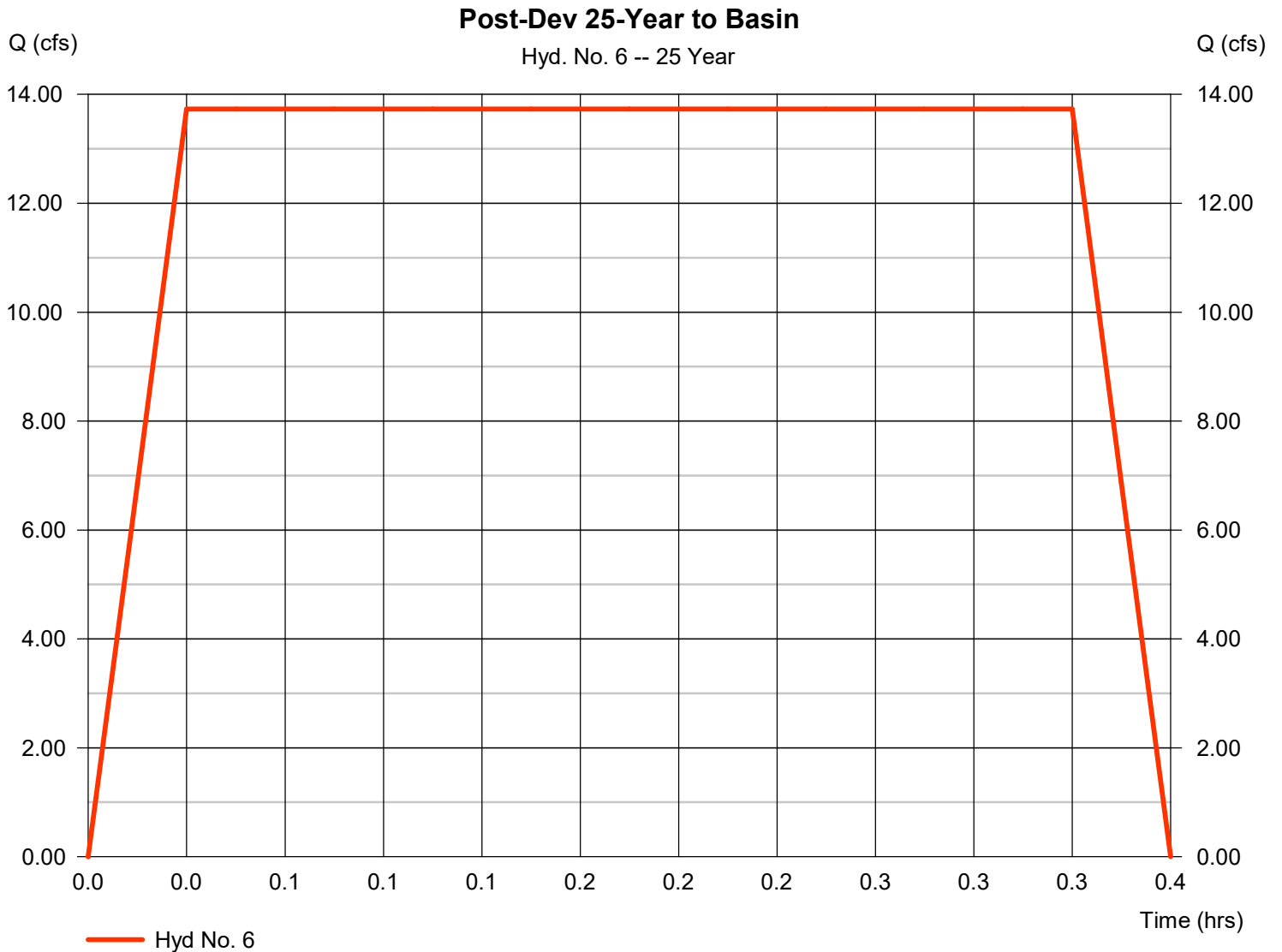
Hydrograph Report

Hyd. No. 6

Post-Dev 25-Year to Basin

Hydrograph type = Manual
Storm frequency = 25 yrs
Time interval = 1 min

Peak discharge = 13.73 cfs
Time to peak = 0.03 hrs
Hyd. volume = 16,476 cuft



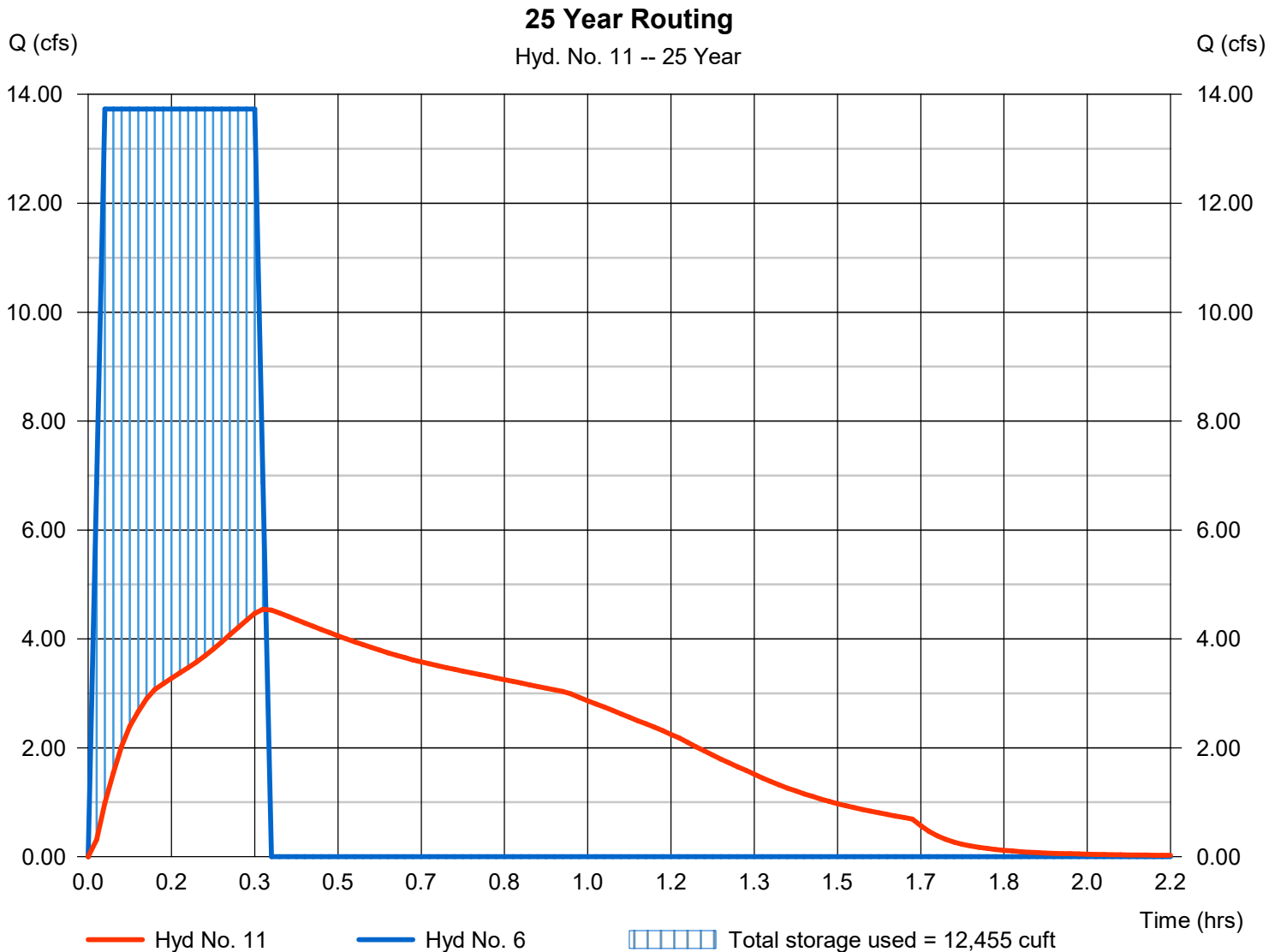
Hydrograph Report

Hyd. No. 11

25 Year Routing

Hydrograph type	= Reservoir	Peak discharge	= 4.544 cfs
Storm frequency	= 25 yrs	Time to peak	= 0.35 hrs
Time interval	= 1 min	Hyd. volume	= 16,472 cuft
Inflow hyd. No.	= 6 - Post-Dev 25-Year to Basin	Max. Elevation	= 488.88 ft
Reservoir name	= Basin	Max. Storage	= 12,455 cuft

Storage Indication method used.



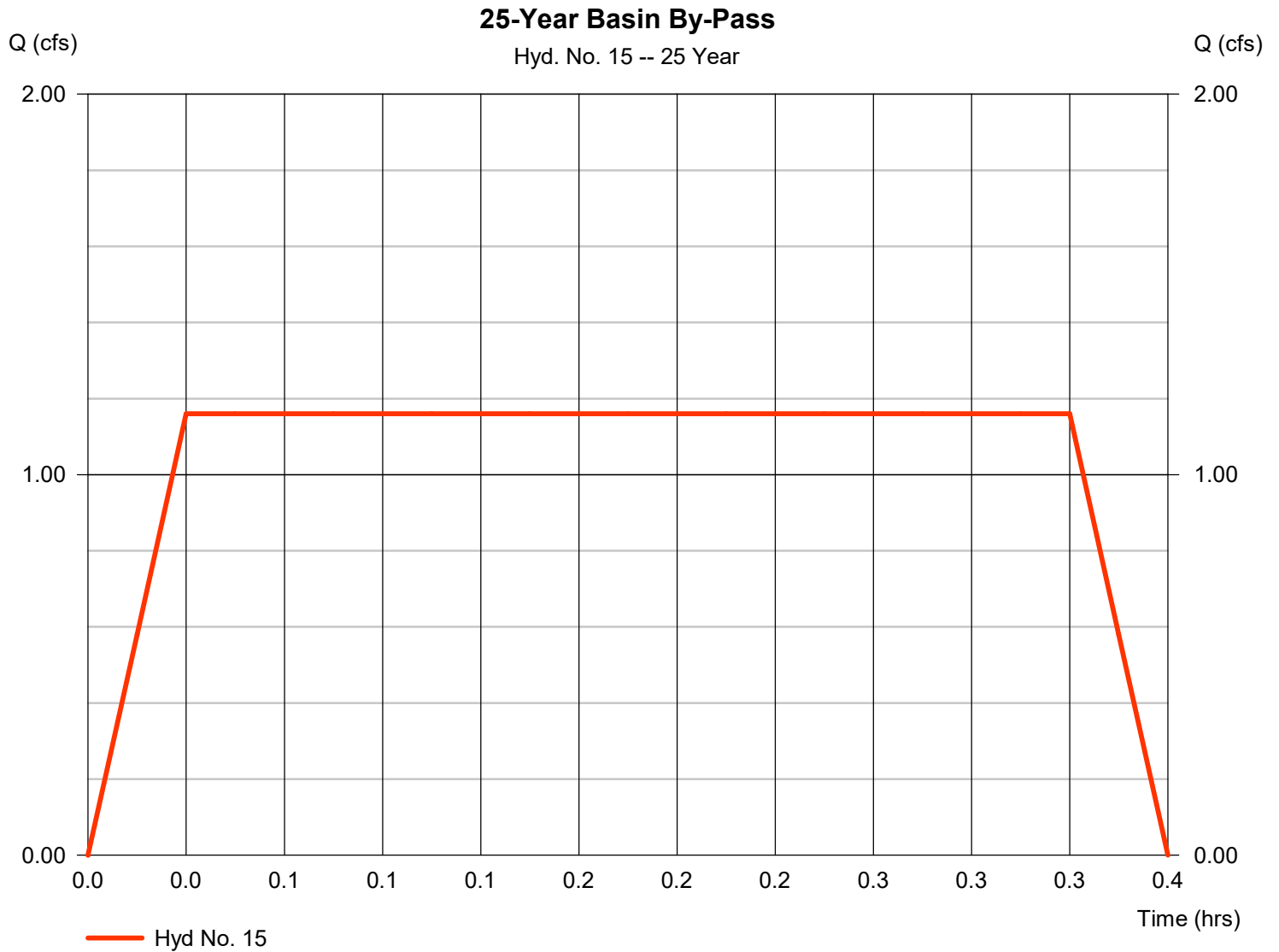
Hydrograph Report

Hyd. No. 15

25-Year Basin By-Pass

Hydrograph type = Manual
Storm frequency = 25 yrs
Time interval = 1 min

Peak discharge = 1.160 cfs
Time to peak = 0.03 hrs
Hyd. volume = 1,392 cuft



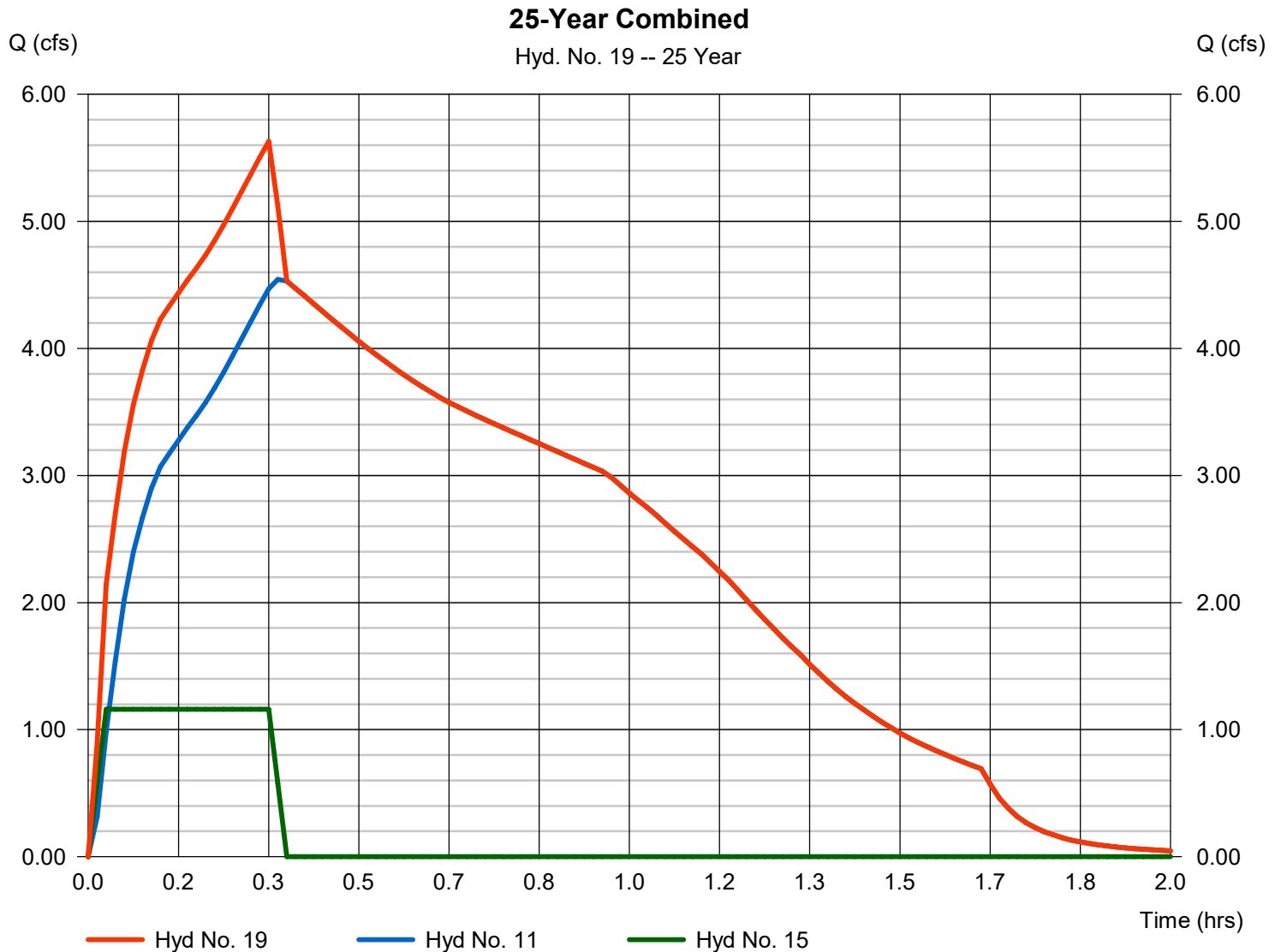
Hydrograph Report

Hyd. No. 19

25-Year Combined

Hydrograph type = Combine
Storm frequency = 25 yrs
Time interval = 1 min
Inflow hyds. = 11, 15

Peak discharge = 5.629 cfs
Time to peak = 0.33 hrs
Hyd. volume = 17,864 cuft
Contrib. drain. area = 0.000 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Manual	0.000	1	n/a	0	----	----	----	Pre-Developed 2-Year
2	Manual	0.000	1	n/a	0	----	----	----	Post-Dev 2-Year to Basin
3	Manual	0.000	1	n/a	0	----	----	----	Pre-Developed 15-Year
4	Manual	0.000	1	n/a	0	----	----	----	Post-Development 15-Year to Basin
5	Manual	0.000	6	n/a	0	----	----	----	Pre-Developed 25-Year
6	Manual	0.000	1	n/a	0	----	----	----	Post-Dev 25-Year to Basin
7	Manual	10.24	1	2	12,288	----	----	----	Pre-Developed 100-Year
8	Manual	17.28	1	2	20,736	----	----	----	Post-Dev 100-Year
9	Reservoir	0.000	1	n/a	0	2	486.57	0.000	2 Year Basin Routing
10	Reservoir	0.000	1	n/a	0	4	486.57	0.000	15 Year Routing
11	Reservoir	0.000	1	n/a	0	6	486.57	0.000	25 Year Routing
12	Reservoir	7.589	1	21	20,732	8	489.18	15,865	100 Year Routing
13	Manual	0.000	1	n/a	0	----	----	----	2-Year Basin By-Pass
14	Manual	0.000	1	n/a	0	----	----	----	15-Year Basin By-Pass
15	Manual	0.000	1	n/a	0	----	----	----	25-Year Basin By-Pass
16	Manual	1.470	1	2	1,764	----	----	----	100-Year Basin By-Pass
17	Combine	0.000	1	n/a	0	9, 13,	----	----	2 Year Combined
18	Combine	0.000	1	n/a	0	10, 14,	----	----	15-Year Combined
19	Combine	0.000	1	n/a	0	11, 15,	----	----	25-Year Combined
20	Combine	8.527	1	20	22,496	12, 16,	----	----	100-Year Combined
Basin Hydraulics.gpw					Return Period: 100 Year			Friday, 09 / 30 / 2022	

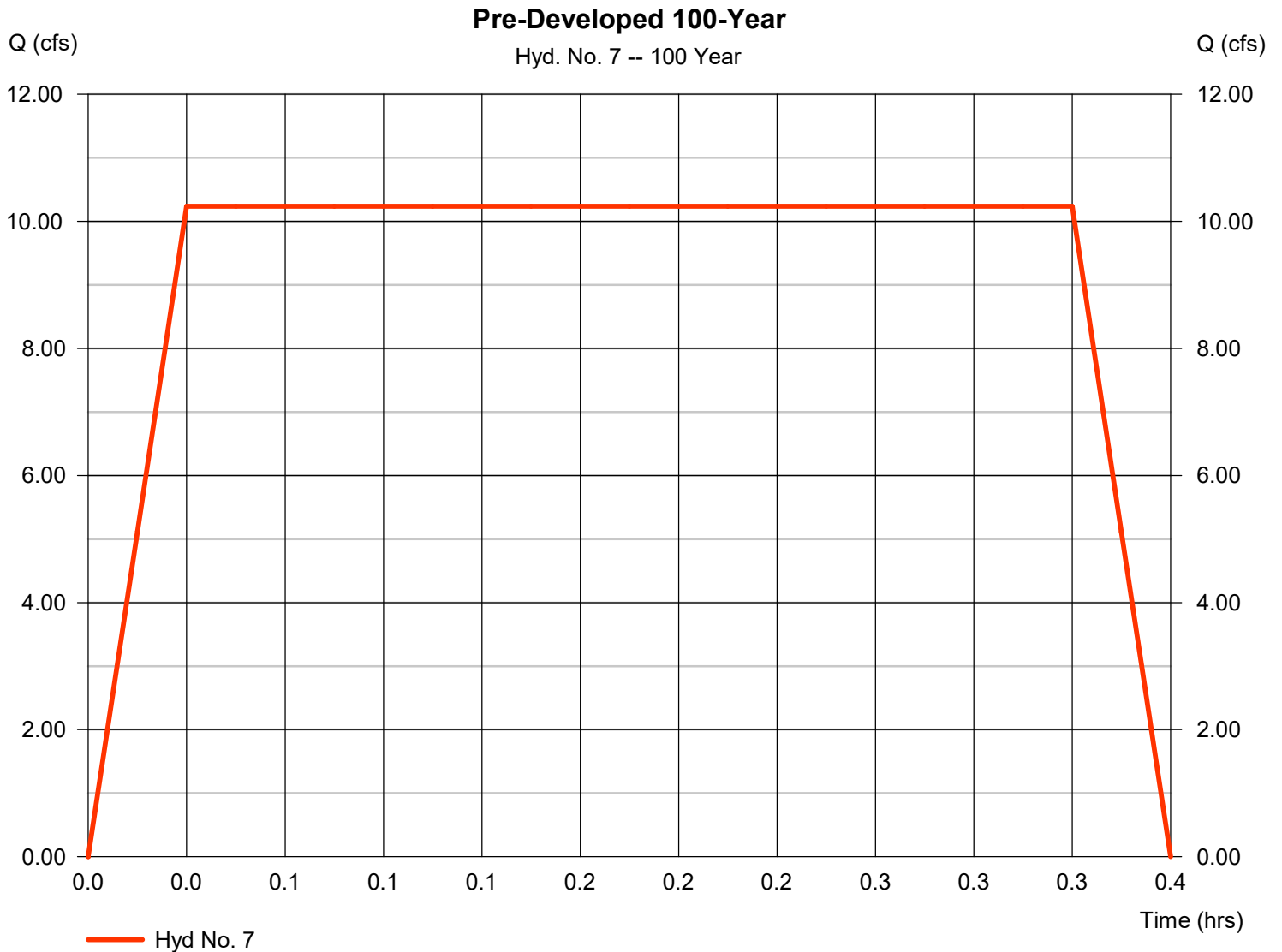
Hydrograph Report

Hyd. No. 7

Pre-Developed 100-Year

Hydrograph type = Manual
Storm frequency = 100 yrs
Time interval = 1 min

Peak discharge = 10.24 cfs
Time to peak = 0.03 hrs
Hyd. volume = 12,288 cuft



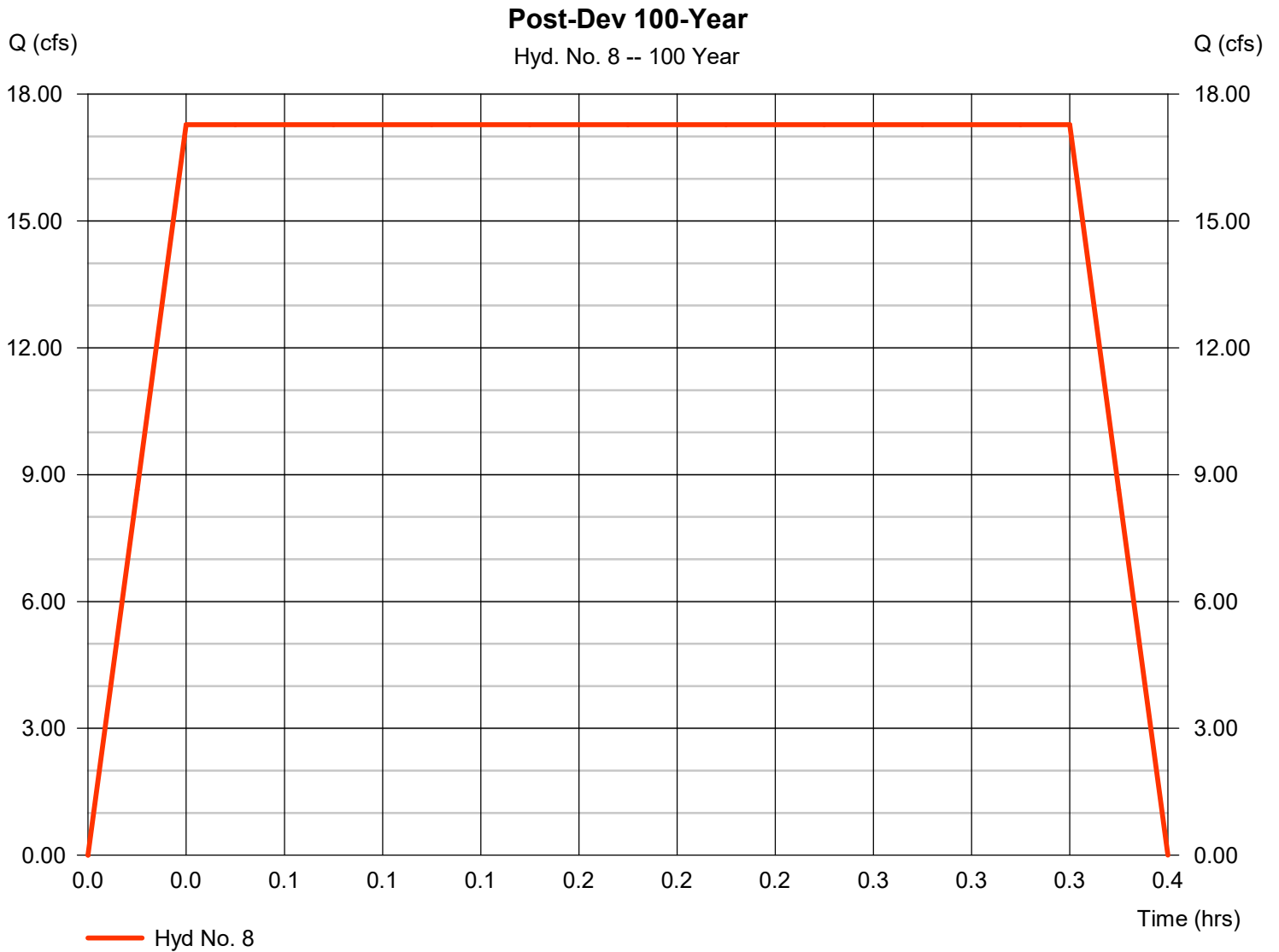
Hydrograph Report

Hyd. No. 8

Post-Dev 100-Year

Hydrograph type = Manual
Storm frequency = 100 yrs
Time interval = 1 min

Peak discharge = 17.28 cfs
Time to peak = 0.03 hrs
Hyd. volume = 20,736 cuft



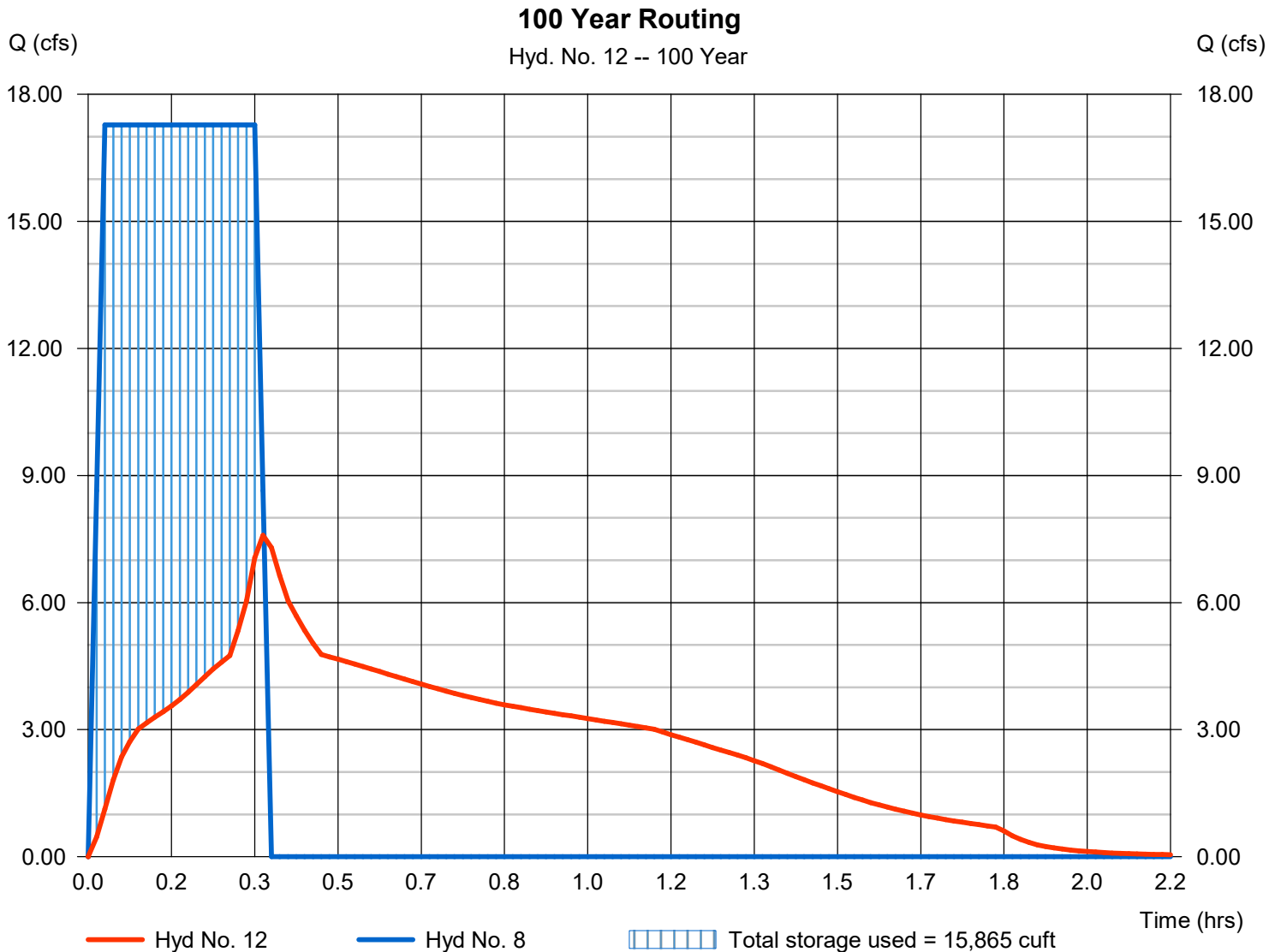
Hydrograph Report

Hyd. No. 12

100 Year Routing

Hydrograph type	= Reservoir	Peak discharge	= 7.589 cfs
Storm frequency	= 100 yrs	Time to peak	= 0.35 hrs
Time interval	= 1 min	Hyd. volume	= 20,732 cuft
Inflow hyd. No.	= 8 - Post-Dev 100-Year	Max. Elevation	= 489.18 ft
Reservoir name	= Basin	Max. Storage	= 15,865 cuft

Storage Indication method used.



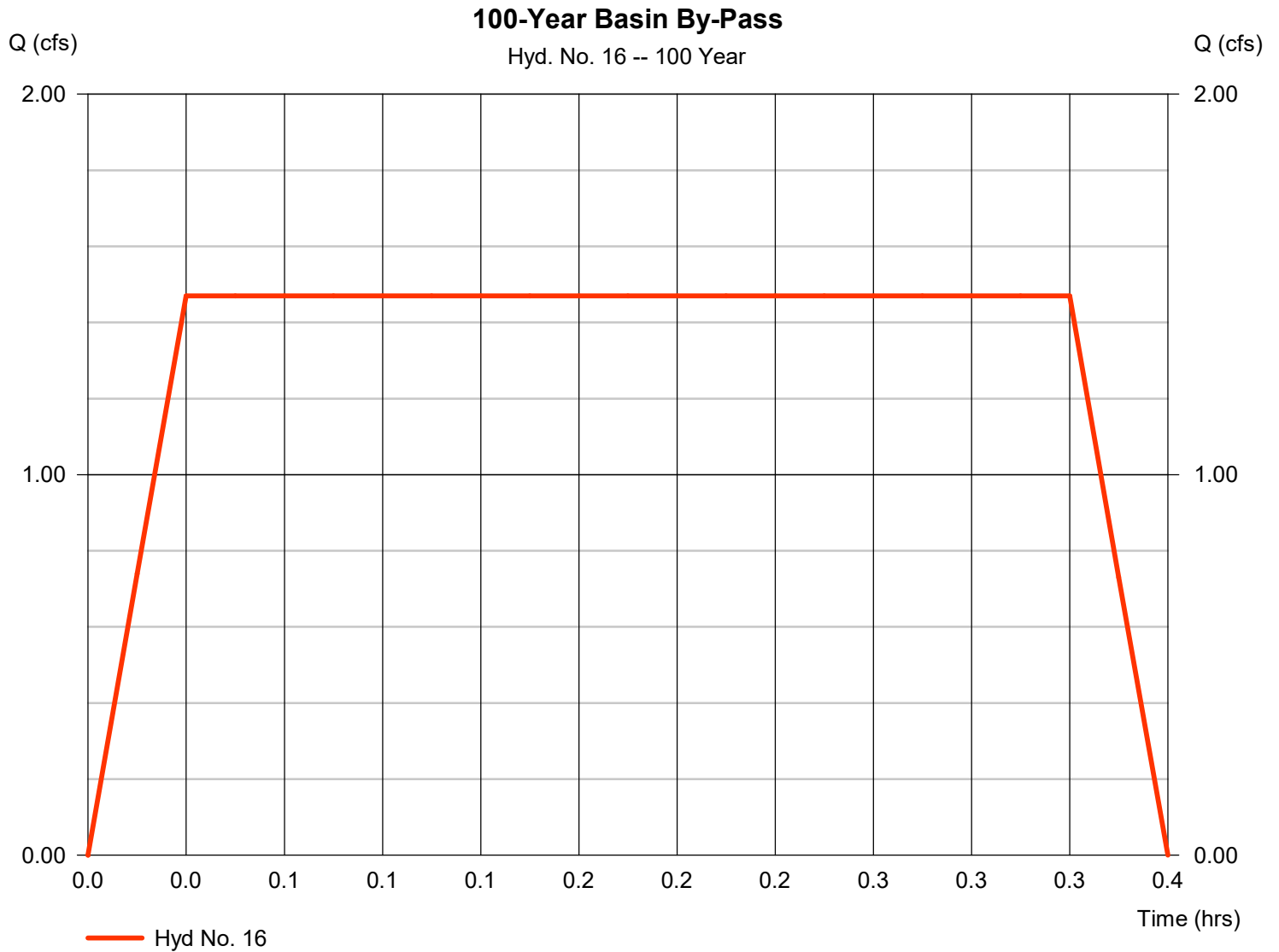
Hydrograph Report

Hyd. No. 16

100-Year Basin By-Pass

Hydrograph type = Manual
Storm frequency = 100 yrs
Time interval = 1 min

Peak discharge = 1.470 cfs
Time to peak = 0.03 hrs
Hyd. volume = 1,764 cuft



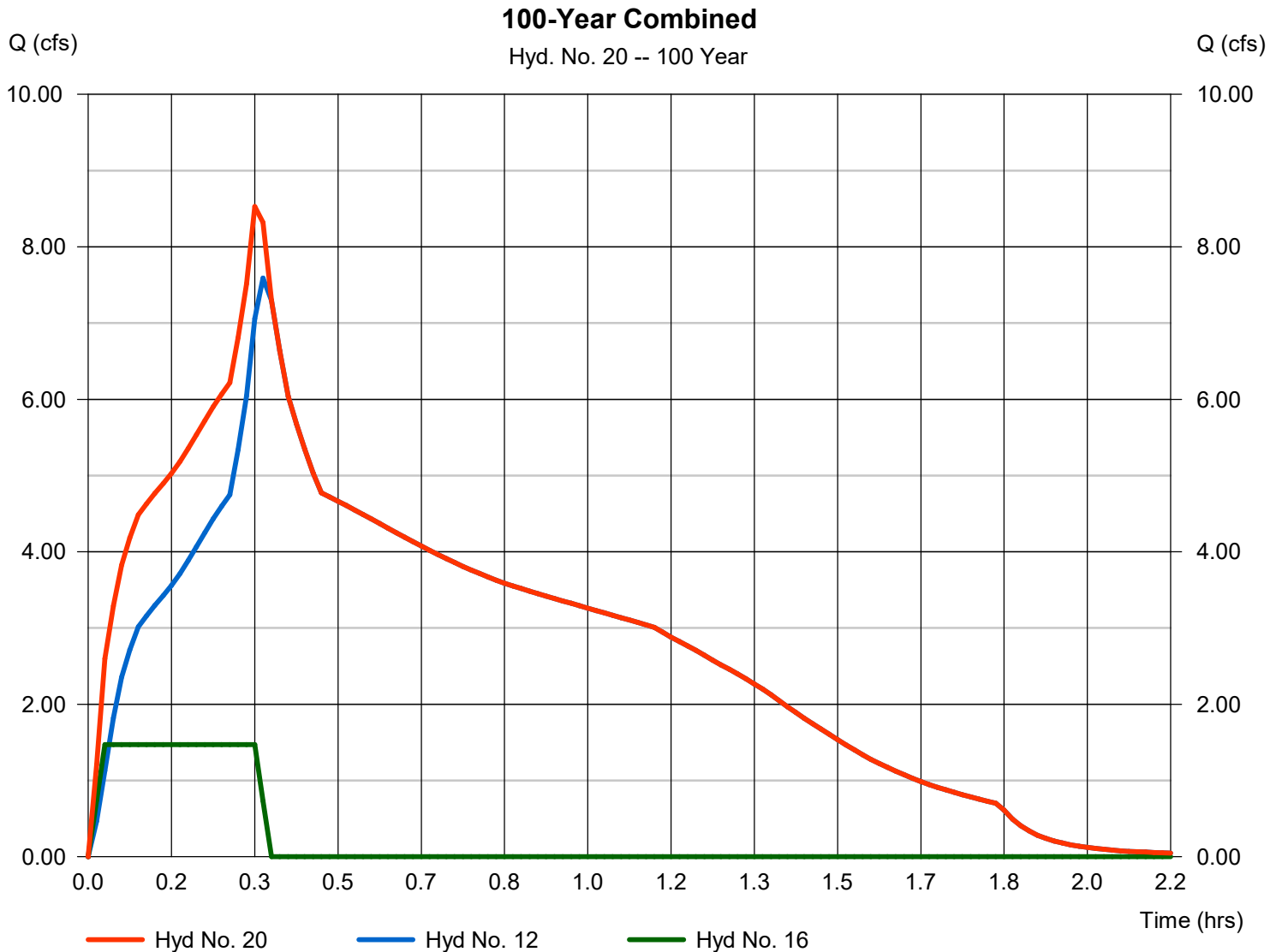
Hydrograph Report

Hyd. No. 20

100-Year Combined

Hydrograph type = Combine
Storm frequency = 100 yrs
Time interval = 1 min
Inflow hyds. = 12, 16

Peak discharge = 8.527 cfs
Time to peak = 0.33 hrs
Hyd. volume = 22,496 cuft
Contrib. drain. area = 0.000 ac



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Friday, 09 / 30 / 2022

Hyd. No. 12

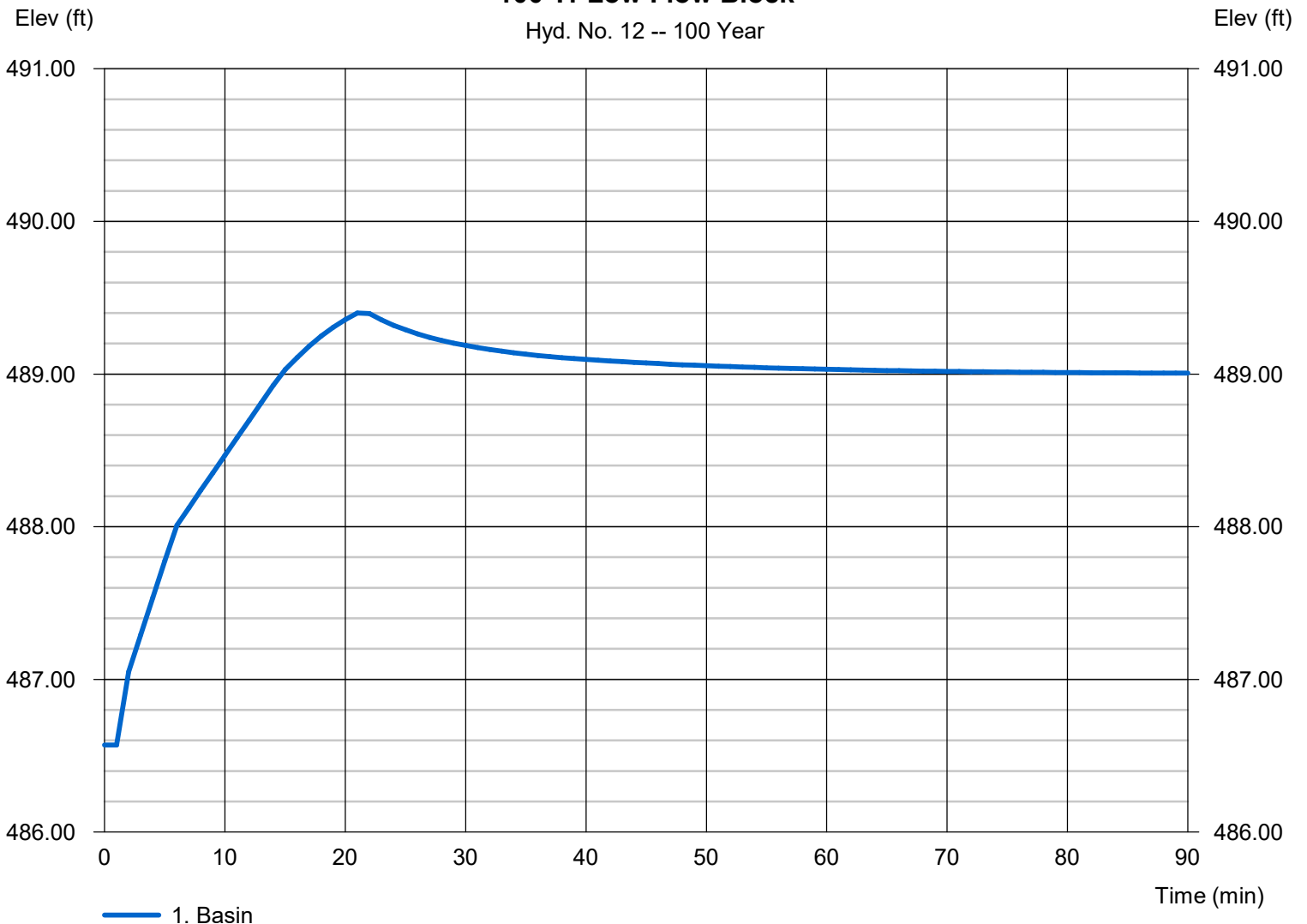
100 Yr Low Flow Block

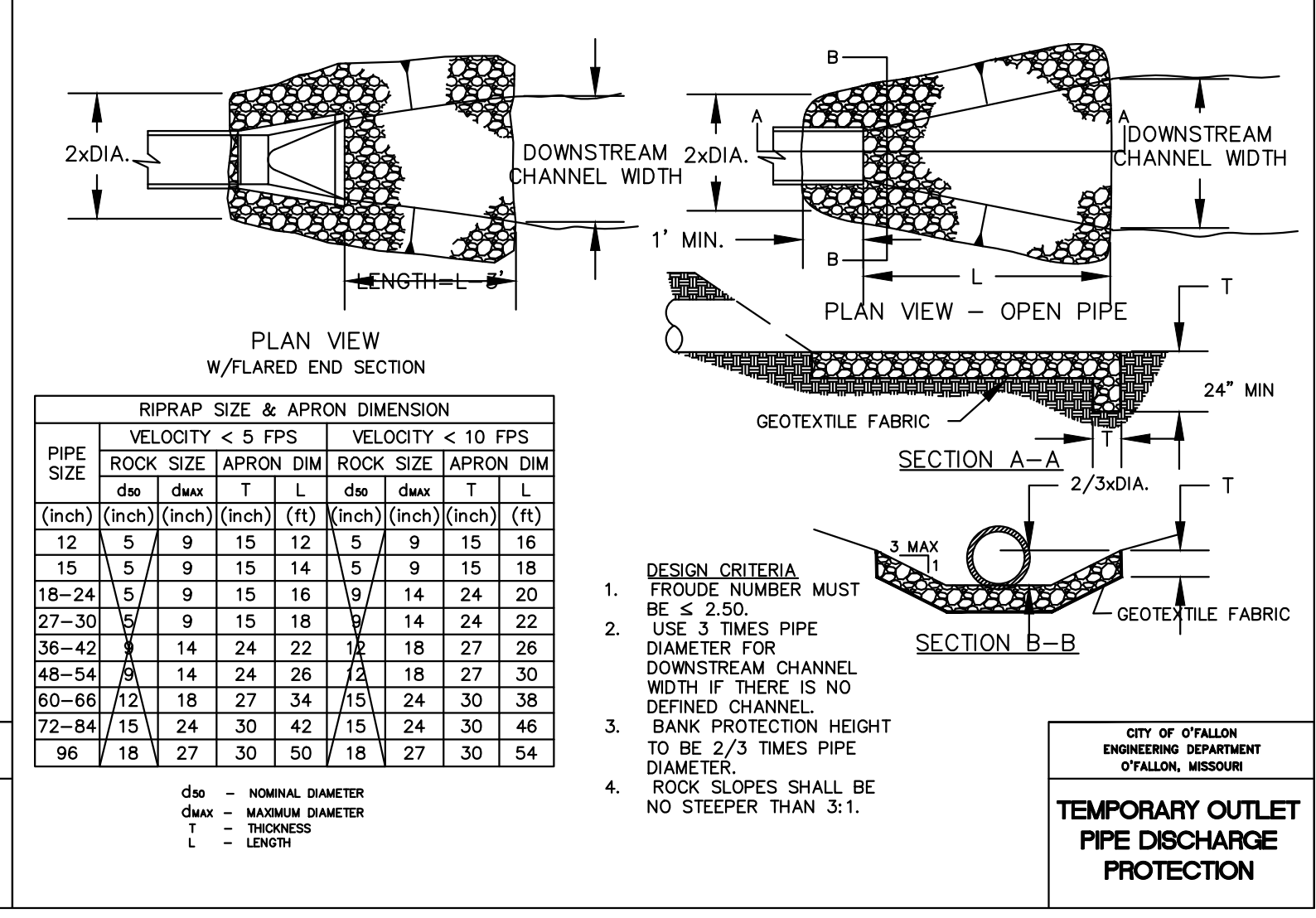
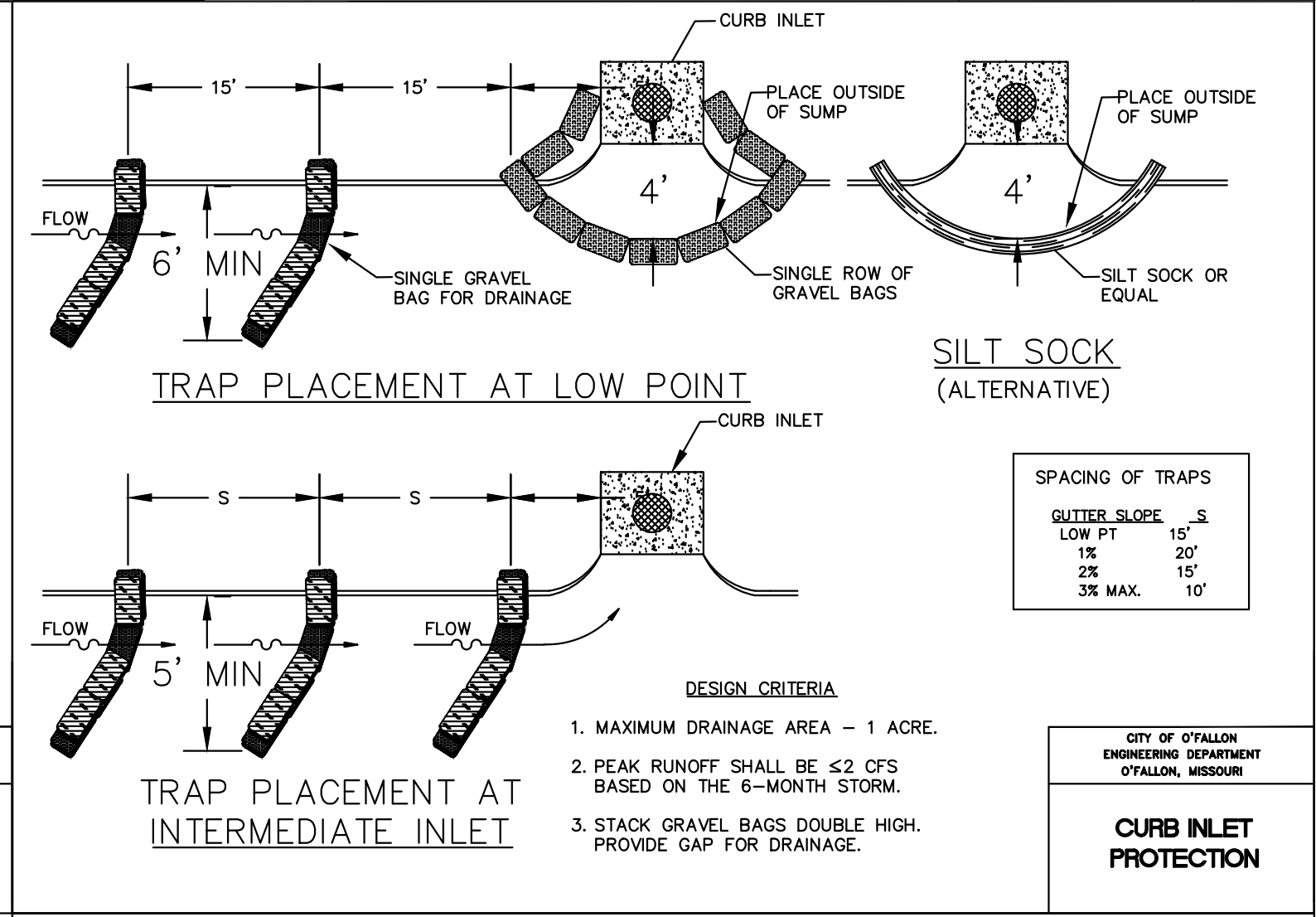
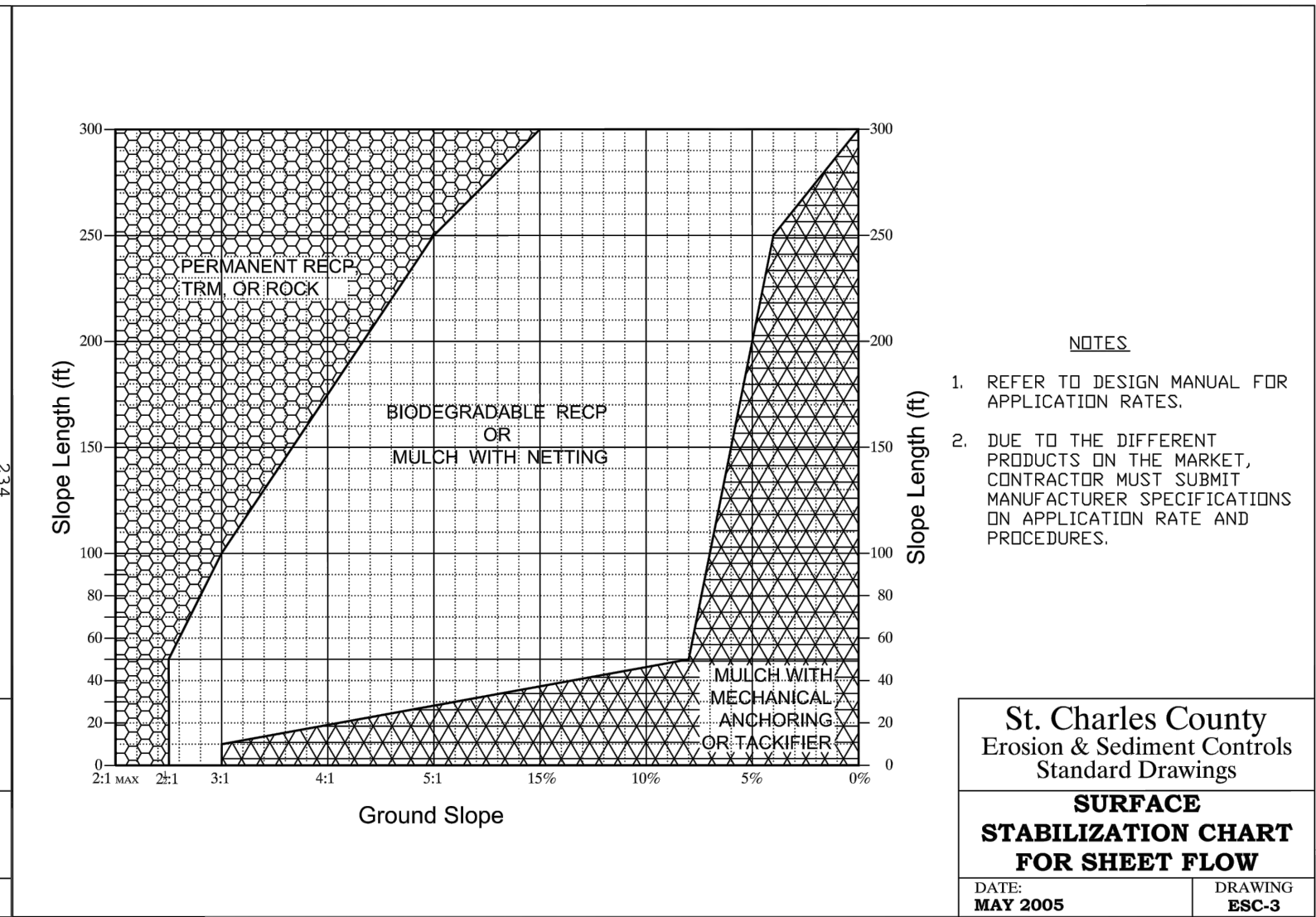
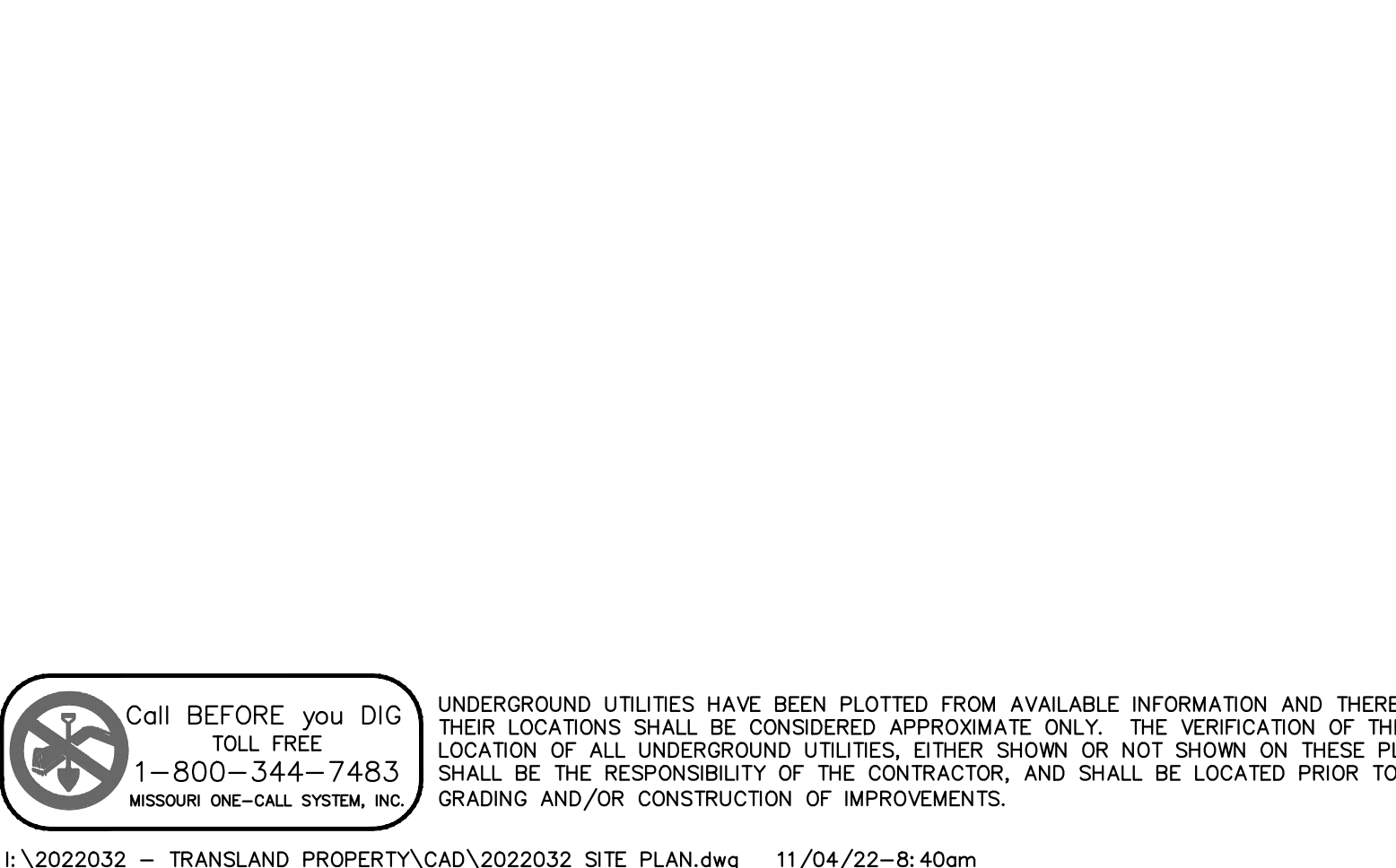
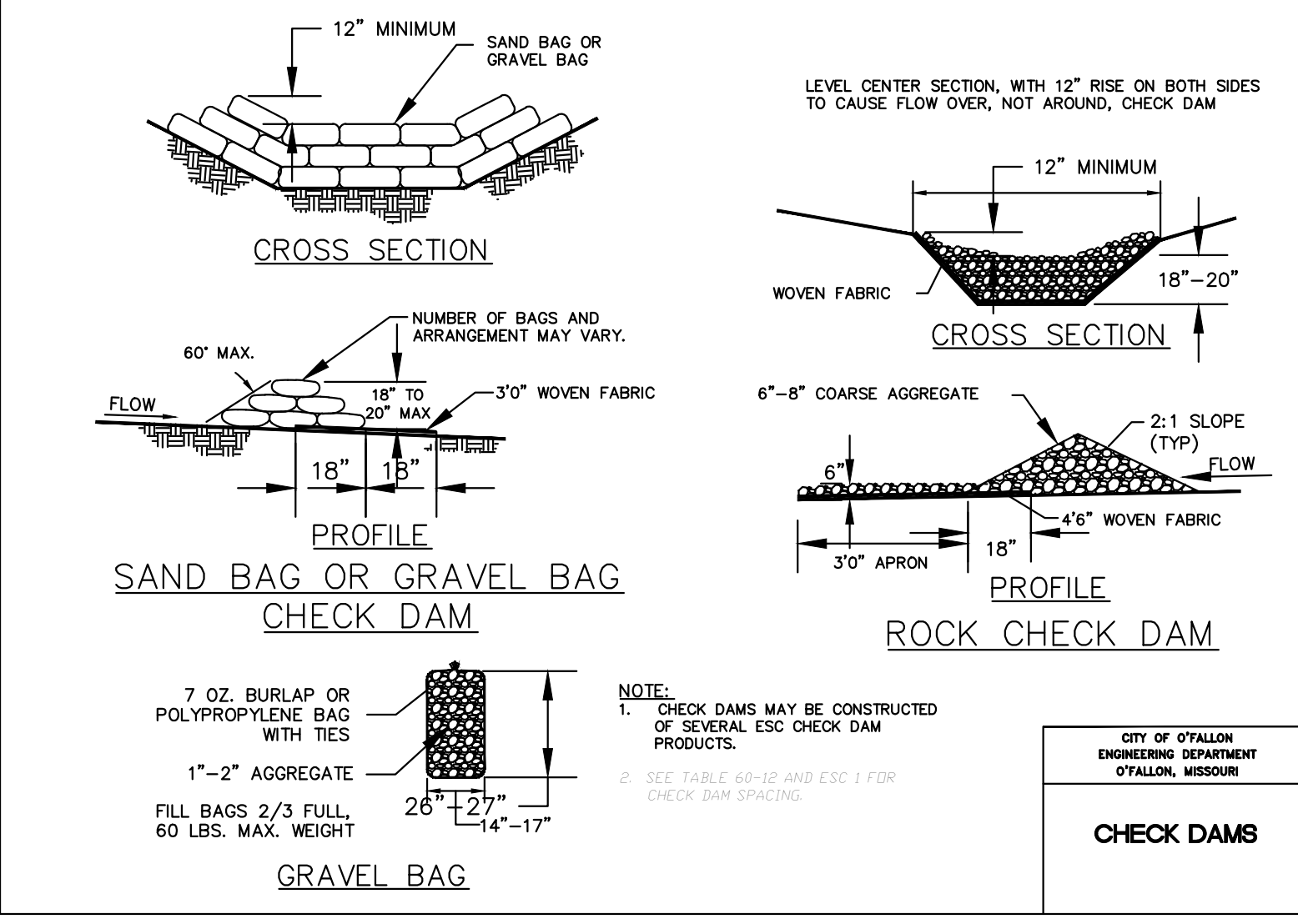
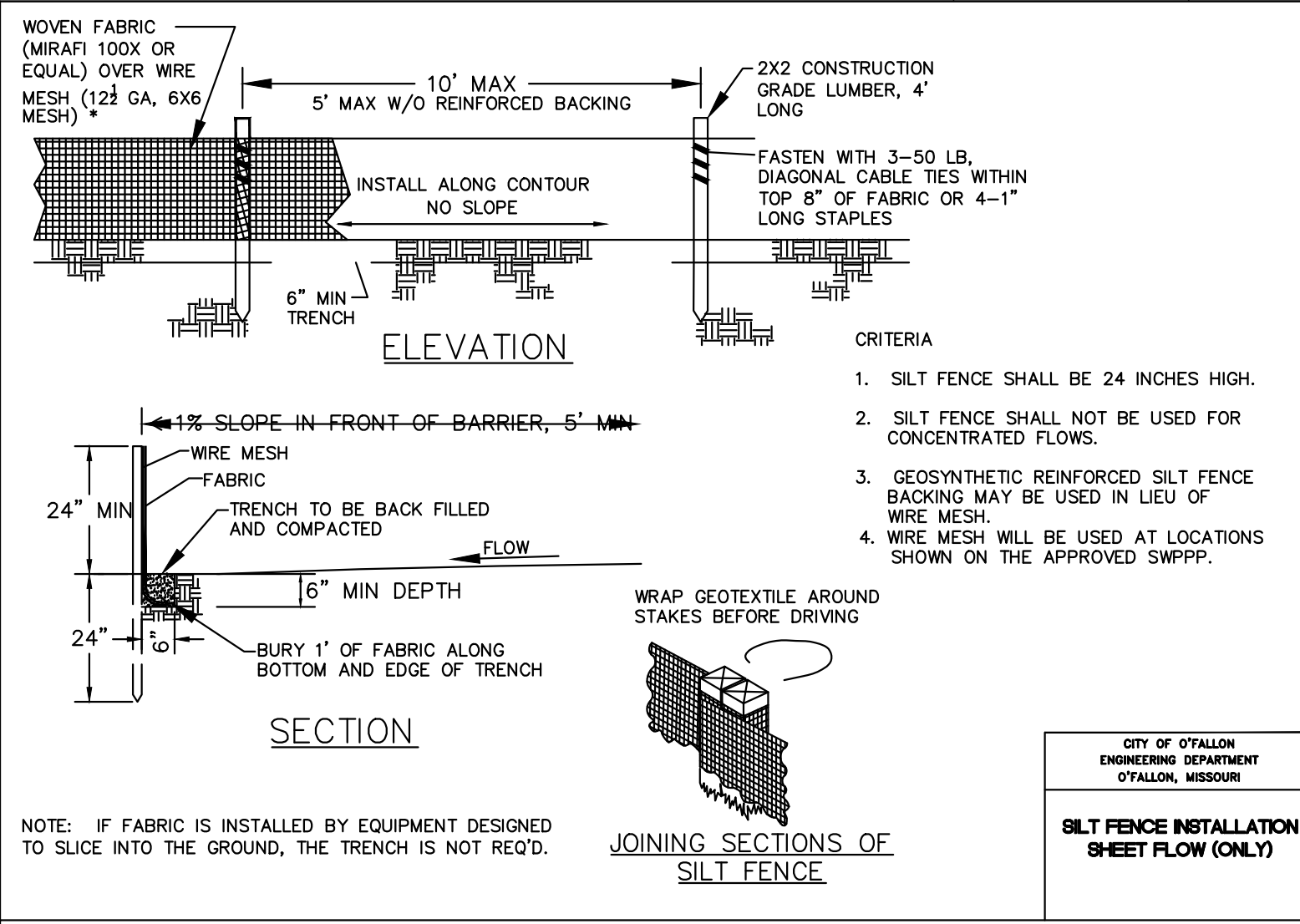
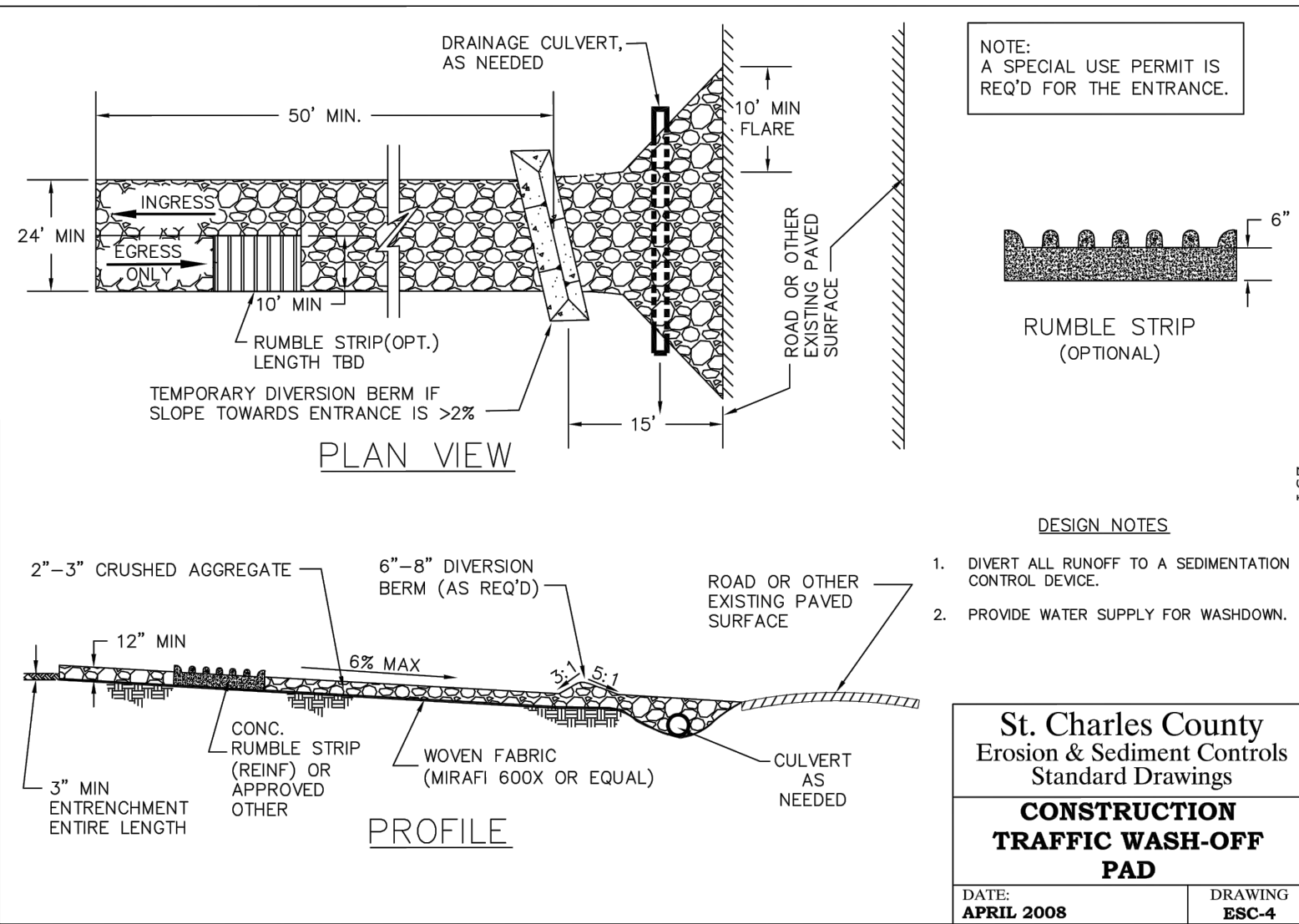
Hydrograph type	= Reservoir	Peak discharge	= 9.194 cfs
Storm frequency	= 100 yrs	Time to peak	= 21 min
Time interval	= 1 min	Hyd. volume	= 7,096 cuft
Inflow hyd. No.	= 8 - Post-Dev 100-Year	Max. Elevation	= 489.40 ft
Reservoir name	= Basin	Max. Storage	= 18,541 cuft

Storage Indication method used.

100 Yr Low Flow Block

Hyd. No. 12 -- 100 Year





DESIGN CRITERIA

- FROUDE NUMBER MUST BE ≤ 2.50 .
- USE 3 TIMES PIPE DIAMETER FOR DOWNSTREAM CHANNEL WIDTH IF THERE IS NO DEFINED CHANNEL.
- BANK PROTECTION HEIGHT TO BE $2/3$ TIMES PIPE DIAMETER.
- ROCK SLOPES SHALL BE NO STEEPER THAN 3:1.

PIPE SIZE (inch)	VELOCITY < 5 FPS			VELOCITY < 10 FPS		
	ROCK SIZE (inch)	APRON DIM (inch)	ROCK SIZE (inch)	APRON DIM (inch)	ROCK SIZE (inch)	APRON DIM (inch)
12	5	9	15	12	5	9
15	5	9	15	14	5	9
18-24	5	9	15	16	9	14
27-30	5	9	15	18	9	14
36-42	9	14	24	22	12	18
48-54	9	14	24	26	14	24
60-66	12	18	27	34	15	24
72-84	15	24	30	42	15	24
96	18	27	30	50	18	27

proposed prior to planting, as recommended by an agronomist, competent nursery company, or refer to NRCS MOFOTG Code 340 (Cover Crop) in Appendix D.

For channels, embankments, and slopes of 4:1 or steeper, seeding shall be a mixture of K31 fescue and rye at a rate of 400 pounds per acre.

Table 60-7 Temporary Fall Seeding

Plant Species	Rate (lb/acre)	Seeding Times
Side-Oats	65	8/16 - 9/30
Winter Rye	50	8/01 - 10/15
Winter Wheat	60	8/01 - 10/15
Orchard Grass	120	8/01 - 10/15
Perennial Ryegrass	80	8/01 - 10/15
Tall fescue, Smooth Brome	80	8/01 - 10/15
K-31 Fescue	120	9/01 - 11/15
Ladino Clover	2 ²	8/15 - 9/15
Crimson Clover	6 ²	8/15 - 9/15
Orchard Grass and Oats or Rye	40 ²	8/15 - 9/15

Table 60-8 Temporary Spring Seeding

Plant Species	Rate (lb/acre)	Seeding Dates
Winter Rye	50	3/15 - 5/31
Spring Oats	65	3/15 - 5/31
Annual Ryegrass	4 ²	3/15 - 6/15
Sudangrass	16 ²	4/15 - 6/15
K-31 Fescue	30 ²	3/15 - 5/31
Red Clover & Oats	2 ²	3/15 - 5/31

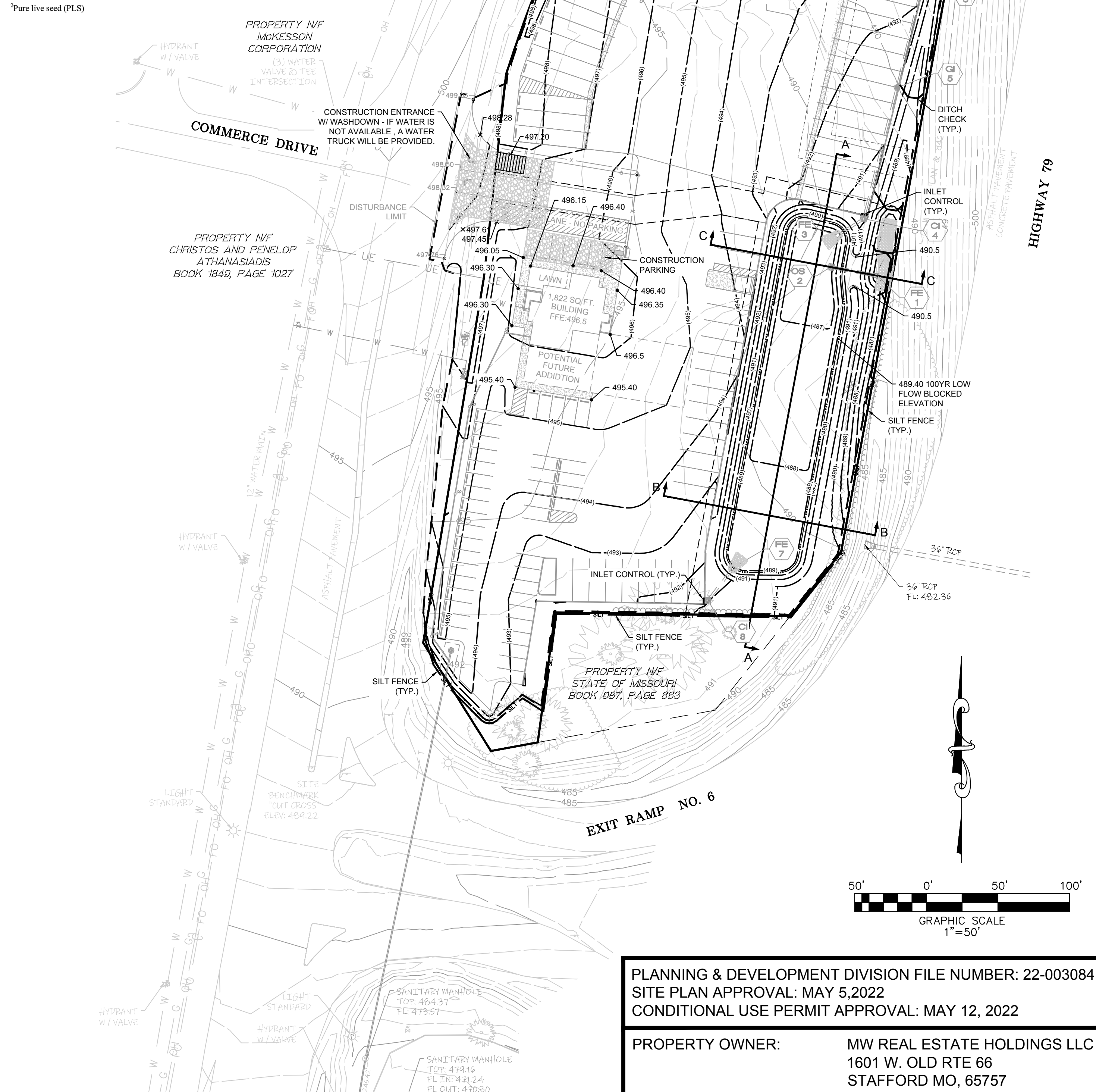
NOTES

- REFER TO DESIGN MANUAL FOR APPLICATION RATES.
- DUE TO THE DIFFERENT PRODUCTS ON THE MARKET, CONTRACTOR MUST SUBMIT MANUFACTURER SPECIFICATIONS ON APPLICATION RATE AND PROCEDURES.

St. Charles County Erosion & Sediment Controls Standard Drawings

SURFACE STABILIZATION CHART FOR SHEET FLOW

DATE: MAY 2005 DRAWING: BSC-3



1ST SUBMITTAL: 08-12-2022
 GRADING PLAN SUBMITTAL: 09-30-2022
 2ND SUBMITTAL: 09-30-2022
 2ND GRADING PLAN SUBMITTAL: 10-18-2022
 MDDOT SUBMITTAL: 10-27-2022
 3RD SUBMITTAL: 11-03-2022

CONSTRUCTION PLANS
TRANSLAND TRUCKING
 3480 E. TERRA LANE
 O'FALLON MO, 63366
 GRADING PLAN & SWPPP

ST. CHARLES ENGINEERING & SURVEYING, INC.
 801 S. FIFTH STREET, SUITE 202
 ST. CHARLES, MO 63301
 TEL: (636) 947-0607 FAX: (636) 947-2448

ST. CHARLES ENGINEERING AND SURVEYING, INC.
 A PROFESSIONAL ENGINEERING AND SURVEYING CORPORATION
 MISSOURI STATE CERTIFICATE OF AUTHORITY: 001647 & 000379

STATE OF MISSOURI
 MICHAEL NEWELL
 MEMBER
 NUMBER
 E-22483
 PROFESSIONAL ENGINEER
 LICENSE NUMBER
 110522

PLANNING & DEVELOPMENT DIVISION FILE NUMBER: 22-003084
SITE PLAN APPROVAL: MAY 5, 2022
CONDITIONAL USE PERMIT APPROVAL: MAY 12, 2022

PROPERTY OWNER: MW REAL ESTATE HOLDINGS LLC
 1601 W. OLD RTE 66
 STAFFORD MO, 65757

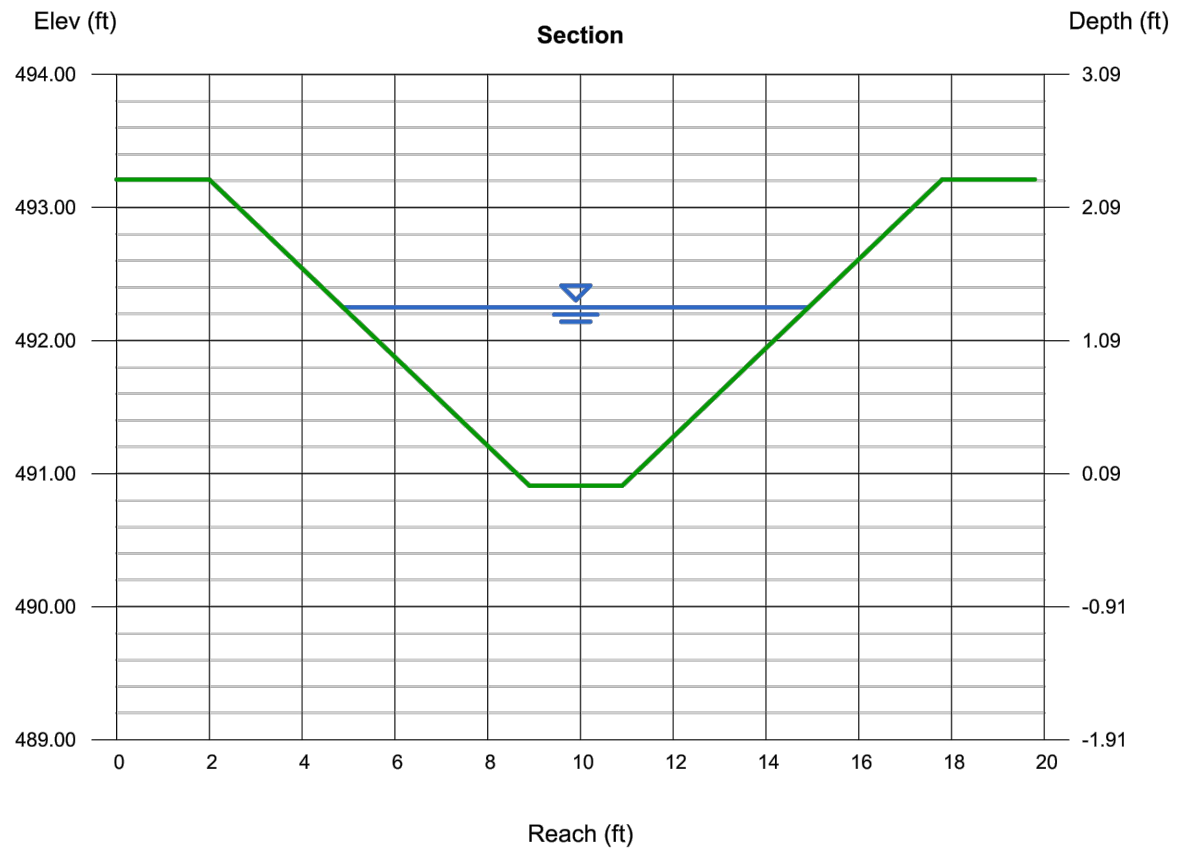
ORDER NO. 2022032
DATE 11/03/2022

4

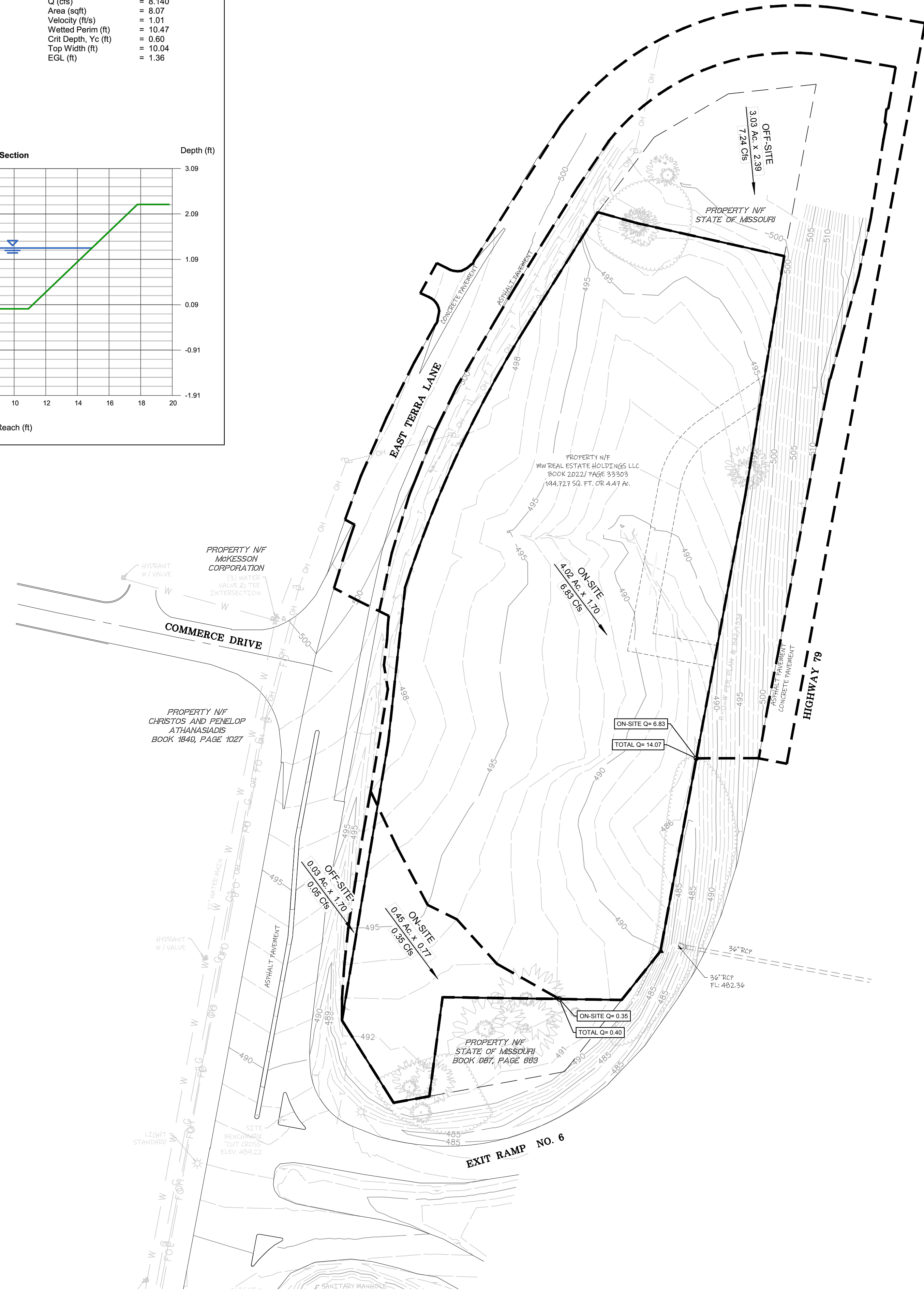
Channel Report
Hydroflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Thursday, Nov 3 2022

SECTION D-D

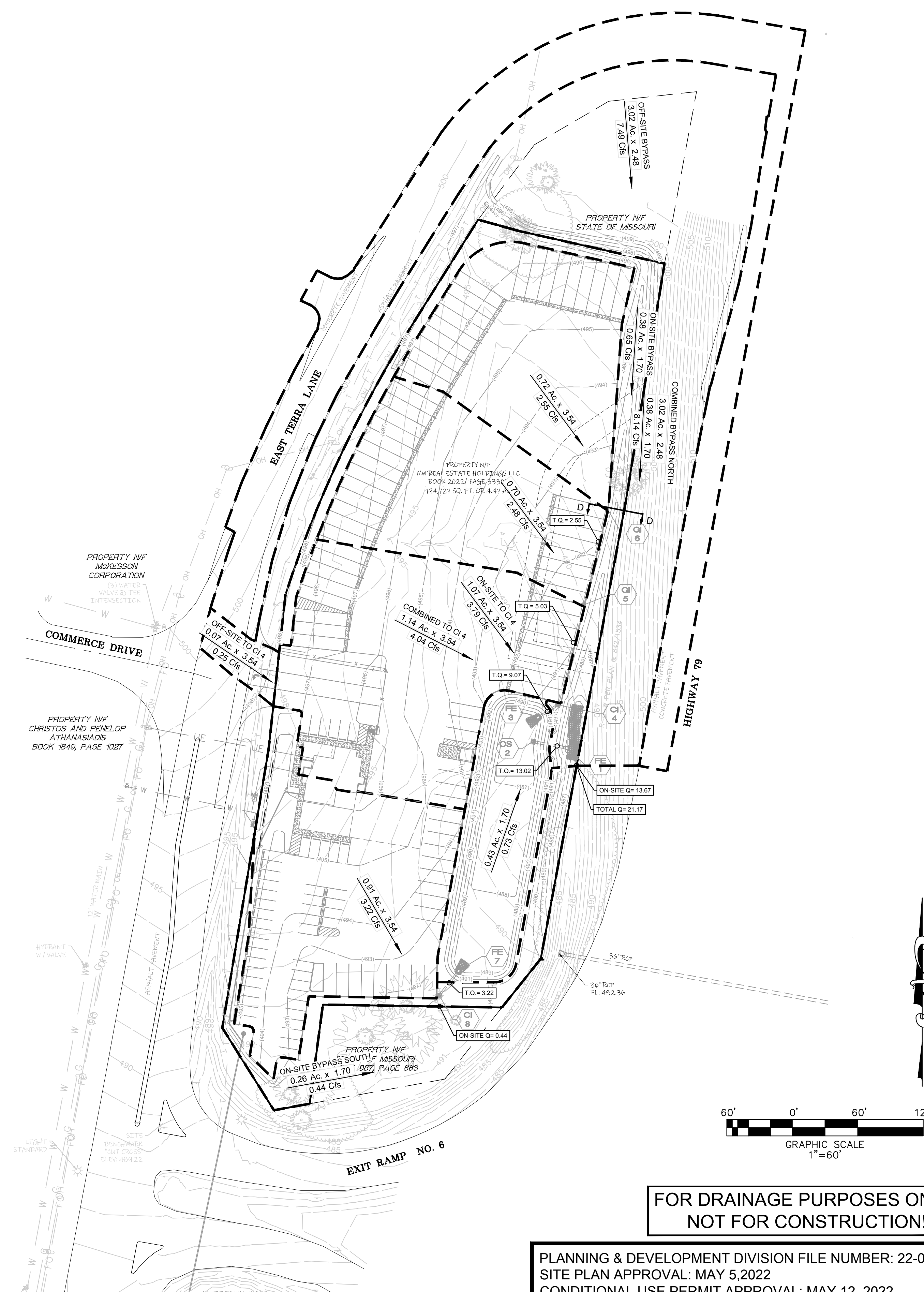
Trapezoidal		Highlighted	
Bottom Width (ft)	= 2.00	Depth (ft)	= 1.34
Side Slopes (z:1)	= 3.00, 3.00	Q (cfs)	= 8.140
Total Depth (ft)	= 2.30	Area (sqft)	= 8.07
Invert Elev (ft)	= 480.91	Velocity (ft/s)	= 1.01
Slope (%)	= 1.50	Wetted Perim (ft)	= 10.47
N-Value	= 0.150	Crit Depth, Yc (ft)	= 0.60
		Top Width (ft)	= 10.04
		EGL (ft)	= 1.36
Calculations	Known Q		
Compute by:	Known Q		
Known Q (cfs)	= 8.14		



EXISTING DRAINAGE AREAS



PROPOSED DRAINAGE AREAS



**FOR DRAINAGE PURPOSES ONLY.
NOT FOR CONSTRUCTION!**

PLANNING & DEVELOPMENT DIVISION FILE NUMBER: 22-003084
 SITE PLAN APPROVAL: MAY 5, 2022
 CONDITIONAL USE PERMIT APPROVAL: MAY 12, 2022

PROPERTY OWNER: MW REAL ESTATE HOLDINGS LLC
 1601 W. OLD RTE 66
 STAFFORD MO, 65757

Call BEFORE you DIG
 TOLL FREE
 1-800-344-7483
 MISSOURI ONE-CALL SYSTEM, INC.

UNDERGROUND UTILITIES HAVE BEEN PLOTTED FROM AVAILABLE INFORMATION AND THEREFORE THEIR LOCATIONS SHALL BE CONSIDERED APPROXIMATE ONLY. THE VERIFICATION OF THE LOCATION OF ALL UNDERGROUND UTILITIES, EITHER SHOWN OR NOT SHOWN ON THESE PLANS, SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR, AND SHALL BE LOCATED PRIOR TO ANY GRADING AND/OR CONSTRUCTION OF IMPROVEMENTS.

CONSTRUCTION PLANS
TRANSLAND TRUCKING
 3480 E. TERRA LANE
 O'FALLOON MO, 63366
 DRAINAGE AREA MAPS

ST. CHARLES ENGINEERING & SURVEYING, INC.
 801 S. FIFTH STREET, SUITE 202
 ST. CHARLES, MO 63301
 TEL: (636) 947-0607 FAX: (636) 947-2448

ST. CHARLES ENGINEERING AND SURVEYING, INC.
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 MISSOURI STATE CERTIFICATE OF AUTHORITY - 001647 & 000379

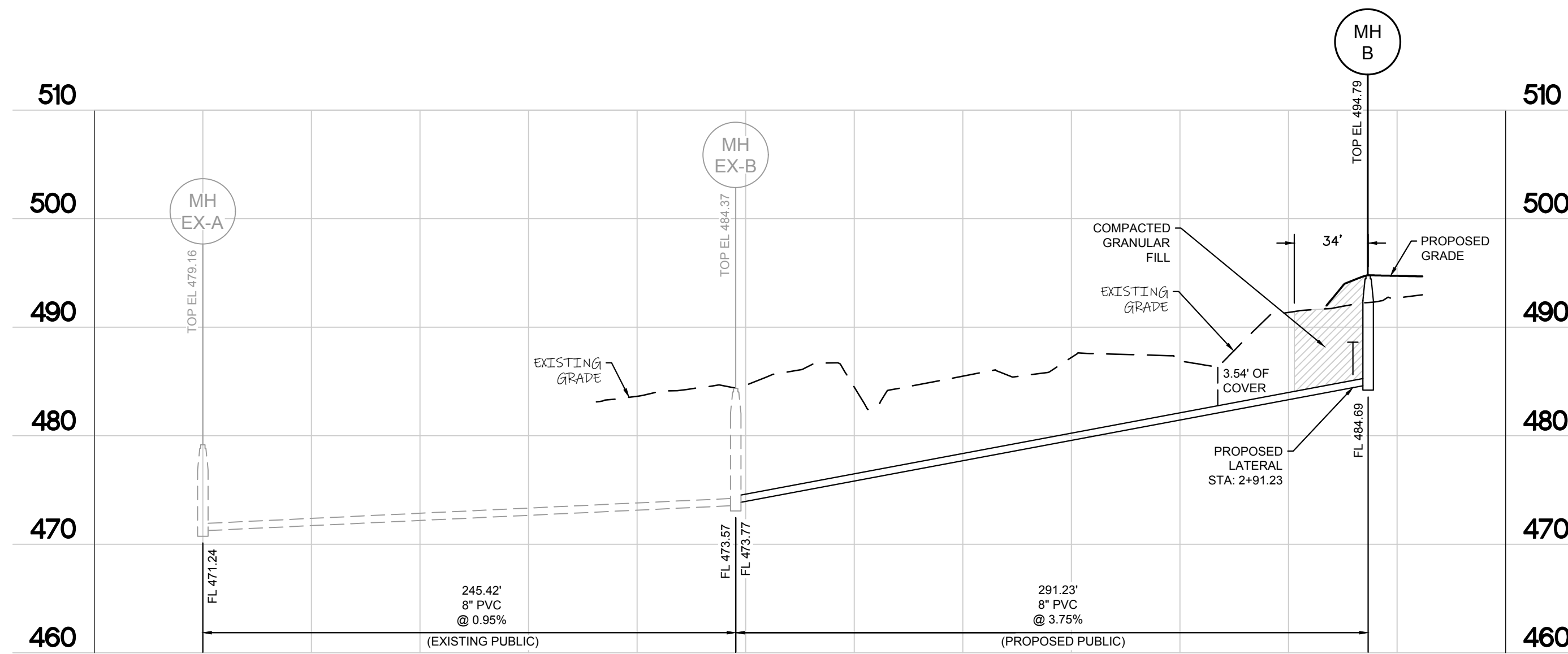
S | C | E | S

STATE OF MISSOURI
 MICHAEL NEWELL
 MEASUREMENTS
 NUMBER
 E-22483
 PROFESSIONAL ENGINEER
 11/03/2022

ORDER NO.
 2022032

DATE
 11/03/2022

6

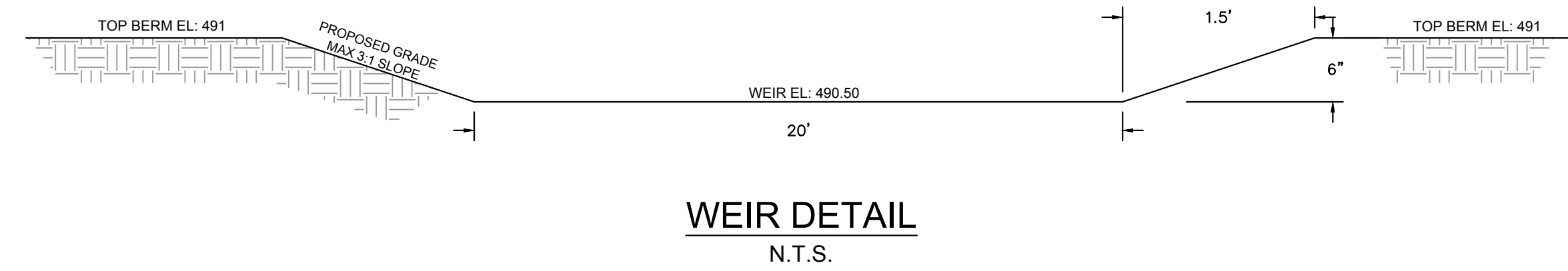


SANITARY SEWER PROFILE

SCALE: HORIZ.: 1" = 50'
VERT.: 1" = 10'

15YR.-20MIN. HYDRAULICS

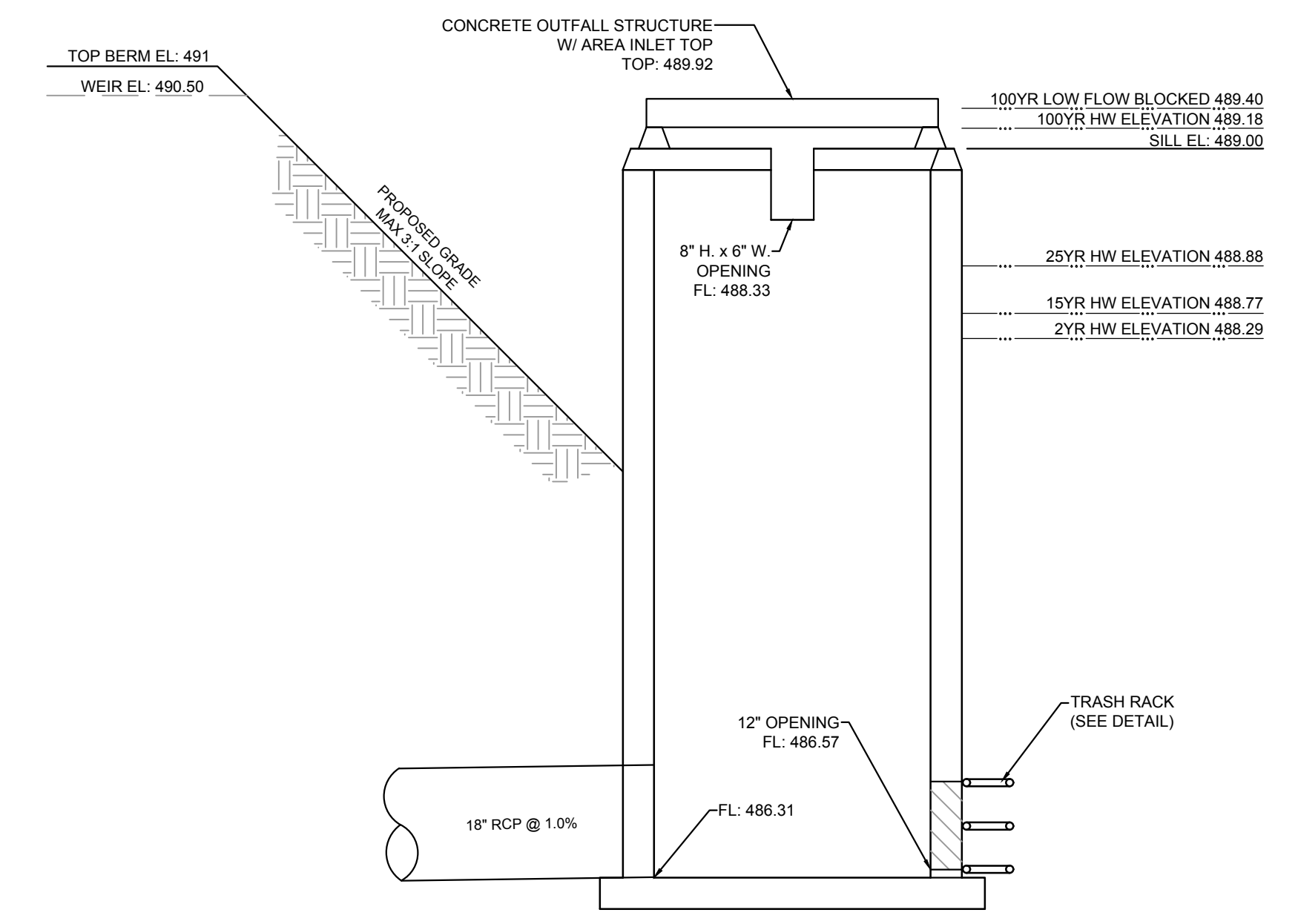
Line No.	Line ID	DnStm Ln No	Invert (ft)	Gnd/Rim El Dn (ft)	Invert Up (ft)	Gnd/Rim El Up (ft)	Line Length (ft)	Line Slope (%)	Line Size (in)	n-val Pipe	Known Q (cfs)	Capac Full (cfs)	Flow Rate (cfs)	Vel Ave (ft/s)	Energy Loss (ft)	Minor Loss (ft)	HGL Dn (ft)	HGL Up (ft)	Rim-Hw (ft)
1	1-2	Outfall	485.98	488.95	486.31	490.92	33.136	1.00	24	0.012	13.02	24.45	13.02	4.35	0.091	0.32	487.98	488.01	2.58
2	3-4	Outfall	487.00	489.21	487.27	491.36	22.825	1.18	18	0.012	4.04	12.37	9.07	5.37	0.140	0.53	488.50	488.56	2.27
3	4-5	2	487.27	491.36	488.22	491.80	94.506	1.01	15	0.012	2.48	7.01	5.03	4.10	0.489	0.13	489.09	489.58	2.09
4	5-6	3	488.22	491.80	489.13	492.70	91.000	1.00	12	0.012	2.55	3.86	2.55	3.28	0.371	0.17	489.71	490.08	2.45
5	7-8	Outfall	489.00	491.21	489.15	492.07	29.991	0.50	18	0.012	3.22	8.05	3.22	4.12	0.000	0.26	489.68	489.83	2.24



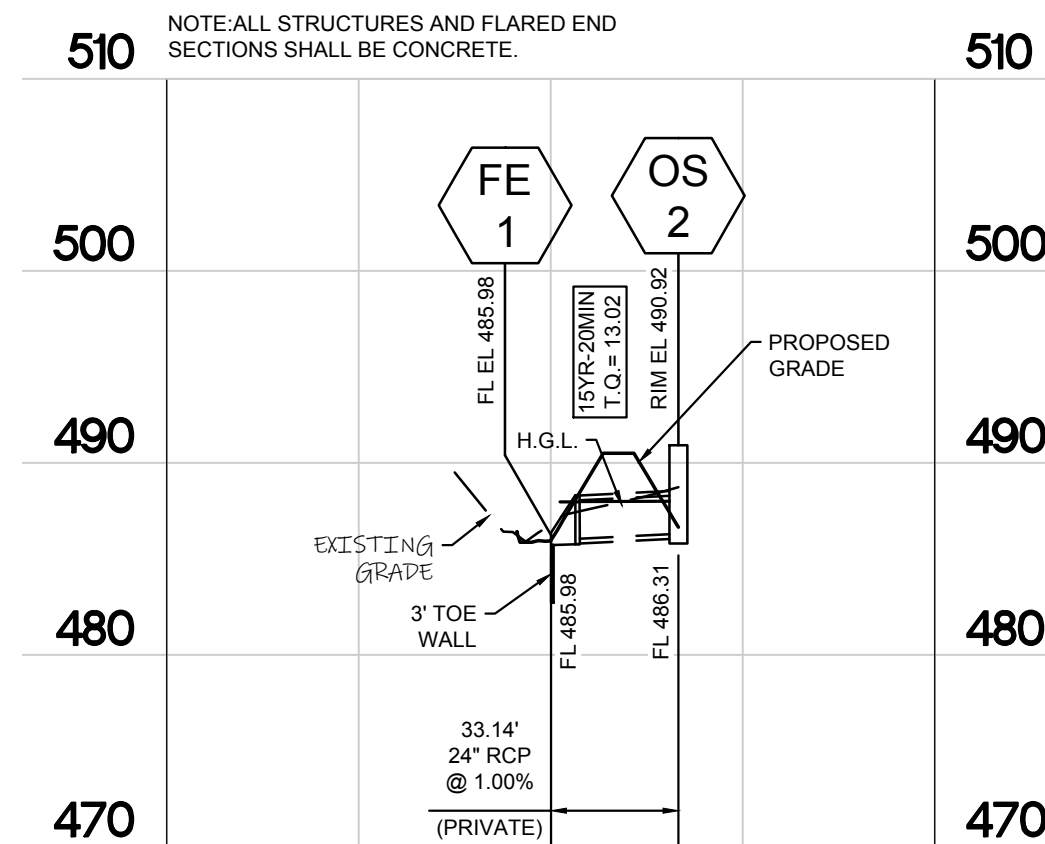
WEIR DETAIL
N.T.S.

STORM SEWER PIPES

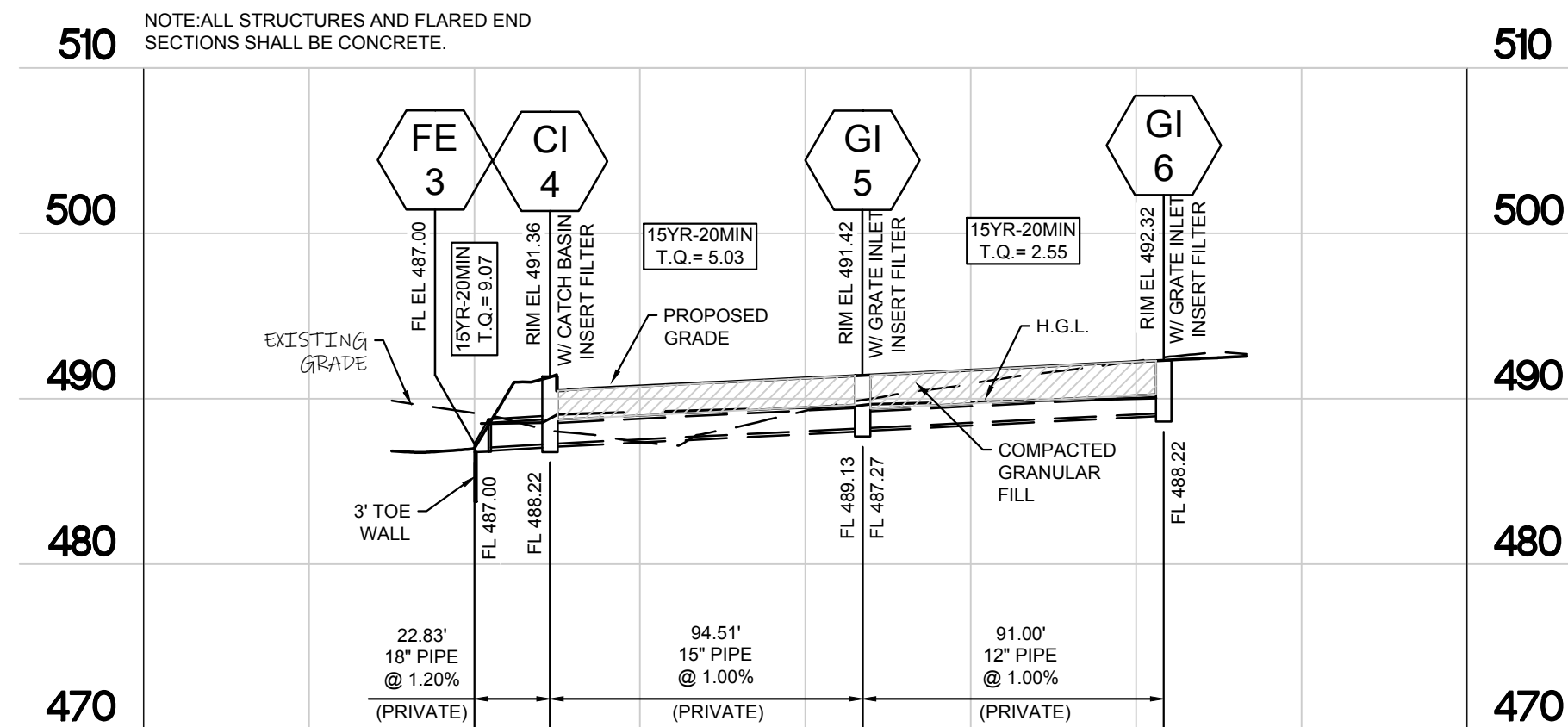
- A. Reinforced Class III Concrete Culvert Pipe.
- B. Dual wall and triple wall polypropylene pipe and fittings 12-inch through 60-inch diameter shall conform to ASTM F2764, except as otherwise specified herein. Dual wall polypropylene pipe (12-30") shall have a smooth interior and annular exterior corrugations. Triple wall polypropylene pipe (30" - 60") shall have a smooth interior and exterior with annular inner corrugations. Pipe shall have a minimum pipe stiffness of 46 psi when tested in accordance with ASTM D2412. Pipe shall be joined with an integral bell and spigot joint on all sizes. The joints shall be watertight in accordance with ASTM D3212. The spigot shall have two gaskets meeting the requirements of ASTM F477. The gaskets shall be installed by the pipe manufacturer and shall be covered with a removable, protective wrap to ensure the gaskets are free from debris. A joint lubricant shall be used on the gasket and pipe bell during assembly. Pipe shall have a reinforced bell with a polymer composite band installed by the manufacturer.



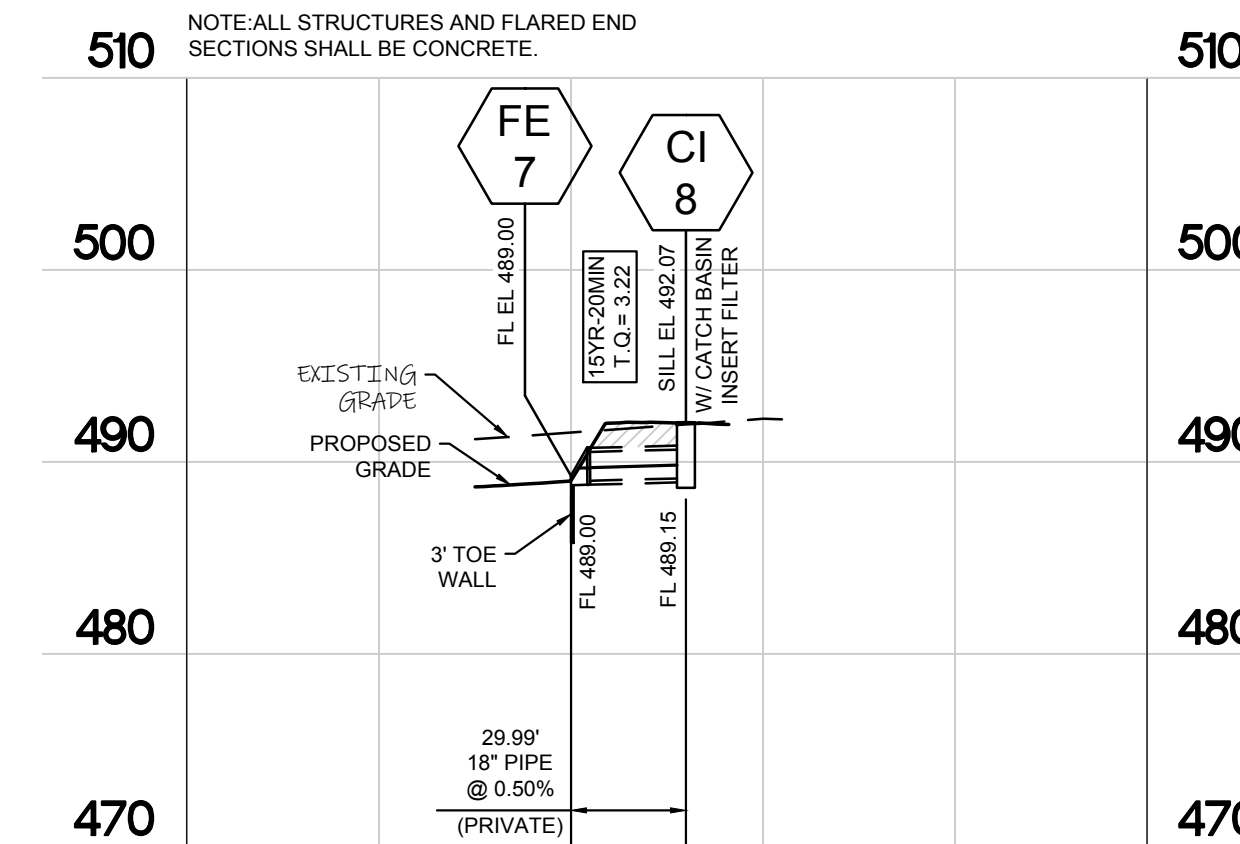
OUTFALL STRUCTURE DIMENSIONAL DETAIL
N.T.S.



BASIN SECTION A-A



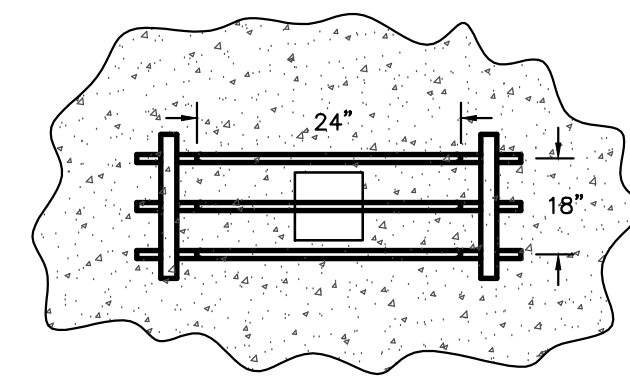
BASIN SECTION B-B



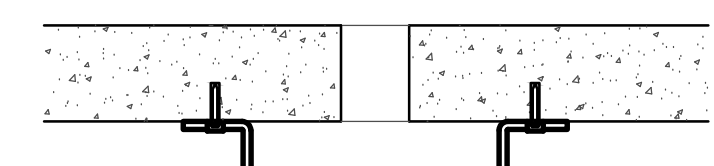
BASIN SECTION C-C

STORM SEWER PROFILES

SCALE: HORIZ.: 1" = 50'
VERT.: 1" = 10'



HORIZONTAL VIEW



TOP VIEW

TRASH RACK DETAIL
N.T.S.

BASIN FLOOD ELEVATIONS:

2 YR. - 20 MIN.	= 488.29
15 YR. - 20 MIN.	= 488.77
25 YR. - 20 MIN.	= 488.88
100YR. - 20 MIN.	= 489.18

BASIN SECTIONS

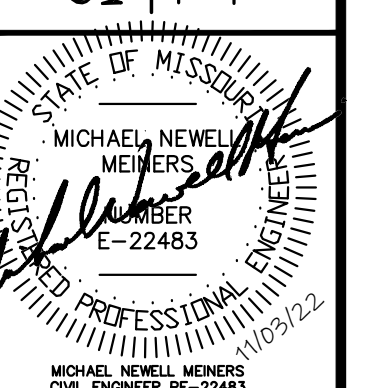
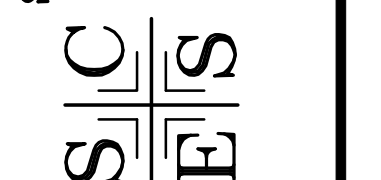
SCALE: HORIZ.: 1" = 50'
VERT.: 1" = 10'

PLANNING & DEVELOPMENT DIVISION FILE NUMBER: 22-003084
SITE PLAN APPROVAL: MAY 5, 2022
CONDITIONAL USE PERMIT APPROVAL: MAY 12, 2022

PROPERTY OWNER: MW REAL ESTATE HOLDINGS LLC
1601 W. OLD RTE 66
STAFFORD MO, 65757

CONSTRUCTION PLANS
TRANSLAND TRUCKING
3480 E. TARRA LANE
O'FALLON MO, 63366
PROFILES

ST. CHARLES ENGINEERING & SURVEYING, INC.
801 S. FIFTH STREET, SUITE 202
ST. CHARLES, MO 63301
TEL: (636) 947-0607 FAX: (636) 947-2448
ST. CHARLES ENGINEERING AND SURVEYING, INC.
A PROFESSIONAL ENGINEERING CORPORATION
MISSOURI STATE CERTIFICATE OF AUTHORITY - 001647 & 000379



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11/03/2022
5

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